Quality Base Material Produced Using Full Depth Reclamation on Existing Asphalt Pavement Structure

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	the three major types of asphalt recyclin				
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and frost heaves.	are frequent transverse and lateral crack	ing, reflective cracking, severe rutting			
	a mix design procedure for FDR. Due	to the large variations among the various			
types of FDR stabilizers, a separate m	h type of FDR. Detailed summary of the				
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Executive Summary

Task 4 outlined preliminary testing of material for potential use as a base course material. The results of this task are as shown below. Some material combinations did not stand up as well as other combinations. The significant findings and testing parameters of this task and recommendations are broken down into the material types tested.

Unstabilized Full Depth Reclamation (FDR)

The good clean (GC) gradation was more difficult to control than the other materials due to its low fines content.

The correlations between the resilient modulus (M_R) and California Bearing Ratio (CBR) values were very weak which led to the recommendation to use the M_R as the strength property for the unstabilized FDR. The M_R properties of the various mixtures seem to be reasonable for a base course layer under a flexible pavement.

FDR Stabilized with Portland Cement (PC) and Fly Ash (FA)

The degree of variability of the measured compressive strength based on three replicate samples was acceptable.

Moisture Sensitivity: the Tube Suction Test (TST) and the Wire Brush Test (ASTM D559) were used to measure the moisture sensitivity of the FDR stabilized with PC and FA. The TST is a research tool and does not have any American Association of State Highway and Transportation Officials (AASHTO) or ASTM International (ASTM) designations. One modification was necessary for the ASTM D559 test which dealt with the removal of materials finer than the #4 (4.75 mm) US standard sieve. This step was omitted since it would result in the removal of a large portion of the material. The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with PC and FA:

- o Dry unconfined compressive strength (UCS): 300 500 psi (2.1 3.4 MPa)
- o Tube Suction Test (TST) 14-days dielectric constant: max. 9

FDR Stabilized with Asphalt Emulsion (AE) and Asphalt Emulsion plus Lime (AE+lime)

The dry indirect tensile strength (ITS) at 77°F (25°C) as specified in AASHTO T283 was used to measure the strength properties of the FDR stabilized with AE and AE+lime. The samples were compacted in the Superpave Gyratory Compactor (SGC) as specified in AASHTO T312. Three major modifications were made to the compaction process in the SGC: 1) use of a slotted mold to allow water to escape during the compaction process, 2) the samples were compacted to 40 gyrations, and 3) the sensor that counts the number of gyrations in the SGC was placed inside the chamber adjacent to the mold carriage to avoid being damaged by the water seeping out of the sample during the compaction process. The tensile strength ratio (TSR), after one freeze-thaw cycle as described in AASHTO T283, was used to evaluate the moisture sensitivity of the FDR stabilized with AE and AE+lime. The samples for the moisture conditioned ITS were compacted in the SGC following the same procedure used for the dry ITS test samples. The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with AE and AE+lime:

o Dry ITS at 77°F (25°C): min. 30 psi (0.2 MPa)

o TSR at 77° F (25°C): 70%

FDR Stabilized with Foamed Asphalt plus Portland Cement (foamed asphalt + PC)

The dry ITS at 77°F (25°C) as specified in AASHTO T283 was used to measure the strength properties of the FDR stabilized with foamed asphalt + PC. The samples were compacted in the SGC as specified in AASHTO T312. Three major modifications were made to the compaction process in the SGC: 1) use of a slotted mold to allow water to escape during the compaction process, 2) the samples were compacted to 40 gyrations, and 3) the sensor that counts the number of gyrations in the SGC was placed inside the chamber adjacent to the mold carriage to avoid being damaged by the water seeping out of the sample during the compaction process. The TSR after one freeze-thaw cycle, as described in AASHTO T283, was used to evaluate the moisture sensitivity of the FDR stabilized with foamed asphalt + PC. The samples for the moisture conditioned ITS were compacted in the SGC following the same procedure used for the dry ITS samples. The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with foamed asphalt + PC:

o Dry ITS at 77°F (25°C): min. 30 psi (0.2 MPa)

o TSR at 77° F (25°C): 70%

UNIT CONVERSIONS

The following is a conversion chart for use in converting units where necessary.

$$1pcf = 157.087 \frac{N}{m^3}$$

 $1in = 25.4 \, \text{mm}$

1psi = 6.895 kPa

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1. INTRODUCTION

The overall objective of Task 4 is to evaluate and recommend the appropriate laboratory tests that can be used in a mix design for FDR. The types of FDR materials evaluated in Task 4 are: unstabilized, stabilized with Portland cement, stabilized with fly ash, stabilized with asphalt emulsion, and stabilized with foamed asphalt. In addition to the stabilization, all FDR materials included three levels of recycled asphalt pavements (RAP) at: 25, 50, and 75%.

The approach used to evaluate the appropriate laboratory test methods consisted of evaluating the standard methods that have been historically used to conduct designs of FDR materials and other methods that have been recently introduced to the pavement industry. In addition, the evaluated methods needed to be consistent with the two currently available flexible pavement design methods: the AASHTO 1993 Pavement Design Guide and the AASHTO Mechanistic-Empirical Pavement Design Guide (MEPDG). In other words, the methods selected for the mix design process of FDR needs to generate properties that can be directly used in the two pavement design methods.

As a first step, the various standard laboratory tests were identified for each of the five FDR types. In this context, the term standard means that the test method has an established AASHTO or ASTM standard procedure. Once the tests were identified, their applicability and repeatability were evaluated using four types of virgin aggregate: good clean, good dirty, poor clean, poor dirty. The applicability of the tests was evaluated in terms of their ability to produce results that are consistent with engineering principles of paving materials. The repeatability of the tests was evaluated in terms of their ability to generate reproducible results on multiple samples.

In cases where standard tests did not exist, new test methods were evaluated and modified as needed to be applicable for the design of FDR materials. An example of this situation is the use of the Superpave Gyratory Compactor (SGC) for the design of FDR stabilized with asphalt emulsion and FDR stabilized with foamed asphalt. FDR materials stabilized with asphalt emulsion or foamed asphalt are not as stable as hot mixed asphalt mixtures and usually contain free moisture. These unique conditions necessitate a few modifications to adopt the SGC to the design of FDR materials while keeping the same overall design concept.

In summary, the evaluated and recommended mix design methods for FDR materials were selected to generate engineering properties that can be directly incorporated into the two commonly used pavement design methods for flexible pavements. This report summarizes the data generated from the evaluations of the various laboratory tests and recommends any needed modifications of the test methods to make them applicable for the design of FDR materials. A concerted effort was made to recommend laboratory tests commonly used in the design of paving materials in order to eliminate the need of state highway agencies and the consulting industry to purchase new equipment which will require additional training of their personnel.

1.1 Material Description

The experimental program included the testing matrix shown in Table 1 which involves five stabilization methods with four levels of Recycled Asphalt Pavement (RAP) material of 0, 25, 50,

and 75%. The 0% RAP represents the unstabilized material. The good source material was defined as aggregates with fractured faces and was obtained from Pete Lien and Sons, Incorporated in Rapid City, South Dakota. The poor source material was uncrushed rock that had no fractured faces and was obtained from the eastern side of South Dakota. The clean gradation was defined as having less than 10% of the material passing a #200 (0.075mm) US standard sieve. The actual percent passing the #200 (0.075mm) sieve for the good dirty gradation was originally 20.7%; however, with stockpile migration problems, the percent passing ended up being 14.7%. The poor source material had 10.7% passing the #200 (0.075mm) sieve. Therefore, an additional 4% fines were added to the poor source clean material to obtain the same fine content for the poor source dirty material. The fines added were pulverized kaolin made from Edgar Plastic Kaolin. The initial gradations are shown in Figure 1. All of the moisture-density curves for the testing regimen are shown in Appendix A.

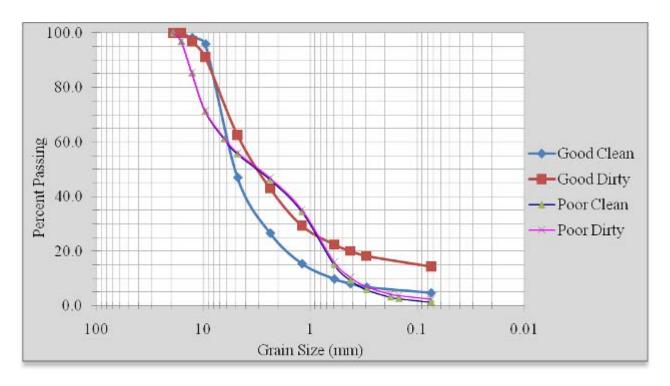


Figure 1: Initial grain size distributions.

Table 1: Testing matrix for task 4.

		FDR Type					
FDR Source	Gradation	Un- stabilized	Stabilized with PC (3, 5, 7%)	Stabilized with Fly Ash (10, 12, 15%)	Stabilized with Asphalt Emulsion (3, 4.5, 6%)	Stabilized with Asphalt Emulsion + 1% Lime (3, 4.5, 6%)	Stabilized with Foamed Asphalt + 1 or 2% PC (2.5, 3, 3.5%)
	Dirty	Moisture- density curve	Moisture- density curve	Moisture- density curve	Superpave Gyratory	Superpave Gyratory	Superpave Gyratory
		M _R and CBR	Compressive strength	Compressive strength	Moisture - density curve	Moisture - density curve	Moisture - density curve
			Moisture sensitivity	Moisture sensitivity	Bulk density using CoreLok	Bulk density using CoreLok	Bulk density using CoreLok
Poor					Moisture sensitivity	Moisture sensitivity	Moisture sensitivity
1 001	Clean	Moisture- density curve	Moisture- density curve	Moisture- density curve	Superpave Gyratory	Superpave Gyratory	Superpave Gyratory
		M _R and CBR	Compressive strength	Compressive strength	Moisture - density curve	Moisture - density curve	Moisture - density curve
			Moisture sensitivity	Moisture sensitivity	Bulk density using CoreLok	Bulk density using CoreLok	Bulk density using CoreLok
					Moisture sensitivity	Moisture sensitivity	Moisture sensitivity
Good	Dirty	Moisture- density curve	Moisture- density curve	Moisture- density curve	Superpave Gyratory	Superpave Gyratory	Superpave Gyratory
		M _R and CBR	Compressive strength	Compressive strength	Moisture - density curve	Moisture - density curve	Moisture - density curve
			Moisture sensitivity	Moisture sensitivity	Bulk density using CoreLok	Bulk density using CoreLok	Bulk density using CoreLok
					Moisture sensitivity	Moisture sensitivity	Moisture sensitivity
	Clean	Moisture- density curve	Moisture- density curve	Moisture- density curve	Superpave Gyratory	Superpave Gyratory	Superpave Gyratory
		M _R and CBR	Compressive strength	Compressive strength	Moisture - density curve	Moisture - density curve	Moisture - density curve
			Moisture sensitivity	Moisture sensitivity	Bulk density using CoreLok	Bulk density using CoreLok	Bulk density using CoreLok
					Moisture sensitivity	Moisture sensitivity	Moisture sensitivity

2. TEST METHODS

The test methods used follow as closely and practically as possible ASTM or AASHTO standards. The following sections describe the tests performed, the deviations from the standards and why these deviations were necessary.

2.1 Unstabilized FDR (Mechanically Stabilized)

This includes materials stabilized using only mechanical compaction means. These tests were performed on 0, 25, 50, and 75% RAP mixtures with good clean, good dirty, poor clean, and poor dirty material.

2.1.1 Moisture -Density Curves (MDC)

A moisture -density curve was established for each material following ASTM D1557-02. Method C was used for a six 6-in. (150 mm) mold with five layers and 56 blows per layer. The material was first sieved through a 3/4-in. (19 mm) sieve, dried out and mixed together with RAP to form the correct proportions. Water was added to target values that would create a five point moisture -density curve. The target mixture values are shown in Appendix B. The 10-lb. (4.5 kg) blows were administered using a Forney air powered automatic compactor. The sector -faced rammer was allowed to fall 18 in. (0.46 m) before contacting the soil. Once the specimen was compacted, the mold was removed from the compactor and the excess material above the collar removed. The sample was then weighed and extruded from the mold and moisture samples were taken from the center of the sample. Figure 2 shows a typical MDC for the 0% RAP, poor source, clean gradation material. The optimum moisture content (MC) and maximum dry density (ρ) are then read off the chart. The material shown in Figure 2 has an optimum MC of 6.7% and a density of 137pcf (2195 kcm). Table 2 summarizes the optimum MC and density for all materials tested.

2.1.2 Resilient Modulus (M_R)

The M_R test was performed on the unstabilized FDR materials using AASHTO T307-99. The samples for the M_R test were made in a four in. (100 mm) dia. mold and compacted by using vibration in five equal lifts. As shown in Figure 3, the material for each lift was weighed based on the optimum moisture and density from the compaction curves (Table 2). A base plate was placed in the bottom of the mold followed by filter paper and material. After the compaction of each lift, the top of the lift was scarified using a metal spatula. Then the next lift was added and compacted as shown in Figure 4. After the fifth and final lift was compacted, a filter paper and a porous stone were placed on the top. The sides of the three sided compaction mold were then carefully removed as shown in Figure 5. The sample was then inverted while holding the top and bottom spacer and then the porous stone was placed on the loading platen as shown in Figures 6 and 7. Finally, the spacer was removed and a porous stone was placed on top followed by the top loading platen. With the grooves lubricated to prevent leaks, a rubber membrane and o-rings were placed on the sample using a vacuum membrane stretcher as shown in Figures 8 through 10.

The sample was tested in the Simple Performance Tester (SPT) manufactured by Interlaken using the stress conditions recommended in the AASHTO standard (Table 2 in T307). Figure 12 shows a sample being tested in the SPT machine. The only deviation from the AASHTO T307-99 standard was the 2:1 height to diameter ratio. The SPT machine could only fit a sample with the dimensions of four -in. (100 mm) dia. and six -in. (150 mm) height leading to a ratio of 1.5. The SPT machine recorded the results in Microsoft Excel file in accordance with AASHTO T307. The measured data at the various stress levels were ran through a regression analysis to develop the relationship between M_R and the state of stresses within the sample as shown in Equation 1.

$$M_R = k_1 \times p_a \times \left(\frac{\theta}{p_a}\right)^{k_2} \times \left(\frac{\tau_{oct}}{p_a} + 1\right)^{k_3}$$
(1)

Where,

 M_R = resilient modulus at optimum MC,

 k_1 , k_2 , k_3 = regression parameters,

p_a = atmospheric (normalizing) pressure,

 $\theta = \sigma_1 + \sigma_2 + \sigma_3 = \text{bulk stress}$, and

$$\tau_{oct} = \frac{1}{3}\sqrt{(\sigma_1 - \sigma_2)^2 + (\sigma_1 - \sigma_3)^2 + (\sigma_2 - \sigma_3)^2} = \text{octahedral shear stress}.$$

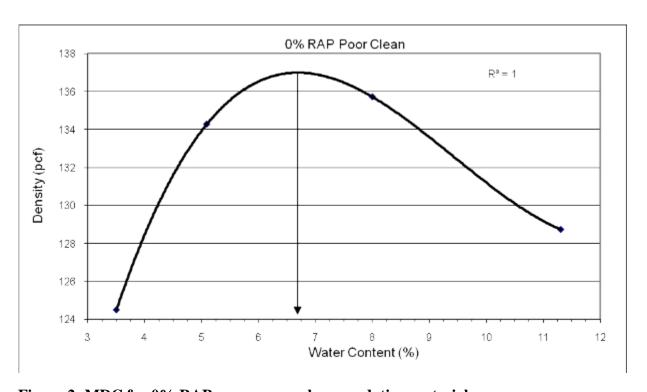


Figure 2: MDC for 0% RAP poor source clean gradation material.

Table 2: Optimum moisture content and maximum dry density for the various FDR mixtures.

GOOD SOURCE - CLEAN GRADATION									
GOOD SOURCE - CLEAN GRADATION Stabilized									
% of		Unstabilized	Cement Fly Ash						
RAP		Unstabilized	3%	5%	7%	10%	12%	15%	
	MC (%) =	9.30	10.25	8.25	9.00	7.28	7.25	6.67	
75%	$\rho (lb/ft^3) =$	120.20	120.90	122.25	124.80	128.00	127.20	129.00	
50%	MC (%) =	7.70	9.20	8.60	8.75	7.50	7.50	7.52	
	$\rho (lb/ft^3) =$	118.90	125.00	125.50	128.00	129.00	130.00	130.30	
	MC (%) =	9.00	9.75	10.30	9.20	7.20	7.30	6.60	
25%	$\rho (lb/ft^3) =$	128.00	128.00	127.50	129.50	131.40	133.00	132.80	
0.04	MC (%) =	8.25							
0%	$\rho (lb/ft^3) =$	132.50							
	, ,	GOOD	SOURCE - 1	DIRTY GRA	ADATION				
0/ of					Stabi	lized			
% of RAP		Unstabilized		Cement			Fly Ash		
10/11			3%	5%	7%	10%	12%	15%	
75%	MC (%) =	9.80	8.85	9.20	8.60	7.60	7.35	6.80	
7370	$\rho (lb/ft^3) =$	119.75	122.80	121.80	125.50	128.50	126.30	128.50	
50%	MC (%) =	7.90	8.60	8.60	7.90	7.10	7.30	6.80	
3070	$\rho (lb/ft^3) =$	128.00	128.00	128.00	130.60	128.00	129.50	128.90	
25%	MC (%) =	8.10	7.70	7.60	7.80	7.60	7.20	7.10	
2570	$\rho (lb/ft^3) =$	132.50	134.50	133.60	133.10	129.90	130.20	130.00	
0%	MC (%) =	7.25							
	$\rho (lb/ft^3) =$	135.30							
		POOR S	OURCE - C	CLEAN GRA		1' 1			
% of		TT4-1-1111		Stabilized					
RAP		Unstabilized	20/	Cement	70/	100/	Fly Ash	1.50/	
	MG (0()	0.40	3%	5%	7%	10%	12%	15%	
75%	$\frac{MC (\%) =}{\rho (lb/ft^3)} =$	9.49 116.63	6.85 131.11	7.04 130.00	6.25 130.40	8.06 123.52	6.58 124.53	8.11 123.82	
	MC (%) =	7.72	7.76	5.12	6.05	6.65	7.08	7.89	
50%	$\rho (lb/ft^3) =$	126.06	129.04	134.26	136.31	127.09	125.04	127.29	
	MC (%) =	7.40	6.04	5.26	4.99	5.92	5.22	6.47	
25%	$\rho (lb/ft^3) =$	131.02	138.38	136.41	140.56	127.08	122.93	131.23	
	MC (%) =	6.70	100.00	100111	1.0.00	127.00	122.50	101120	
0%	$\rho (lb/ft^3) =$	137.00							
	1 (2 2 2)		SOURCE - I	DIRTY GRA	ADATION				
0/ 6					Stabi	lized			
% of RAP		Unstabilized		Cement			Fly Ash		
KAP			3%	5%	7%	10%	12%	15%	
75%	MC (%) =	7.40	7.75	7.30	7.20	7.35	6.25	6.50	
75%	$\rho (lb/ft^3) =$	127.00	128.50	128.70	131.50	127.10	131.20	132.60	
50%	MC (%) =	7.00	5.80	6.30	7.00	7.25	5.80	4.50	
3070	$\rho (lb/ft^3) =$	131.00	135.50	134.00	132.90	129.00	134.80	136.80	
25%	MC (%) =	7.25	5.50	6.10	6.50	6.50	5.20	5.15	
	$\rho (lb/ft^3) =$	133.50	137.00	134.50	135.30	134.60	137.00	135.80	



Figure 3: Preparing samples for compaction.



Figure 4: Compacting resilient modulus samples.



Figure 5: Removing the sides of the mold.



Figure 6: Preparing to flip the specimen.



Figure 7: Placing the flipped specimen on the bottom platen.



Figure 8: Testing specimen before the membrane is placed over it.



Figure 9: Placing the membrane on the sample.



Figure 10: Placing the rubber o-rings on the platens.



Figure 11: Adjusting the o-rings for testing.



Figure 12: Resilient modulus sample in the SPT being tested.

2.1.3 California Bearing Ratio (CBR)

The California Bearing Ratio (CBR) test was conducted following ASTM D1883-05. The material was first sieved and dried. A six -in. (150 mm) dia. mold was used to compact the samples at the optimum moisture-dry density condition shown in Table 2. The same compaction method used for the M_R samples was also used for the CBR samples. Soaked CBR samples were not produced for this task, as they were not representative of field conditions. Ten -pound (4.5 kg) surcharge weights were added to the top of the sample and a 1.954 in (1261 mm²) penetration piston was lowered into the sample through the holes in the weights at a rate of approximately 0.05 in (1.3 mm)/min. Two replicates were tested for each material type and the results were averaged. The CBR samples were tested in a GeoTac Sigma-1 automated load test frame. Measurements from the CBR tests were recorded digitally and stored automatically to data files. Figures 13 and 14 show the load frame and setup of the CBR testing.

2.2 FDR Stabilized with Portland Cement (PC) and Fly Ash (FA)

The samples stabilized with PC were tested at three different levels of cement at 3, 5, and 7% by weight of dry aggregate. All evaluations were performed on 0, 25, 50, and 75% RAP mixtures with good clean, good dirty, poor clean, and poor dirty material.

The samples stabilized with FA were tested at three different levels of FA at 10, 12, and 15% by weight of dry aggregate. All evaluations were performed on 0, 25, 50, and 75% RAP mixtures with good clean, good dirty, poor clean, and poor dirty material. The same tests were used to evaluate the FDR materials stabilized with PC and FA.

2.2.1 Moisture -Density Curves (MDC)

The moisture -density curves were obtained using the same process as the unstabilized material (Table 2).

2.2.2 Unconfined Compressive Strength

The unconfined compressive strength (UCS) cylinders were made following ASTM D1633-00 for the PC -stabilized FDR and ASTM C593-95 for the FA -stabilized FDR. The PC -stabilized samples were made following Method A of the specification. The material was mixed to the correct percentages and moisture contentMC as determined by the compaction curves (Table 2). The samples were placed in a four -in. (100 mm) dia. mold and compacted using the Forney compactor in five layers using 25 blows per layer. The samples were kept in the molds overnight and placed in a moisture curing room. The samples were removed from the molds the next day and placed in the moist curing room for six more days for a total of seven days. Following the seven day moist curing, the samples were placed in water for four hours and then broken in the GeoTacan InstroTek, Incorporated loading frame. The load was applied at a rate of approximately 0.05in. (1.3mm)/min. Figures 15 and 16 show the loading and failure mode of the tested cylinders.



Figure 13: CBR sample in loading frame.



Figure 14: CBR with loading piston being lowered into test specimen.



Figure 15: Loading of the unconfined compression samples.



Figure 16: Breaking of the unconfined compression samples.

2.2.3 Moisture Sensitivity

The moisture sensitivity of the PC -stabilized and FA -stabilized FDR were evaluated using both the TST and the Wire Brush Tests.

2.2.3.1 Tube Suction Test

The TST samples were compacted using the same method as the compressive strength samples up to the curing period. The samples were cured on a porous stone and then placed in the oven for 48 hours. The samples were removed and allowed to cool to room temperature at which time a membrane was placed around each sample. Five initial readings of the dielectric constant were taken at equal intervals around the sample using the Rainbow dielectric meter. The testing meter is shown in Figure 17. The specimens were then placed on a grate and water was added until approximately ¼ in. (6 mm) above the porous stone as shown in Figure 18.

Plastic was placed over the samples to prevent moisture from escaping. Five additional readings of the dielectric constant were taken every day for 14 days as shown in Figure 19. Following the readings, the UCS test was run on the sample to ensure they met the required strength ratio of 75% of the original unsoaked specimens.



Figure 17: Rainbow dielectric meter.



Figure 18: Moisture sensitivity samples.



Figure 19: Testing the samples with the dielectric sensor.

2.2.3.2 Wire Brush Test

Specimen preparation was conducted in accordance with ASTM D559-03 test Method B. All materials (including RAP) were air dried and sieved through a 3/4 -in. (19 mm) sieve. The determination of the amount of material and water, based on the maximum dry densities and

optimum MC from Table 2, were placed into a mixer. PC was added at three different percentages of 3, 5, and 7%. FA was added at three different percentages of 10, 12, and 15%. ASTM D559 test Method B requires the use of a # 4 (4.75 mm) sieve to remove soil particles finer than 0.19 in. (4.75 mm). This step was not used because too much of the material would have been discarded. Six samples of each material mix were made.

The compaction method followed the modified Proctor test method used to make the MDC samples. A four -in. (100 mm) dia. Proctor mold was weighed and had the inside coated with lubricant. The specimen was compacted with 25 blows per layer in five equal layers by weight. The Proctor hammer weighed ten pounds (4.5 kg) and dropped 18 in. (0.46 m). The Proctor mold extension was removed after compaction and the surface was leveled with a straight edge by shearing the protruding soil. Then, the sample was weighed and recorded together as the mold and compacted specimen weight. All produced specimens were then placed in a cure room for seven days; the mold however, may be removed from the specimen after one day. After the molded samples were placed in a curing room for seven days they were weighed and dimensions measured.

The moisture conditioning test continued in accordance with ASTM D559 procedure 7.3. After removal from the curing room, the specimen's weight, diameter, and height were measured with a scale and caliper. Cycle one began with the specimen being submerged in water for five hours with a deviation of \pm 15 minutes. Afterward, the samples were removed from the water and allowed to reach a saturated surface dry state. The specimen's weight, diameter, and height were again recorded. The specimens were then placed in an oven at 160°F (71°C) for 42 hours. The samples were then removed from the oven and allowed to cool until they could be handled. The specimen's weight, height, and diameter were recorded. Figure 20 shows samples that were molded and ready to be wire brushed. The specimen was then brushed 20 times on the side and four times on each end (Figures 21 and 22), after which the final specimens were weighed, ending cycle one. Figure 23 shows the samples being submerged in water for five hours. The test procedure was then repeated with 11 more cycles for a total of 12 cycles. Figure 24 shows a set of samples following the 12 cycle testing procedure.



Figure 20: Molded and cured wire brush samples.



Figure 21: Samples being wire brushed 4 times on the ends.



Figure 22: Samples being wire brushed 18 to 20 times on the sides.



Figure 23: Samples being soaked between the wire brush cycles.



Figure 24: Good Clean material with 50% RAP and 12% FA after 12 cycles.

2.3 FDR Stabilized with Asphalt Emulsion (AE) and Asphalt Emulsion and Lime (AE+Lime)

The samples stabilized with AE were tested at three different levels of emulsion of 3, 4.5, and 6% by weight. The AE+Lime materials were made by the addition of 1% lime to the AE mixtures. All evaluations were performed on 0, 25, 50, and 75% RAP mixtures with good clean, good dirty, poor clean, and poor dirty material.

2.3.1 Moisture -Density Curves (MDC)

The MDC for this material was used from the unstabilized material. These values were used because of the small amount of emulsion used and the non-absorptive nature of the aggregate. It was assumed that this would not change the values of the optimum MC or the maximum dry density enough to make a significant difference in the results.

2.3.2 Superpave Gyratory Compactor (SGC)

The amount of FDR source material and the percentage of RAP being tested were weighed into the mixing bowl and mixed for 60 seconds. If lime and/or fines were required, they were weighed and added to the mixing bowl, then mixed for an additional 60 seconds. The amount of water was calculated based on the optimum moisture percentage, added to the mixing bowl and mixed for an additional 60 seconds. After all of these steps were completed, the mixing bowl was placed back onto the scale and the percentage of emulsion was added. Three different percentages of AE were tested: 3%, 4.5%, and 6% by weight. Before the emulsion was added to the mixture, the emulsion containers were mixed by shaking the container vigorously for several minutes. If needed, the emulsion was stirred until its consistency was the same throughout the container. The contents of the mixing bowl were then mixed for another 60 seconds. The amount of material required for each sample was calculated using the optimum MC and maximum dry density as well as the approximate dimensions of the sample that were to be achieved. The material was placed into a plastic container having a diameter of approximately six in. (150 mm) and a height of about 12 in. (300 mm). The three samples were placed into the oven set to 104°F (40°C) for 30 minutes. The oven used for this procedure was a Fisher Scientific Isotemp Premium 700 Series Oven, Forced Air Model 750F, as shown in Figure 25.

The appropriate amount of material was placed into a slotted mold to allow water to escape during compaction as shown in Figure 26. Both the top and bottom plates were lubricated with WD-40 before placing filter paper disks. The flange of the mold was also lubricated with a rag moistened with WD-40. The mold was placed into the SGC (Figure 27) and subjected to 40 gyrations at an angle of gyration of 1.26°. After the 40 gyrations, the mold was removed from the chamber, placed over the specimen extruder and the sample was removed from the mold. An example of a specimen being extruded can be seen in Figure 28. The specimen was placed into the oven for 72 hours at $104^{\circ}F$ ($40^{\circ}C$). The SGC used for compaction was model number AFGC125X, Pine Instrument Company, Grove City, Pennsylvania.



Figure 25: Fisher Scientific Oven.



Figure 26: Gyratory compactor with sample in the mold carriage.



Figure 27: Slotted mold and sample placed in the SGC.



Figure 28: Sample being extruded.

A few problems did arise while using the SGC to compact specimens of FDR stabilized with AE. The biggest issue was the excess liquid being forced out of the samples during compaction. This water would escape out of the slotted mold and leak down inside the SGC, causing one of the sensors to malfunction. This sensor counts the number of gyrations and is located in the compartment below the mold carriage. To fix this problem, the machine was altered with the sensor being moved inside the chamber adjacent to the mold carriage. This alteration eliminated most of the problems. Another problem that took place was the lower chambers of the compactor needed to be cleaned after each day of use. This was due to the build-up from the liquid on the mechanical parts (such as the chain) of the compactor.

2.3.3 Bulk and Maximum Density Using CoreLok

The bulk specific gravity of the compacted samples was determined using an InstroTek, Inc. CoreLok machine. The procedure for this test was obtained from the CoreLok Operator's Guide. The first step was to check the bag for holes or damage, and with none, weigh the bag on the scale with the cushioned weighing basket attached to the scale in the water tank. The dry sample was also weighed before sealing. The sample was placed inside the bag and then placed inside the CoreLok machine on top of the sliding plate (with rubber strips up to reduce the friction to the bag) in the chamber with approximately one in.(25 mm) of the open end of the bag resting on the sealing bar. The vacuum was set to 99% on the CoreLok machine. Once the sample was positioned, the lid was closed for 2 – 3 seconds until the vacuum started as shown in Figure 29. After the cycle had run through and the lid automatically opened, the sample was removed and the bag was gently pulled on to make sure the bag was tightly conformed to the sample as demonstrated in Figure 30.

The sample was submerged into the water tank and gently placed on the cushioned weighing basket being sure any air bubbles were removed from the folded areas. The scale was allowed to stabilize and the weight was recorded. The sample was removed from the water tank and the bag was cut open without getting water on the sample. The sample was again weighed and the weight recorded which had to be within \pm 0.18 oz. (5.0 gm) of the original weight of the dry sample. If it was not within the allowance, the test was repeated after the sample had dried. The calculations were performed as indicated on the data collection table. An example of this can be found on the data CD.

The maximum specific gravity of the loose mix was also determined with the CoreLok machine using the procedures from the CoreLok Operator's Guide. The machine was set to a dwell time of 300 seconds and all three filler plates were placed inside the chamber. The total weight of one large bag and one channel bag was recorded, including the weight of the dry sample of up to 70.55 oz. (2,000 grams). The sample was placed inside the channel bag and the channel bag with sample were placed inside a large bag with the channel side face down. The large bag was placed inside the chamber and the sample was spread evenly within the internal channel bag. The external bag opening was placed over the seal, making sure the internal bag was not over the seal. The lid was closed and the machine ran its cycle with the door opening automatically. The sample was placed inside the water tank and, while the bag was completely submerged (at least two in. (50 mm) under water), the bag was cut under the seal joint leaving about one in.(25 mm) intact. Both bags were opened while under water for 15 seconds, allowing the water to enter the bags and wet the sample. The bags were folded over, placed on the weighing basket, and opened back up to freely let water enter the bags. Once the scale had stabilized, the weight was recorded and the calculations were performed according to the data collection table as shown on the data CD.



Figure 29: Specimen in bag before testing.

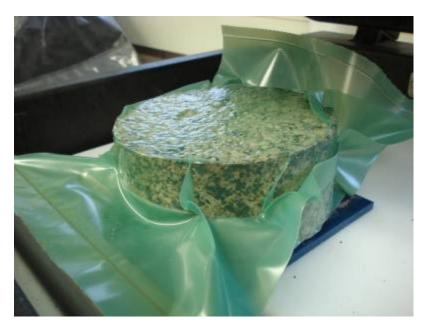


Figure 30: Specimen in bag after testing.

2.3.4 Moisture Sensitivity

Two options for moisture conditioning were available for both the AE- and the foamed asphalt - stabilized FDR. The first option was the TST, which is the same method that was used to determine the moisture susceptibility of the unstabilized FDR. The TST works by measuring the dielectric constants for each material sample and recording the changes that occur as moisture is absorbed into the sample due to capillary action. The second method was the moisture sensitivity according to the AASHTO T283 standard. These tests were performed on the optimum design mixtures.

2.3.4.1 AASHTO T283 Test

The AASHTO T283 uses a freeze-thaw cycle to simulate the long term moisture damage of asphalt mixtures in the field. After the sample is subjected to a saturation level between 70 and 80%, it is tightly wrapped with a plastic sheet and placed in a plastic bag with 0.34 ± 0.17 ounces $(10 \pm 5 \text{ mL})$ of water. The bag is sealed and placed in a freezer at a temperature of $0 \pm 5^{\circ}\text{F}$ (-18 $\pm 3^{\circ}\text{C}$) for a minimum of 16 hours. The sample is removed and placed in a water bath at $140 \pm 2^{\circ}\text{F}$ (60 $\pm 1^{\circ}\text{C}$) for 24 ± 1 hour. The sample is placed into another water bath at $77 \pm 1^{\circ}\text{F}$ (25 $\pm 0.5^{\circ}\text{C}$) for 2 hours ± 10 min. This entire process constitutes a complete freeze-thaw cycle.

A total of six samples are compacted in the SGC; three samples are kept dry and referred to as unconditioned, the other three samples are subjected to a freeze-thaw cycle and referred to as conditioned. The indirect tensile strength (ITS) is measured for both the unconditioned and conditioned samples following the AASHTO T283 standard. The ITS testing setup and the testing apparatus are shown in Figure 31. Per AASHTO T283, the standard displacement rate for the loading ram used in the ITS test is two in (50 mm)/min. The maximum load the sample can withstand is measured and the ITS is calculated using Equation 2.

(3)

$$S_t = \frac{2 \times P}{\pi \times t \times D} \tag{2}$$

where,

 S_t = tensile strength (psi)

P = maximum load (lbs)

t = specimen thickness (in)

D = specimen diameter (in)

The TSR is expressed using Equation 3:

$$TSR = \frac{S_c}{S_m} \times 100$$

where,

 S_c = indirect tensile stress for the conditioned specimen

 S_u = indirect tensile stress for the unconditioned specimen.



Figure 31: Indirect tensile strength (ITS) testing setup.

2.4 FDR Stabilized with Foamed Asphalt and Portland Cement

The procedure used in making the foamed asphalt samples was adopted from the Foamed Bitumen Mix Design Procedure Using the Wirtgen WLB10 obtained from Wirtgen America, Incorporated. The entire mix design procedure can be found in the reference section. The mix design procedure includes the preparation of the foamed asphalt binder using the Wirtgen WLB10 machine shown in

Figure 32. Wirtgen America provided the asphalt binder, PG64-22, from Flint Hills Resources. The amount of material for each sample was calculated using the same method as the AE specimens.

The material to be tested was weighed out prior to mixing, along with the appropriate amount of cement and fines, if applicable. Water was also weighed out according to the optimum MC already determined for the material. After the foamed asphalt machine and binder was prepared according to the Wirtgen guide, the material was placed into the pugmill mixer, shown in Figure 33, and turned on until well blended. The PC was then added and mixed, as well as half of the total amount of water. Once all components were mixed thoroughly, the pugmill mixer was relocated directly under the nozzle of the foaming machine, as shown in Figure 34. The timer was then set according to the proper equations in the Wirtgen guide, which depended on the percentage of foamed asphalt being tested. Three different percentages of foamed asphalt were tested for each combination of RAP and material type: 2.5%, 3.0%, and 3.5%. While the pugmill mixer was running at full speed, the automatic timer start button was pushed and the machine began to foam the asphalt into the mixer. After the machine had finished its cycle, the remaining half of the water was added while still mixing. The pugmill mixer was then moved away from the machine to finish mixing. The mix was then removed and placed into a sealed bucket. The process was repeated for each of the FDR mixtures. After mixing, the foaming machine and pugmill were emptied and cleaned according to the Wirtgen guide. The material was not required to be cured in an oven before making samples, but after the samples were compacted in the SGC, they were cured in the oven at 104°F (40°C) for 72 hours.



Figure 32: Wirtgen asphalt binder foaming machine.



Figure 33: Pugmill mixer.



Figure 34: Asphalt binder foaming machine with pugmill mixer.

2.4.1 Moisture -Density Curves (MDC)

The MDC for this material was used from the unstabilized material. These values were used because of the small amount of emulsion used and the non-absorptive nature of the emulsion. It is assumed this would not change the values of the optimum MC or the maximum dry density enough to make a significant difference in the results.

2.4.2 Superpave Gyratory Compactor (SGC)

The procedure for this test was the same one used for the FDR stabilized with emulsion.

2.4.3 Bulk Density Using CoreLok

The procedure for this test was the same one used for the FDR stabilized with emulsion.

2.4.4 Moisture Sensitivity

The procedure for this test was the same one used for the FDR stabilized with emulsion.

3. RESULTS AND ANALYSIS

This section presents the analyses of the overall results from the various tests. The individual test results are summarized in the Appendices.

3.1 Unstabilized FDR (Mechanically Stabilized)

The results and analysis of the unstabilized materials mixed with the proper proportions of RAP are presented in this section.

3.1.1 Moisture -Density Curves (MDC)

The actual values for maximum dry density and optimum MC are summarized in Table 2. Figures 35 and 36 show the maximum dry density decreases and the optimum MC increases as the amount of RAP increases. As shown, the good clean (GC) material was hard to control and did not follow the general trend of the rest of the material. This is due to the gradation of the GC material.

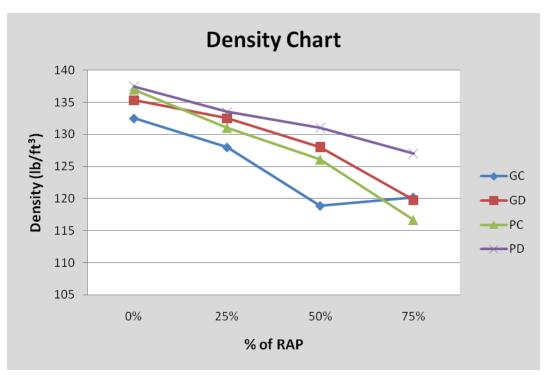


Figure 35: Unstabilized FDR material density chart.

3.1.2 Resilient Modulus (M_R)

Equation 1 was used with the laboratory data to establish a relationship between M_R and the state of stresses within the sample for the unstabilized FDR material. The individual M_R values and calculations are included in Appendix D. However, in order to compare the impact of the various factors, a single representative M_R value was needed. For this purpose, a multi-layer elastic analysis was conducted on a flexible pavement having four in. (100 mm) of Hot Mix Asphalt over a 16 in. (400 mm) of FDR base layer to determine the state of stresses at the middle of the FDR layer under a standard single axle load.

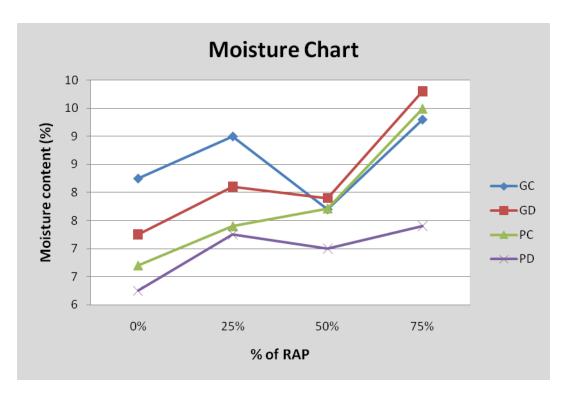


Figure 36: Unstabilized FDR material moisture content chart.

Table 3 summarizes the state of the stresses at the middle of the FDR layer estimated by the multi-layer elastic analysis. The stresses in Table 3 were used in the appropriate M_R relationship to estimate a representative value of M_R at the middle of the FDR layer. The estimated representative M_R values are summarized in Table 4. Figure 37 shows the relationships between the M_R and RAP content of the unstabilized FDR. The data shown in Figure 37 indicate that the impact of RAP on the M_R property is specific to the type of material used, i.e. good or poor. The amount of RAP shows a minor impact on the good material and a significant impact on the poor material.

Table 3: Average stress values used to calculate resilient modulus values.

Stress conver	ntion	Geotech conv	ention	θ bulk (psi)	32.77	τ _{oct} (psi)	8.19
σ^1 and σ^2 sides (psi)	5.14	σ _d (psi)	17.37	θ bulk using dev. Stress (psi)	32.77	τ _{oct} using dev. Stress (psi)	8.19
σ^3 top (psi)	22.50	σ^3 sides (psi)	5.14	Atm. Pressure (p	osi)	14.7	

Table 4: Resilient modulus results and constants.

Good Clean										
RAP %	R ² value	k ¹	k ²	k ³	M _R (psi)					
75	0.96	1,310	0.58	-0.30	26,814					
50	0.97	1,038	0.54	-0.07	22,833					
25	0.96	1,011	0.67	-0.17	23,546					
0	0.90	1,055	0.67	-0.12	25,250					
		Good D								
RAP %	R ² value	k^1	k ²	k^3	M _R (psi)					
75	0.94	1,209	0.56	-0.19	25,581					
50	0.68	1,031	0.63	-0.12	23,843					
25	0.91	992	0.69	-0.26	22,606					
0	0.96	1,126	0.61	-0.10	25,709					
		Poor C								
RAP %	R ² value	k^1	k^2	k^3	M _R (psi)					
75	0.96	1,039	0.68	-0.35	22,589					
50	0.96	772	0.00							
	0.70	112	0.68	-0.22	17,792					
25	0.93	776	0.68	-0.22 -0.10	17,792 18,490					
25 0										
	0.93	776 683 Poor D	0.65 0.74 irty	-0.10 -0.12	18,490					
	0.93	776 683	0.65 0.74	-0.10	18,490					
0	0.93 0.95	776 683 Poor D	0.65 0.74 irty	-0.10 -0.12	18,490 17,197					
0 RAP %	0.93 0.95 R ² value	776 683 Poor D k ¹	0.65 0.74 $irty$ k^2	-0.10 -0.12	18,490 17,197 M _R (psi)					
0 RAP % 75	0.93 0.95 R ² value 0.92	776 683 Poor D k ¹ 1,197	0.65 0.74 irty k ² 0.60	-0.10 -0.12 k ³ -0.31	18,490 17,197 M _R (psi) 24,760					

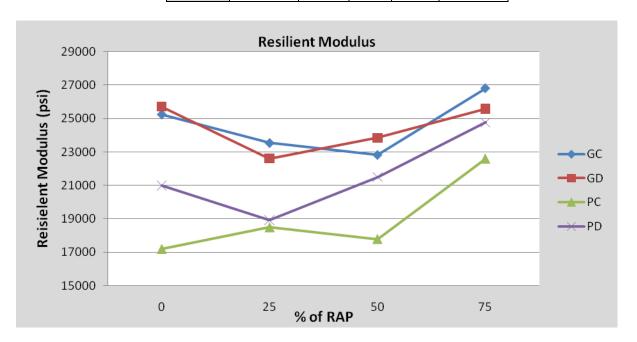


Figure 37: Average resilient modulus values versus RAP content for unstabilized FDR.

3.1.3 California Bearing Ratio (CBR)

Table 5 summarizes the corrected CBR results and the coefficient of variation (CV) as a data quality indicator. The CV is defined as the ratio of standard deviation over the average of the three replicate measurements. The majority of the calculated CVs for the CBR are above 20% indicating high variability of the replicate measurements. Figure 38 shows the relationship between CBR and RAP content which indicates an increase in CBR as the RAP content increases, which does not coincide well with the relationship between M_R and RAP content (Figure 37).

A literature search was conducted in order to determine whether or not state DOTs specify minimum CBR values for the base materials. Unfortunately, no state specifications were located that provided a minimum CBR magnitude. In most instances, state DOTs reference the resilient modulus, M_R , for the design. There are several correlation equations to convert from a CBR value to a M_R value and thus it is possible to determine an approximate range of desirable CBR values from these correlations. However, the published correlations indicate a direct relationship between the CBR and M_R . For this project, it was shown that this relationship was not true for FDR materials and thus it is recommended that the M_R value be solely utilized in the design of these materials.

Table 5: CBR results and statistics.

	(Corrected			
Material	CBR 1	CBR 2	CBR 3	Average CBR	CV (%)
GC-0% RAP	29	45	41	38	21
GC-25% RAP	12	18	17	15	21
GC-50% RAP	9	9	10	9	8
GC-75% RAP	7	11	12	10	23
GD-0% RAP	41	63	52	52	20
GD-25% RAP	24	29	26	26	9
GD-50% RAP	16	15	16	16	6
GD-75% RAP	5	7	7	6	17
PC-0% RAP	27	25	35	29	19
PC-25% RAP	16	26	32	24	34
PC-50% RAP	13	16	11	13	18
PC-75% RAP	5	9	9	8	29
PD-0% RAP	26	47	57	44	36
PD-25% RAP	19	23	26	23	15
PD-50% RAP	11	18	22	17	34
PD-75% RAP	7	11	9	9	20

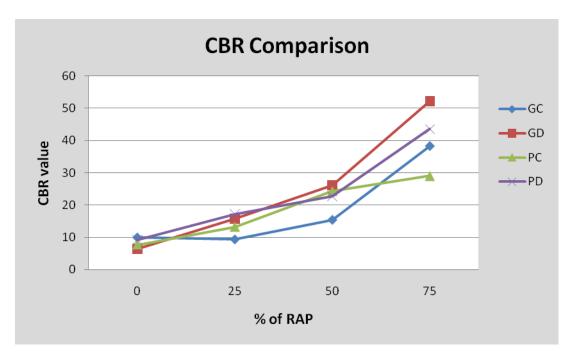


Figure 38: CBR values versus RAP content for unstabilized FDR.

3.2 FDR Stabilized with Portland Cement (PC) and Fly Ash (FA)

The FDR material stabilized with PC and FA are combined in this section because the tests are very similar.

3.2.1 Moisture -Density Curves (MDC)

The actual values for maximum dry density and optimum MC are summarized in Table 2. The only obvious trend in the data summarized in Table 2 for PC- and FA -stabilized FDR is the increase in density as the RAP content increases. On the other hand, the optimum MC does not show any specific trend as a function of RAP content for both the PC and FA contents.

3.2.2 Unconfined Compressive Strength

The UCS test results are summarized in Tables 6 and 7 for the PC- and FA -stabilized FDR, respectively. The "Ratio" column in Tables 6 & 7 represents the ratio of the compressive strength of the TST samples that were soaked for 14 days over the compressive strength of the unconditioned samples. The ratios are over 100% in most cases because the samples continued to hydrate during the TST time of 14 days. The majority of the CV's are below 20%, indicating low variability of the UCS test for the PC- and FA -stabilized FDR. The general trend of the PC - stabilized data indicates a strong positive relationship between compressive strength and PC content. In the case of FA -stabilized FDR, the relationship between UCS and FA content is inconsistent. In addition, the data show that there is an optimum FA content for each type of FDR material.

Table 6: Unconfined compressive strength for PC -stabilized FDR.

		UCS	Sample	e, psi	UCS @ 77 ps	°F,		ST le, psi	UCS @ 77 ps	°F,	
	Cement				Avg.	CV			Avg.	CV	Ratio
Material	%	A	В	C	O		D	Е	O		%
	3	680	174	178	344	84	427	351	389	14	113%
GC-25% RAP	5	306	262	280	282	8	603	675	639	8	226%
	7	249	244	278	257	7	788	788	788	0	306%
	3	123	153	138	138	11	342	370	356	6	259%
GC-50% RAP	5	323	229	374	309	24	401	504	452	16	147%
	7	341	415	466	407	15	788	716	752	7	185%
	3	173	205	177	185	10	278	284	281	2	152%
GC-75% RAP	5	223	213	274	237	14	449	482	466	5	197%
	7	460	385	382	409	11	515	462	489	8	120%
	3	301	371	385	352	13	673	499	586	21	166%
GD-25% RAP	5	581	534	550	555	4	1011	1082	1046	5	189%
	7	1082	698	963	915	21	1504	1536	1520	1	166%
	3	222	232	237	230	3	434	443	438	1	190%
GD-50% RAP	5	462	403	375	413	11	632	630	631	0	153%
	7	1066	1066	1146	1093	4	1162	1042	1102	8	101%
	3	133	173	129	145	17	266	319	293	13	202%
GD-75% RAP	5	200	250	207	219	12	443	453	448	1	205%
	7	359	370	393	374	5	543	537	540	1	144%
	3	300	308	278	295	5	501	632	567	16	192%
PC-25% RAP	5	606	585	576	589	3	1098	1210	1154	7	196%
	7	786	987	786	853	14	1233	1098	1166	8	137%
	3	136	151	186	158	16	322	356	339	7	215%
PC-50% RAP	5	445	380	312	379	18	566	762	664	21	175%
	7	525	619	602	582	9	1050	1090	1070	3	184%
	3	193	144	130	156	21	384	265	325	26	208%
PC-75% RAP	5	257	281	231	256	10	346	388	367	8	143%
	7	510	455	596	520	14	531	604	567	9	109%
	3	468	425	469	454	6	1257	1154	1206	6	266%
PD-25% RAP	5	708	939	731	793	16	1281	1377	1329	5	168%
	7	1035	971	725	910	18	1544	1353	1448	9	159%
	3	478	368	416	421	13	725	692	709	3	168%
PD-50% RAP	5	648	607	728	661	9	697	747	722	5	109%
	7	736	665	787	730	8	1130	1027	1078	7	148%
	3	260	261	242	254	4	443	335	389	20	153%
PD-75% RAP	5	411	461	355	409	13	618	530	574	11	140%
	7	612	614	650	625	3	655	634	645	2	103%

Table 7: Unconfined compressive strength for FA -stabilized FDR.

		UCS	Sample	e, psi	UCS @ 77	_		ST iple,	UCS @ 77		
					ps	i	p	si	ps	i	
	Fly Ash				Avg.	CV			Avg.	CV	Ratio
Material	%	A	В	C			D	Е			%
	10	1552	1353	1695	1533	11	791	796	793	0	52%
GC-25% RAP	12	795	N/A	995	895	16	795	797	796	0	89%
	15	1058	875	1154	1029	14	798	791	795	1	77%
	10	351	311	464	375	21	783	792	788	1	210%
GC-50% RAP	12	412	369	304	362	15	779	784	781	0	216%
	15	370	424	421	405	8	793	789	791	0	195%
	10	254	281	309	281	10	614	633	623	2	222%
GC-75% RAP	12	307	376	321	335	11	590	543	566	6	169%
	15	363	495	376	412	18	796	773	785	2	191%
	10	552	571	615	579	6	262	258	260	1	45%
GD-25% RAP	12	610	693	635	646	7	240	253	246	4	38%
	15	662	445	630	579	20	333	320	326	3	56%
	10	363	342	532	412	25	168	190	179	9	44%
GD-50% RAP	12	671	605	537	604	11	184	242	213	19	35%
	15	428	510	643	527	21	268	243	255	7	48%
	10	223	312	293	276	17	117	107	112	6	41%
GD-75% RAP	12	345	342	304	330	7	124	127	125	2	38%
	15	513	381	543	479	18	185	199	192	5	40%
	10	561	534	580	559	4	636	552	594	10	106%
PC-25% RAP	12	682	652	607	647	6	641	625	633	2	98%
	15	844	955	1066	955	12	774	634	704	14	74%
	10	301	374	361	345	11	453	521	487	10	141%
PC-50% RAP	12	368	434	409	404	8	615	523	569	12	141%
	15	581	601	596	593	2	731	716	723	2	122%
	10	208	177	269	218	21	358	401	380	8	174%
PC-75% RAP	12	275	341	364	327	14	500	532	516	4	158%
	15	343	411	474	409	16	471	621	546	19	133%
	10	92	66	76	78	17	547	331	439	35	562%
PD-25% RAP	12	177	166	149	164	9	382	343	362	8	221%
	15	172	188	151	170	11	424	359	392	12	230%
	10	35	29	38	34	14	309	447	378	26	1113%
PD-50% RAP	12	61	71	52	61	15	295	298	296	1	482%
	15	154	156	166	159	4	406	274	340	27	214%
	10	37	40	40	39	5	327	384	356	11	915%
PD-75% RAP	12	63	56	64	61	7	277	444	360	33	594%
	15	65	71	53	63	14	272	416	344	30	547%

3.2.3 Moisture Sensitivity

As discussed earlier, the moisture sensitivity of the PC- and FA -stabilized FDR were evaluated using both the TST and Wire Brush Tests. However, the TST was conducted on all combinations of factors while the Wire Brush Test was only conducted on the selected optimum designs that were selected based on the compressive strength properties of the mixtures.

Earlier studies at the Texas Transportation Institute (TTI) on the TST showed a dielectric constant below 10 indicates good resistance to moisture damage as stated in the Texas DOT TST protocol dated 12/31/05. The data in Table 8 indicate the great majority of the PC- and FA -stabilized FDR materials exhibit TST values well below the threshold of 10.

The Wire Brush Tests were then conducted on the optimum design samples. Table 9 summarizes the average percent weight loss after 12 cycles. The full data set is shown in Appendix E. The weight loss data show that poor dirty materials stabilized with either PC or FA suffered the highest weight loss.

The relationships between weight loss and number of cycles indicate that the weight loss for most materials would level-off after the 6th cycle. This indicates that the full 12 cycles may not be needed to fully describe the moisture sensitivity of FDR materials stabilized with PC and FA and a shorter test which consists of just six cycles may be adequate.

 $\label{thm:constants} \textbf{Table 8: Tube suction results measured in dielectric constants for PC- and FA - stabilized FDR.}$

		San	nple				San	nple		
Material	Cement %	A	В	Avg.	CV	Fly Ash %	Α	В	Avg.	CV
	3	4	6	5	22	10	5	4	4	9
GC-25% RAP	5	6	3	5	48	12	5	5	5	3
	7	3	5	4	38	15	6	6	6	3
	3	5	3	4	32	10	6	4	5	28
GC-50% RAP	5	3	3	3	17	12	5	3	4	24
	7	5	4	5	11	15	6	6	6	6
	3	4	4	4	0	10	5	6	5	17
GC-75% RAP	5	3	3	3	3	12	5	4	5	4
	7	3	5	4	44	15	6	3	4	53
	3	8	5	6	39	10	11	10	11	7
GD-25% RAP	5	7	7	7	2	12	11	11	11	3
	7	5	6	6	8	15	9	8	8	6
	3	6	6	6	3	10	7	8	7	11
GD-50% RAP	5	6	5	5	17	12	14	8	11	34
	7	6	8	7	17	15	9	9	9	1
	3	8	10	9	16	10	8	8	8	6
GD-75% RAP	5	6	7	6	18	12	12	7	9	36
	7	5	7	6	21	15	6	8	7	29
	3	6	6	6	1	10	5	7	6	20
PC-25% RAP	5	8	7	8	16	12	4	5	5	16
	7	4	7	5	41	15	6	4	5	37
	3	8	8	8	2	10	6	8	7	21
PC-50% RAP	5	4	4	4	9	12	6	7	7	7
	7	5	4	4	10	15	3	4	4	14
	3	5	6	6	20	10	8	4	6	40
PC-75% RAP	5	7	7	7	7	12	6	5	6	12
	7	5	4	4	14	15	8	9	9	11
	3	7	5	6	26	10	5	8	6	34
PD-25% RAP	5	3	4	3	20	12	4	7	6	36
	7	5	3	4	34	15	6	8	7	21
	3	3	3	3	6	10	9	5	7	40
PD-50% RAP	5	4	5	4	11	12	7	7	7	2
	7	3	4	4	20	15	9	10	10	10
	3	4	6	5	21	10	5	5	5	2
PD-75% RAP	5	3	4	4	4	12	9	8	8	9
	7	4	4	4	4	15	7	11	9	34

Table 9: Average percent of weight loss from the Wire Brush Test.

Specimen ID	Average % loss
Good Clean 25% RAP 5% PC	0.7%
Good Clean 50% RAP 7% PC	0.6%
Good Clean 75% RAP 7% PC	0.7%
Good Clean 25% RAP 12% FA	12.4%
Good Clean 50% RAP 12% FA	failed sample
Good Clean 75% RAP 12% FA	9.6%
Good Dirty 25% RAP 3% PC	2.6%
Good Dirty 50% RAP 5% PC	1.4%
Good Dirty 75% RAP 7% PC	1.0%
Good Dirty 25% RAP 12% FA	4.7%
Good Dirty 50% RAP 10% FA	5.4%
Good Dirty 75% RAP 10% FA	4.1%
Poor Clean 25% RAP 3% PC	1.5%
Poor Clean 50% RAP 5% PC	0.6%
Poor Clean 75% RAP 5% PC	0.9%
Poor Clean 25% RAP 12% FA	2.1%
Poor Clean 50% RAP 12% FA	1.0%
Poor Clean 75% RAP 10% FA	1.6%
Poor Dirty 25% RAP 3% PC	11.1%
Poor Dirty 50% RAP 3% PC	12.9%
Poor Dirty 75% RAP 5% PC	1.5%
Poor Dirty 25% RAP 15% FA	6.7%
Poor Dirty 50% RAP 15% FA	7.5%
Poor Dirty 75% RAP 15% FA	12.5%

3.3 FDR Stabilized with Asphalt Emulsion (AE) and, Asphalt Emulsion plus Lime (AE+Lime) and, Foamed Asphalt plus 1% Portland Cement (Foamed Asphalt+1%PC), and Foamed Asphalt plus 2% Portland Cement (Foamed Asphalt+2%PC)

The FDR material stabilized with AE and Foamed Asphalt was combined in this section because the tests are very similar.

3.3.1 Moisture -Density Curves (MDC)

The optimum moisture and maximum dry density values used were from the mechanically stabilized values. These values were used because the addition of the emulsion and foamed asphalt made no difference on the moisture content of the mixtures. The actual values for maximum dry density and optimum MC are summarized in Table 2.

3.3.2 Indirect Tensile Strength (ITS)

The results of the ITS testing for the dry samples are presented in Tables 10 through 13. The highlighted samples are the optimums. The optimum design mixtures had a dry ITS strength above

30 psi. (0,2 MPa). It is of note that the AE and AE + 1% Lime samples had GD and PD optimums only. The same results are shown graphically in Figures 39 to 50.

Table 10: Results of the ITS tests for the AE specimens.

				NO LIME		
Material	Emulsion (%)	Dry Tensile Strength (psi)	COV (%)	Bulk Density (lb/ft ³)	Max Specific Gravity	Air Voids (%)
a 1 a	3	11	18	130.0	2.51	16.8
Good Clean	4.5	12	3	131.8	2.49	15.3
25 % RAP	6	16	19	135.9	2.42	10.2
G 1 G	3	14	27	129.6	2.48	16.1
Good Clean	4.5	17	2	132.5	2.43	12.7
50 % RAP	6	36	49	133.7	2.39	10.1
G 1.61	3	22	5	132.7	2.46	13.4
Good Clean	4.5	19	6	133.4	2.42	11.6
75 % RAP	6	20	6	134.1	2.37	9.3
C 1D: 4	3	42	7	138.3	2.56	13.5
Good Dirty 25 % RAP	4.5	41	9	138.9	2.54	12.4
25 % KAP	6	45	3	141.1	2.50	9.7
G 1D:	3	52	2	136.2	2.52	13.5
Good Dirty	4.5	47	1	135.9	2.49	12.5
50 % RAP	6	46	7	138.2	2.44	9.3
G 15'	3	42	10	137.2	2.47	10.9
Good Dirty 75 % RAP	4.5	46	9	138.3	2.41	8.0
13 % KAP	6	42	6	139.3	2.51	11.2
Poor Clean	3	23	15	139.0	2.38	6.3
25 % RAP	4.5	18	7	138.0	2.54	12.9
23 70 KAF	6	15	7	137.3	2.43	9.5
Poor Clean	3	24	13	137.8	2.47	10.5
50 % RAP	4.5	18	4	136.5	2.47	11.4
30 % KAP	6	18	3	135.5	2.37	8.2
Poor Clean	3	18	10	131.4	2.43	13.3
75 % RAP	4.5	18	7	133.3	2.41	11.4
73 70 KAF	6	17	5	134.7	2.41	10.4
Door Diety	3	30	8	137.5	2.49	11.5
Poor Dirty 25 % RAP	4.5	30	6	135.2	2.45	11.7
23 70 KAF	6	33	5	133.7	2.47	13.1
Poor Dinty	3	52	5	137.0	2.53	13.4
Poor Dirty 50 % RAP	4.5	50	7	133.9	2.48	13.4
30 % KAF	6	39	3	131.9	2.44	13.5
Poor Dinty	3	56	6	135.1	2.46	11.8
Poor Dirty 75 % RAP	4.5	51	3	133.2	2.43	12.0
75 /0 KAI	6	44	6	132.2	2.41	12.1

Table 11: Results of the ITS tests for the AE+1%lime specimens.

				1% LIME		
Material	Emulsion (%)	Dry Tensile Strength (psi)	COV (%)	Bulk Density (lb/ft³)	Max Specific Gravity	Air Voids (%)
Cood Cloor	3	11	25	131.1	2.56	18.0
Good Clean 25 % RAP	4.5	16	18	133.5	2.55	16.1
23 % KAP	6	19	13	137.3	2.53	13.1
Good Clean	3	18	9	136.2	2.52	13.3
50 % RAP	4.5	20	14	132.7	2.50	15.0
30 % KAP	6	19	8	134.8	2.44	11.7
C 1 C1	3	20	11	132.1	2.47	14.2
Good Clean 75 % RAP	4.5	24	12	134.6	2.46	12.4
/3 % KAP	6	23	9	133.4	2.44	12.3
Cood Distri	3	34	9	138.5	2.54	12.6
Good Dirty	4.5	45	7	139.5	2.62	14.7
25 % RAP	6	52	2	140.8	3.29	31.5
C 1 D:	3	25	29	132.4	2.48	14.5
Good Dirty	4.5	37	5	137.5	2.47	10.8
50 % RAP	6	52	5	138.0	2.44	9.2
G 1D: (3	38	5	134.1	2.47	13.0
Good Dirty	4.5	44	6	134.0	2.46	12.7
75 % RAP	6	44	2	134.5	2.40	10.2
Door Cloon	3	17	11	139.8	2.54	11.9
Poor Clean 25 % RAP	4.5	17	4	139.9	2.50	10.2
23 % KAP	6	17	7	139.8	2.56	12.4
Door Cloor	3	18	10	137.6	2.40	8.0
Poor Clean 50 % RAP	4.5	19	2	137.9	2.48	10.7
30 % KAP	6	18	20	137.6	2.45	9.9
D Cl	3	14	8	134.2	2.18	1.4
Poor Clean	4.5	18	15	135.1	2.19	1.3
75 % RAP	6	14	6	135.1	2.18	0.8
Da a v Dista	3	22	7	136.9	2.55	14.0
Poor Dirty	4.5	22	4	135.1	2.51	13.8
25 % RAP	6	25	6	133.8	2.50	14.3
Door Dist	3	32	3	136.5	2.52	13.1
Poor Dirty	4.5	38	4	134.7	2.49	13.3
50 % RAP	6	28	7	133.3	2.46	13.0
D D' /	3	35	9	133.9	2.41	11.1
Poor Dirty	4.5	34	6	133.5	2.41	11.4
75 % RAP	6	27	34	132.4	2.45	13.3

Table 12: Results of the ITS tests for the Foamed Asphalt+1%PC specimens.

			1%	Portland Cen	nent	
Material	Foamed Asphalt (%)	Dry Tensile Strength (psi)	COV (%)	Bulk Density	Max Specific	Air Voids
	, ,	10	` '	(lb/ft ³)	Gravity	(%)
Good Clean	2.5	19	9	123.7	2.44	18.9
25 % RAP	3	28	14 5	125.3	2.50	19.8
	3.5	33		125.4	2.46	18.2
Good Clean	2.5	30	3 2	125.3	2.27	11.6
50 % RAP	3	32		125.0	2.25	11.0
	3.5	30	11	124.7	2.45	18.3
Good Clean	2.5	28	9	123.6	2.45	19.1
75 % RAP	3	34	10	124.5	2.36	15.6
	3.5	39	6	125.5	2.41	16.6
Good Dirty	2.5	33	12	129.5	2.51	17.4
25 % RAP	3	38	6	129.6	2.50	16.8
20 /0 1011	3.5	45	6	130.2	2.46	15.2
Good Dirty	2.5	41	6	127.4	2.45	16.7
50 % RAP	3	44	4	127.7	2.46	16.8
30 /0 K/11	3.5	44	3	129.2	2.41	14.0
Good Dietry	2.5	41	9	127.8	2.36	13.1
Good Dirty 75 % RAP	3	46	5	128.6	2.33	11.7
13 % KAF	3.5	51	6	128.2	2.40	14.3
D Cl	2.5	44	3	132.8	2.74	22.4
Poor Clean	3	45	5	133.5	2.34	8.7
25 % RAP	3.5	54	4	133.9	2.59	17.2
D CI	2.5	44	2	132.5	2.47	13.9
Poor Clean	3	49	9	133.0	2.29	7.1
50 % RAP	3.5	53	4	131.0	2.42	13.4
D C1	2.5	43	3	131.9	2.35	10.2
Poor Clean	3	48	6	132.7	2.30	7.6
75 % RAP	3.5	48	5	131.4	2.41	12.6
D D1	2.5	41	9	133.8	2.50	14.4
Poor Dirty	3	43	3	133.5	2.46	13.0
25 % RAP	3.5	34	9	132.9	2.51	15.1
_	2.5	43	5	132.2	2.45	13.5
Poor Dirty	3	48	5	132.6	2.36	9.9
50 % RAP	3.5	47	11	132.4	2.45	13.4
	2.5	53	6	132.8	2.42	11.9
Poor Dirty	3	55	2	129.9	2.20	5.4
75 % RAP	3.5	53	4	131.5	2.39	11.8

Table 13: Results of the ITS tests for the Foamed Asphalt+2%PC specimens.

	D1		2%	Portland Cen	nent	
Material	Foamed Asphalt (%)	Dry Tensile Strength (psi)	COV (%)	Bulk Density (lb/ft ³)	Max Specific Gravity	Air Voids (%)
G 1.61	2.5	38	11	128.7	2.50	17.5
Good Clean	3	53	4	130.9	2.49	15.8
25 % RAP	3.5	63	4	132.1	2.46	13.9
C 1.Cl	2.5	47	7	128.2	2.45	16.0
Good Clean	3	51	3	128.5	2.43	15.3
50 % RAP	3.5	53	10	129.0	2.41	14.1
C 1.Cl	2.5	54	5	128.3	2.42	15.2
Good Clean	3	58	6	128.8	2.41	14.2
75 % RAP	3.5	59	8	130.1	2.39	12.8
C ID:	2.5	46	11	134.0	2.52	14.7
Good Dirty	3	56	8	133.5	2.44	12.4
25 % RAP	3.5	59	3	131.6	2.46	14.2
C 1D: (2.5	N/A	N/A	N/A	N/A	N/A
Good Dirty	3	N/A	N/A	N/A	N/A	N/A
50 % RAP	3.5	N/A	N/A	N/A	N/A	N/A
C 1D: (2.5	N/A	N/A	N/A	N/A	N/A
Good Dirty	3	N/A	N/A	N/A	N/A	N/A
75 % RAP	3.5	N/A	N/A	N/A	N/A	N/A
D Cl	2.5	44	13	130.7	2.36	11.0
Poor Clean 25 % RAP	3	46	0	130.4	2.48	15.6
23 % KAP	3.5	50	2	131.0	2.46	14.8
Poor Clean	2.5	43	8	129.4	2.47	15.9
50 % RAP	3	47	5	129.8	2.44	14.7
30 % KAP	3.5	52	8	130.2	2.39	12.7
Door Cloor	2.5	48	9	130.9	2.43	13.9
Poor Clean 75 % RAP	3	56	2	131.1	2.41	12.8
13 % KAP	3.5	59	4	132.2	2.41	11.9
Door Dinter	2.5	41	8	130.6	2.52	17.0
Poor Dirty 25 % RAP	3	43	9	130.0	2.49	16.4
25 % KAP	3.5	48	9	129.9	2.48	15.9
D D:	2.5	N/A	N/A	N/A	N/A	N/A
Poor Dirty 50 % RAP	3	N/A	N/A	N/A	N/A	N/A
JU % KAP	3.5	N/A	N/A	N/A	N/A	N/A
Down Dire	2.5	N/A	N/A	N/A	N/A	N/A
Poor Dirty	3	N/A	N/A	N/A	N/A	N/A
75 % RAP	3.5	N/A	N/A	N/A	N/A	N/A

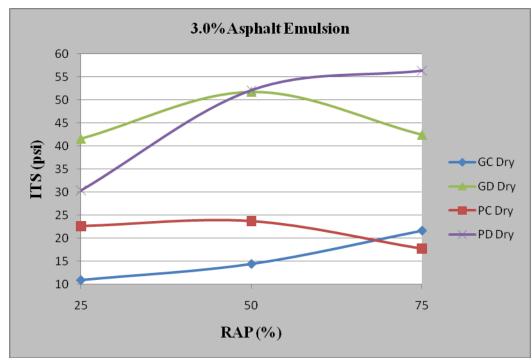


Figure 39: 3.0% AE dry material results.

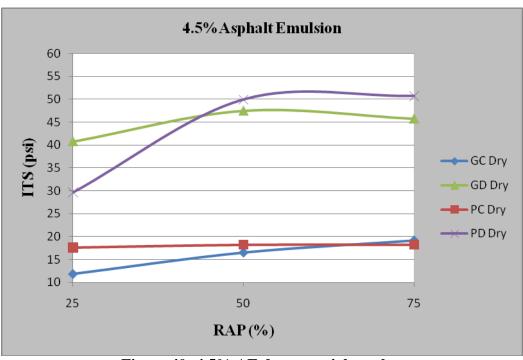


Figure 40: 4.5% AE dry material results.

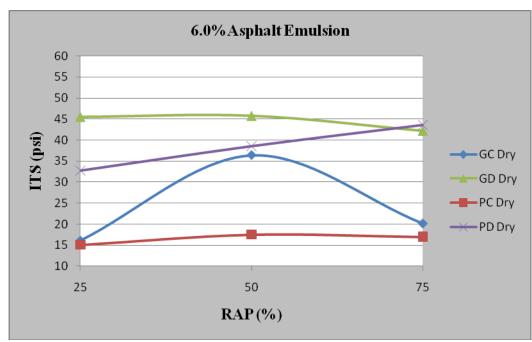


Figure 41: 6.0% AE dry material results.

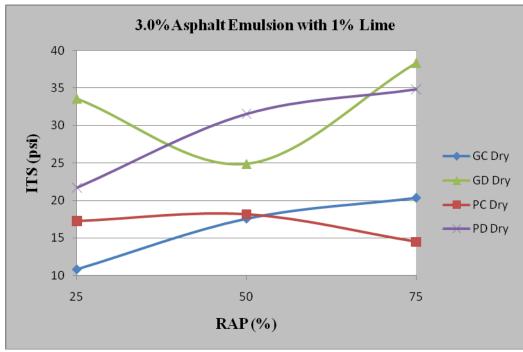


Figure 42: 3.0% AE + 1% Lime material results.

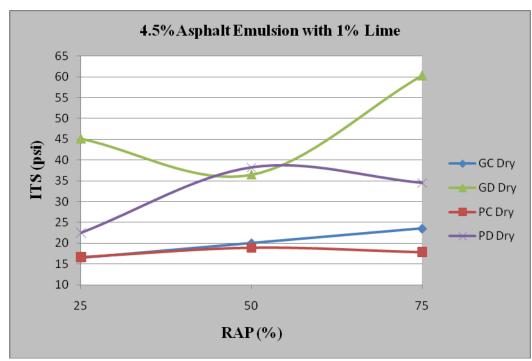


Figure 43: 4.5% AE + 1% Lime material results.

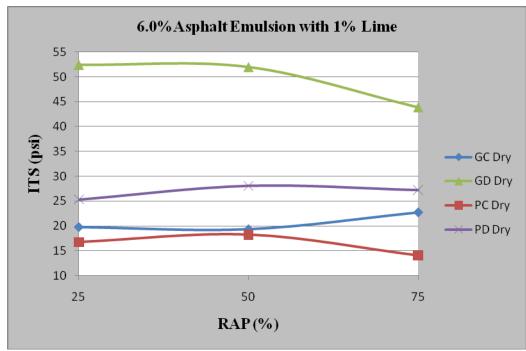


Figure 44: 6.0% AE + 1% Lime material results.

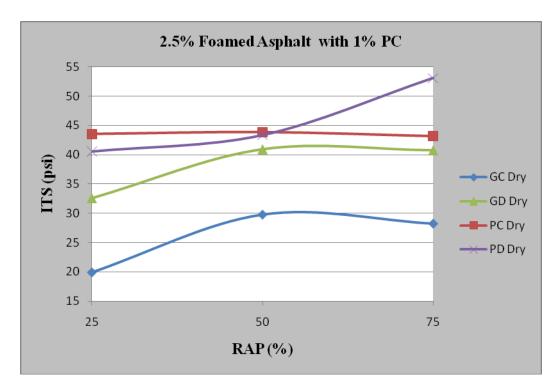


Figure 45: 2.5% Foamed Asphalt+1% PC material results.

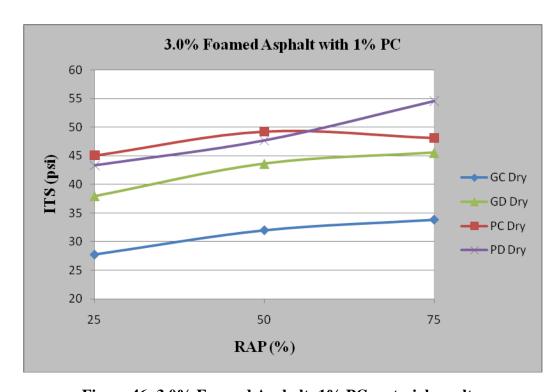


Figure 46: 3.0% Foamed Asphalt+1% PC material results.

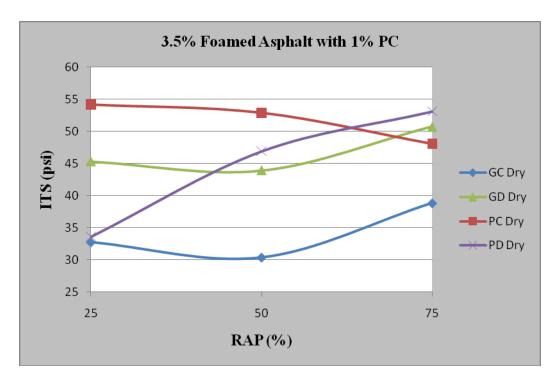


Figure 47: 3.5% Foamed Asphalt +1% PC material results.

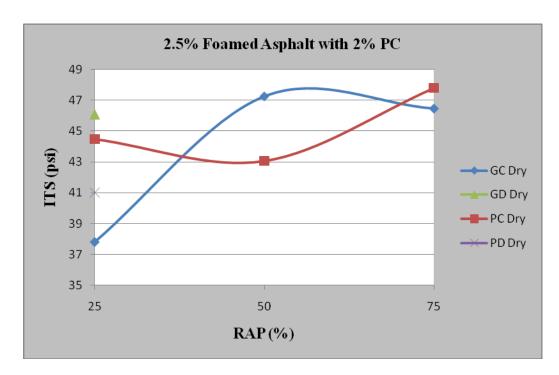


Figure 48: 2.5% Foamed Asphalt +2% PC material results.

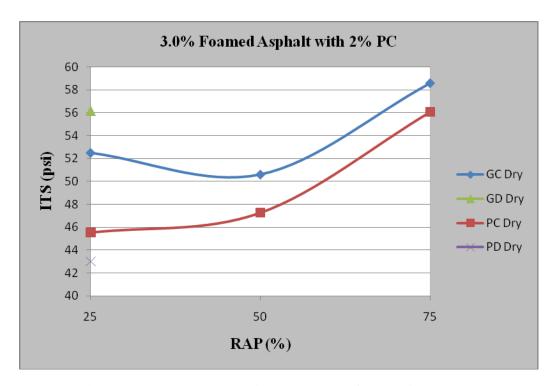


Figure 49: 3.0% Foamed Asphalt+2% PC material results.

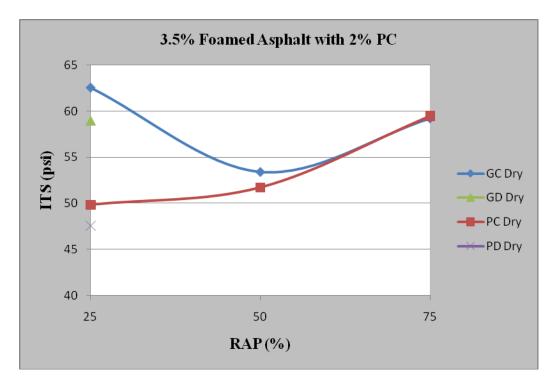


Figure 50: 3.5% Foamed Asphalt +2% PC material results.

3.3.3 Moisture Conditioning

The results of the AASHTO T283 freeze-thaw moisture conditioning test for the AE, AE+1% Lime, Foamed Asphalt+1% PC, and Foamed Asphalt+2% PC samples are presented in Tables 14 and 15. These tests were only performed on the optimum designs that were selected based on the dry ITS criterion. The results are shown graphically in Figures 51 and 52.

Table 14: AASHTO T283 results after one freeze-thaw cycle for AE and AE+1% Lime.

Sample	A	В	С	D	Е	Mean (psi)	STD	COV	Average air voids (%)
GD 25% R 4.5% E	14	16	16	14	17	15	1.2	7%	14
GD 50% R 4.5% E	19	24	17	19	21	20	2.2	11%	13
GD 75% R 4.5% E	24	26	17	17	21	21	3.6	17%	12
PD 25% R 4.5% E	6	7	6	8	6	7	0.6	9%	13
PD 50% R 4.5% E	6	8	8	8	9	8	0.9	12%	14
PD 75% R 4.5% E	9	8	9	7	9	8	0.8	10%	13
					wi	th 1% Lime			
GD 25% R 4.5% E	26	27	28	30	22	26	2.7	10%	17
GD 50% R 4.5% E	33	32	30	27	37	32	3.2	10%	12
GD 75% R 4.5% E	33	33	28	29	31	31	2.0	7%	13
PD 25% R 4.5% E	14	13	11	14	11	13	1.6	12%	15
PD 50% R 4.5% E	15	19	17	21	15	17	2.6	15%	13
PD 75% R 4.5% E	24	18	19	17	18	19	2.5	13%	12

Table 15: AASHTO T-283 freeze -thaw results after one cycle for Foamed Asphalt+PC.

Sample	A	В	С	D	Е	Mean (psi)	STD	COV	Average air voids (%)
GC 25%R 1% PC 3.5% Foamed Asphalt	14	14	15	15	N/A	14	0.6	4%	18
GC 50%R 1% PC 3.5% Foamed Asphalt	11	10	10	11	N/A	10	0.7	6%	23
GC 75%R 1% PC 3.5% Foamed Asphalt	10	11	9	12	N/A	10	1.1	10%	21
GC 25%R 2% PC 3% Foamed Asphalt	44	45	44	37	42	42	2.8	6%	14
GC 50%R 2% PC 3% Foamed Asphalt	43	39	38	44	42	41	2.4	6%	12
GC 75%R 2% PC 3% Foamed Asphalt	45	48	47	39	45	45	3.0	7%	12
GD 25%R 1% PC 3.5% Foamed Asphalt	35	31	33	36	34	34	1.7	5%	13
GD 50%R 1% PC 3.5% Foamed Asphalt	47	42	43	42	42	43	1.9	4%	12
GD 75%R 1% PC 3.5% Foamed Asphalt	41	42	41	46	41	42	1.8	4%	13
GD 25%R 2% PC 3.0% Foamed Asphalt	18	18	22	22	N/A	20	1.9	10%	15
PC 25%R 1% PC 3.5% Foamed Asphalt	35	32	31	31	29	32	1.8	6%	11
PC 50%R 1% PC 3.5% Foamed Asphalt	44	62	35	32	48	44	10.7	24%	11
PC 75%R 1% PC 3.5% Foamed Asphalt	30	34	35	33	32	33	1.7	5%	13
PC 25%R 2% PC 3.0% Foamed Asphalt	40	43	39	37	41	40	1.8	5%	14
PC 50%R 2% PC 3.0% Foamed Asphalt	19	17	17	17	N/A	18	0.9	5%	18
PC 75%R 2% PC 3.0% Foamed Asphalt	8	8	9	9	N/A	9	0.5	6%	25
PD 25%R 1% PC 3.0% Foamed Asphalt	23	22	24	30	30	26	3.4	13%	12
PD 50%R 1% PC 3.0% Foamed Asphalt	33	29	25	30	27	29	2.8	10%	12
PD 75%R 1% PC 3.0% Foamed Asphalt	34	36	30	37	36	35	2.4	7%	12
PD 25%R 2% PC 3.0% Foamed Asphalt	20	16	20	17	N/A	18	1.6	9%	15

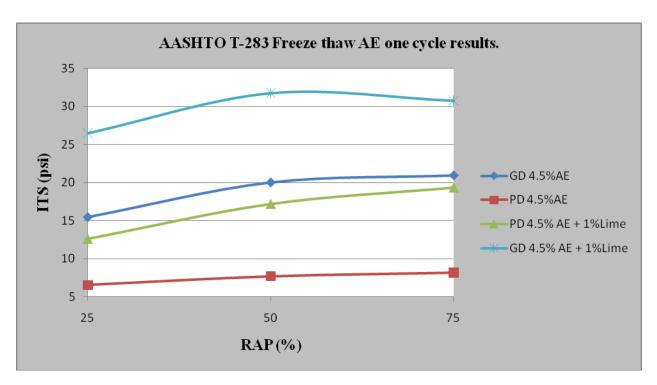


Figure 51: AASHTO T283 Freeze-thaw AE one cycle results.

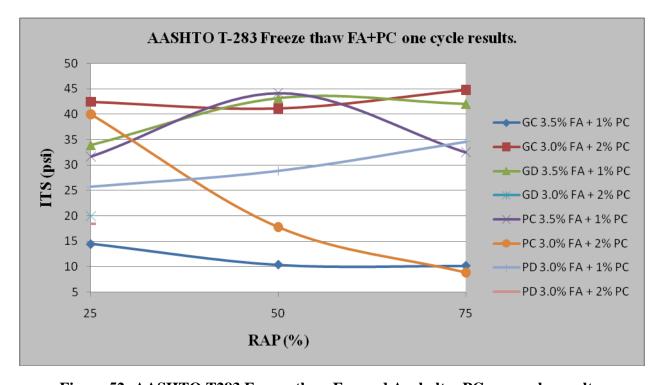


Figure 52: AASHTO T283 Freeze-thaw Foamed Asphalt + PC one cycle results.

4. OPTIMUM DESIGNS

4.1 FDR Stabilized with Portland Cement and Fly Ash

The optimum designs for the PC- and FA -stabilized FDR materials were selected based on the following criteria:

• UCS: 300 – 500 psi (2.1 – 3.4 MPa)

• TST: < 9

Tables 16 and 17 summarize the selected optimum mix designs for PC- and FA -stabilized FDR materials. The data show few designs do not meet the compressive strength range of 300 - 500 psi (2.1 - 3.4 MPa). The bolded values in Tables 16 and 17 are the optimums as established by the design parameters.

4.2 FDR Stabilized with Emulsion and Foamed Asphalt

The optimum designs for both the emulsion and foamed asphalt stabilized FDR were selected based on the following criteria:

- Minimum dry tensile strength at 77°F (25°C): of 40 psi (0.28 MPa).
- A minimum TSR of 70%.

Table 18 highlights the optimum designs for the AE -stabilized FDR materials with and without lime. It can be seen from the data in Table 18 that all of the mixtures involving good clean and poor clean materials did not achieve the minimum TSR value of 70%. Based on these observations, all of the emulsion -stabilized FDR samples were re-evaluated with 1% lime. The addition of 1% lime significantly improved the ITS after one freeze-thaw cycle testing and the resulting TSR but only two materials passed the TSR minimum of 70%. In addition, the 1% lime showed reduction in the air voids which could lead to direct improvements in the long term durability of the FDR materials. These are shown graphically in Figures 54 and 55.

Initial attempts to produce optimum designs for foamed asphalt -stabilized FDR were un-successful. The next step was to include 1% PC with the foamed asphalt -stabilized FDR. Table 19 summarizes the optimum designs for the foamed asphalt with 1% and 2% PC -stabilized FDR. The data in Table 19 indicates that no optimum designs were achieved for the good clean materials. It is also of note that several of the optimum designs did not achieve the minimum TSR. These are represented graphically in Figures 55 to 58.

Table 16: Optimum designs for PC -stabilized FDR.

Material	%PC	MC _{opt}	Density (pcf)	Dry UCS	CV	Tube Suction	Optimum Design
	3	(%) 9.8	128	(psi) 176*	(%) 2	5	Design
GC-25%	5	10.3	128	283	8	5	√
	7	9.2	130	257	7	4	V
	3	9.2	125	138	11	4	
GC-50%	5	8.6	126	309	24	3	
GC-3070	7	8.8	128	407	15	5	√
	3	10.3	121	185	10	4	Y
GC-75%	5	8.3	122	237	14	3	
GC 7570	7	9	125	409	11	4	√
	3	7.7	135	352	13	6	√
GD-25%	5	7.6	134	555	4	7	,
GD 2370	7	7.8	133	915	21	6	
	3	8.6	128	230	3	6	
GD-50%	5	8.6	128	413	11	5	√
02 5070	7	7.9	131	1093	4	7	,
	3	8.9	123	145	17	9	
GD-75%	5	9.2	122	219	12	6	
	7	8.6	126	374	5	6	√
	3	6	139	295	5	6	√
PC-25%	5	5.3	136	589	3	8	
	7	5	141	853	14	5	
	3	7.8	129	158	16	8	
PC-50%	5	5.1	134	379	18	4	√
	7	6.1	136	582	9	4	
	3	6.9	131	156	21	6	
PC-75%	5	7	130	256	10	7	√
	7	6.3	130	520	14	4	
	3	5.5	137	454	6	6	√
PD-25%	5	6.1	135	793	16	3	
	7	6.1	135	910	18	4	
	3	5.8	136	421	13	3	√
PD-50%	5	6.3	134	661	9	4	
	7	6.3	134	730	8	4	
	3	7.8	129	254	4	5	
PD-75%	5	7.3	129	409	13	4	√
	7	7.3	129	625	3	4	

Table 17: Optimum designs for FA -stabilized FDR.

Material	%FA	MC _{opt}	Density	Dry UCS	CV	Tube Suction	Optimum
	10	(%) 7.2	(pcf)	(psi) 1533	(%) 11	5 Suction 5	Design
GC-25%	12	7.3	131	895	16	5	√
	15	6.6	133	1029	14	6	Υ
	10	7.5	129	375	21	5	
GC-50%	12	7.5	130	362	15	4	✓
GC-3070	15	7.5	130	405	8	6	ν
	10	7.3	128	281	10	5	
GC-75%	12	7.3	128	335	11	<u> </u>	√
GC-1370	15	6.7	129	412	18	4	V
	10	7.6	130	579	6	11	√
GD-25%	12	7.2	130	646	7	11	,
GD 2570	15	7.1	130	579	20	8	
	10	7.1	128	412	25	7	√
GD-50%	12	7.3	130	604	11	11	,
GD 3070	15	6.8	129	527	21	9	
	10	7.6	129	276	17	8	
GD-75%	12	7.4	126	330	7	9	√
02 7070	15	6.8	129	479	18	7	,
	10	5.9	127	558	4	6	√
PC-25%	12	5.2	123	647	6	5	
	15	6.5	131	955	12	5	
	10	6.7	127	345	11	7	
PC-50%	12	7.1	125	404	8	7	√
	15	7.9	127	593	2	4	
	10	8.1	124	218	21	6	
PC-75%	12	6.6	125	327	14	6	√
	15	8.1	124	409	16	9	
	10	6.5	135	78	17	6	
PD-25%	12	5.2	137	164	9	6	
	15	5.2	136	170	11	7	√
PD-50%	10	7.3	129	34	14	7	
	12	5.8	135	61	15	7	
	15	4.5	137	159	4	10	√
	10	7.4	127	39	5	5	
PD-75%	12	6.3	131	61	7	8	
	15	6.5	133	63	14	9	√

Table 18: Optimum designs for AE- and AE+1%Lime-stabilized FDR.

Asphalt Emulsion									
Material	ITS dry	ITS dry +1% Lime	ITS after Freeze-Thaw	ITS after Freeze- Thaw +1% Lime	TSR	TSR +1%Lime			
GD-25%	41	45	15	26	38%	59%			
GD-50%	47	37	20	32	43%	86%			
GD-75%	46	44	21	31	45%	70%			
PD-25%	30	22	7	13	22%	57%			
PD-50%	50	38	8	17	15%	45%			
PD-75%	51	34	8	19	16%	57%			

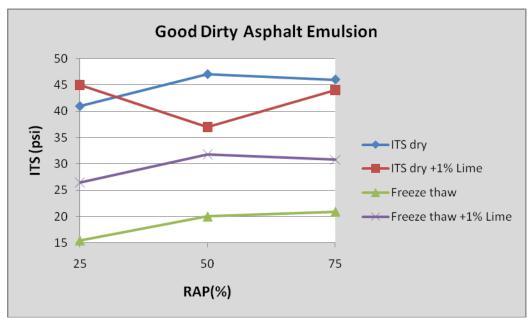


Figure 53: Good dirty AE and AE+1% Lime material results.

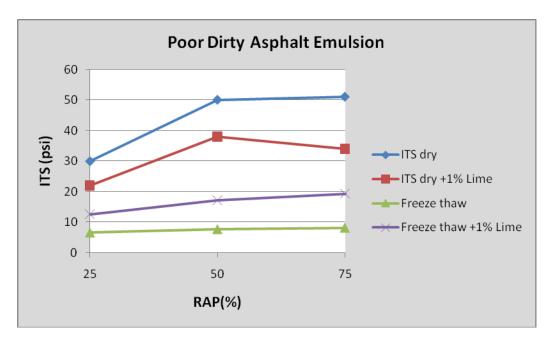


Figure 54: Poor dirty AE and AE+1% Lime material results.

Table 19: Optimum designs for Foamed Asphalt+1%PC- and Foamed Asphalt+2%PC - stabilized FDR.

Foamed Asphalt								
	ITS dry	ITS dry	ITS after Freeze	ITS after Freeze	TSR	TSR		
Material	+1% PC	+2% PC	thaw +1% PC	thaw +2% PC	+1% PC	+2%PC		
GC-25%	33	53	14	42	44%	80%		
GC-50%	30	51	10	41	35%	81%		
GC-75%	39	58	10	45	26%	77%		
GD-25%	45	56	34	20	75%	36%		
GD-50%	44	N/A	43	N/A	98%	N/A		
GD-75%	51	N/A	42	N/A	82%	N/A		
PC-25%	54	46	32	40	59%	87%		
PC-50%	53	47	44	18	83%	38%		
PC-75%	48	56	33	9	68%	16%		
PD-25%	43	43	26	18	60%	43%		
PD-50%	48	N/A	29	N/A	60%	N/A		
PD-75%	55	N/A	35	N/A	63%	N/A		

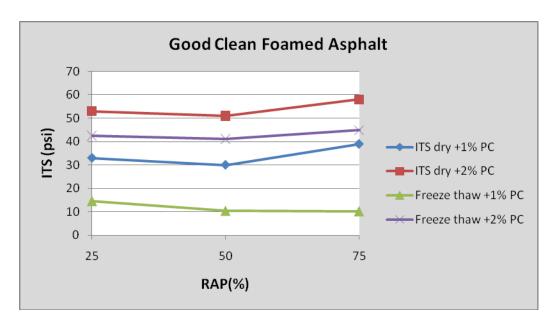


Figure 55: Good clean Foamed Asphalt+1%PC and +2%PC material results.

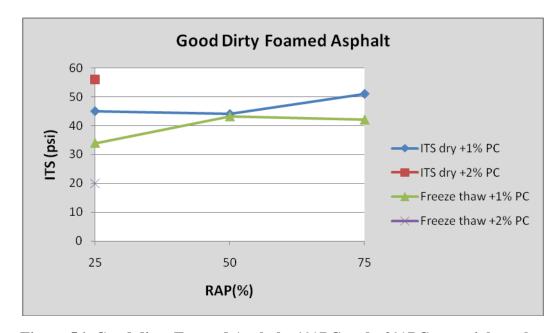


Figure 56: Good dirty Foamed Asphalt+1%PC and +2%PC material results.

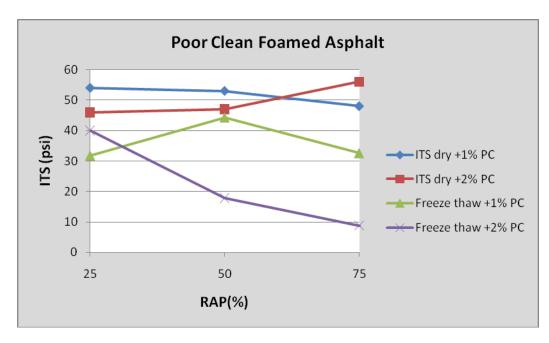


Figure 57: Poor clean Foamed Asphalt+1%PC and +2%PC material results.

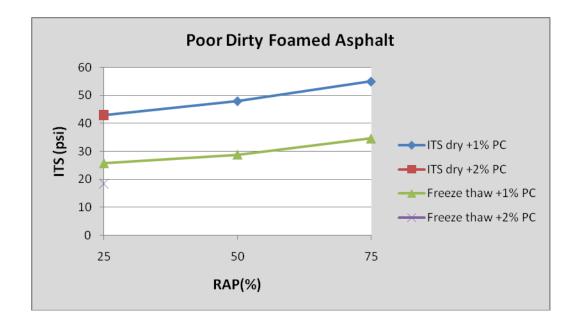


Figure 58: Poor dirty Foamed Asphalt+1%PC and +2%PC material results.

5. SUMMARY AND RECOMMENDATIONS

The objective of this task is to develop a mix design procedure for FDR. Due to the large variations among the various types of FDR stabilizers, a separate mix design method was developed for each type of FDR. The following represents a summary of the processes that were used in the mix design for each FDR type.

5.1 Unstabilized FDR

The following steps were used in the mix design for unstabilized FDR:

- Moisture-Density Curve: the modified Proctor test (ASTM D1557) was used to establish the
 moisture-density relationship for each combination of the unstabilized FDR. The optimum
 MC and maximum dry density values were identified from the developed Moisture Density
 Curves. The GC gradation was more difficult to control than the other materials due to its
 low fines content. No modifications were necessary for the ASTM D1557 when used with
 the unstabilized FDR with various levels of RAP.
- Strength Parameter: the M_R as described in AASHTO T307 and the CBR as described in ASTM D1883 were used to measure the strength properties of the unstabilized FDR with various levels of RAP. The correlations between the M_R and CBR values were very weak which led to the recommendation to use the M_R as the strength property for the unstabilized FDR. No modifications were necessary for AASHTO T307 and ASTM D1883 when used with the unstabilized FDR with various levels of RAP.
- Table 4 summarizes the overall properties of the unstabilized FDR mixtures that were evaluated in this research. No mix design criteria were necessary for the unstabilized FDR. The M_R properties of the various mixtures seem to be reasonable for a base course layer in a flexible pavement.

5.2 FDR Stabilized with PC and FA

The following steps were used in the mix design for FDR stabilized with PC and FA:

- Moisture Density Curve: the same Moisture Density Curves that were developed for the
 unstabilized FDR were also used for the FDR stabilized with PC and FA. In other words,
 the same optimum MC and maximum dry density values for each combination of
 unstabilized FDR and RAP materials were used for the FDR stabilized with PC and FA at
 the similar RAP contents.
- Strength Parameter: UCS as specified in ASTM D1633 was used to measure the strength properties of the FDR stabilized with PC and FA. The UCS samples were fabricated following ASTM D1633 and ASTM C593 for FDR stabilized with PC and FA, respectively. No modifications were necessary for ASTM D1633 and C593 standards when used with the FDR stabilized with PC and FA with various levels of RAP. The degree of variability of the measured compressive strength based on three replicate samples was acceptable.

- Moisture Sensitivity: the TST and the Wire Brush Test as described in ASTM D559 were used to measure the moisture sensitivity of the FDR stabilized with PC and FA. The TST is a research tool and does not have any AASHTO or ASTM designations. One modification was necessary for the ASTM D559 test which dealt with the removal of materials finer than the #4 (4.75 mm) sieve. This step was omitted since it would have resulted in the removal of a large portion of the material. The Tube Suction Test is a research tool and does not have any AASHTO nor ASTM designations.
- The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with PC and FA:

o Dry UCS: 300 – 500 psi (2.1 – 3.4 MPa)

o TST 14-day dielectric constant: max. 9

Tables 16 and 17 summarize the optimum mix designs for the FDR stabilized with PC and FA, respectively.

5.3 FDR Stabilized with Asphalt Emulsion (AE) and Asphalt Emulsion plus Lime (AE+lime)

The following steps were used in the mix design for FDR stabilized with AE and AE+lime:

- Moisture Density Curve: the same Moisture Density Curves that were developed for the
 unstabilized FDR were also used for the FDR stabilized with AE and AE+lime. In other
 words, the same optimum MC and maximum dry density values for each combination of
 unstabilized FDR and RAP materials were used for the FDR stabilized with AE and
 AE+lime at the similar RAP contents.
- Strength Parameter: the dry ITS at 77°F (25°C) as specified in AASHTO T283 was used to measure the strength properties of the FDR stabilized with AE and AE+lime. The samples were compacted in the SGC as specified in AASHTO T312. Three major modifications were made to the compaction process in the SGC: 1) use of a slotted mold to allow water to escape during the compaction process, 2) the samples were compacted to 40 gyrations, and 3) the sensor that counts the number of gyrations in the SGC was placed inside the chamber adjacent to the mold carriage to avoid being damaged by the water seeping out of the sample during the compaction process.
- Moisture Sensitivity: the TSR after one freeze-thaw cycle as described in AASHTO T283
 was used to evaluate the moisture sensitivity of the FDR stabilized with AE and AE+lime.
 The samples for the moisture conditioned ITS were compacted in the SGC following the
 same modified procedure used for the dry ITS samples.

• The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with AE and AE+lime:

o Dry ITS at 77°F (25°C): min. 30 psi (0.2 MPa)

o TSR at 77° F (25°C): 70%

Tables 18 and 19 summarize the optimum mix designs for the FDR stabilized with AE and AE+lime, respectively.

5.4 FDR Stabilized with Foamed Asphalt plus Portland Cement (PC)

The following steps were used in the mix design for FDR stabilized with foamed asphalt + PC:

- Moisture Density Curve: the same Moisture Density Curves that were developed for the
 unstabilized FDR were also used for the FDR stabilized with foamed asphalt + PC. In other
 words, the same optimum MC and maximum dry density values for each combination of
 unstabilized FDR and RAP materials were used for the FDR stabilized with foamed asphalt
 + PC at the similar RAP contents.
- Strength Parameter: the dry ITS at 77°F (25°C) as specified in AASHTO T283 was used to measure the strength properties of the FDR stabilized with foamed asphalt + PC. The samples were compacted in the SGC as specified in AASHTO T312. Three major modifications were made to the compaction process in the SGC: 1) use of a slotted mold to allow water to escape during the compaction process, 2) the samples were compacted to 40 gyrations, and 3) the sensor that counts the number of gyrations in the SGC was placed inside the chamber adjacent to the mold carriage to avoid being damaged by the water seeping out of the sample during the compaction process.
- Moisture Sensitivity: the TSR after one freeze-thaw cycle as described in AASHTO T283
 was used to evaluate the moisture sensitivity of the FDR stabilized with foamed asphalt +
 PC. The samples for the moisture conditioned ITS were compacted in the SGC following
 the same modified procedure used for the dry ITS samples.
- The following criteria were used in the selection of the optimum mix designs for the FDR stabilized with foamed asphalt + PC:

o Dry ITS at 77°F (25°C): min. 30 psi (0.2 MPa)

o TSR at 77° F (25°C): 70%

Tables 20 and 21 summarize the optimum mix designs for the FDR stabilized with foamed asphalt + 1 % PC and foamed asphalt + 2 % PC, respectively.

6. REFERENCES

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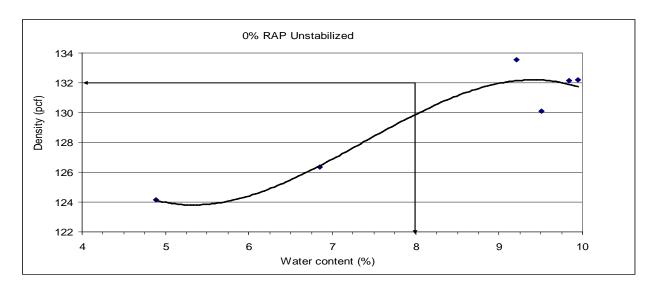
7. ACKNOWLEDGEMENTS

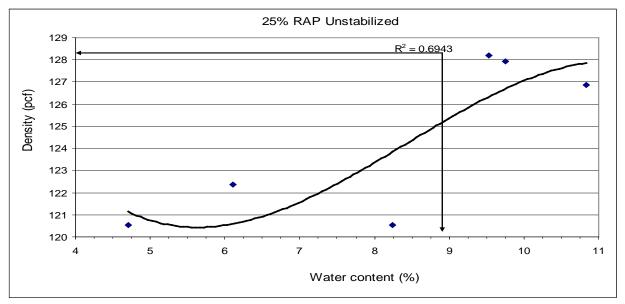
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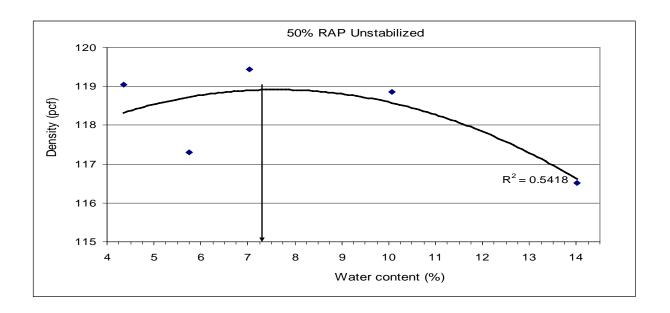
Randy Battey, Mississippi DOT Todd Casey, Base Construction Co. (ARRA) John Epps, Granite Construction, Inc. Joe Feller, SDDOT Gary Goff, FHWA ND Division David Gress, Univ. of New Hampshire Gregory Halsted, PCA (ARRA) Brett Hestdalen, FHWA SD Division John Huffman, Terex Roadbuilding (ARRA) Tim Kowalski, Wirtgen America David Lee, Univ. of Iowa Chuck Luedders, FHWA Direct Federal Lands Ken Skorseth, SDSU Ken Swedeen, Dakota Asphalt Pavement Association Todd Thomas, SemMaterials (ARRA) Mike Voth, Central Federal Lands Division, FHWA

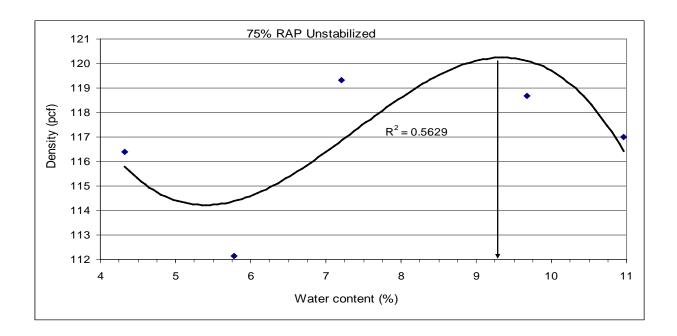
Appendix A Moisture -Density Curves

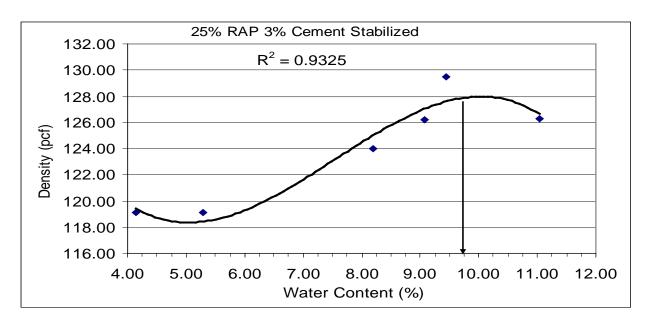
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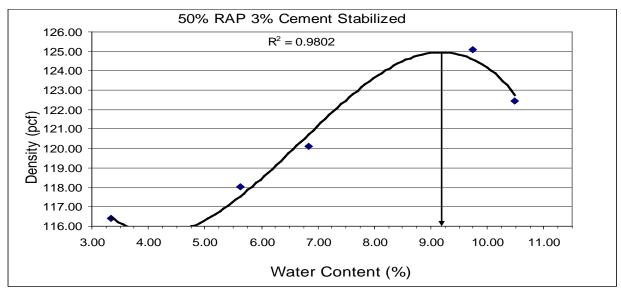


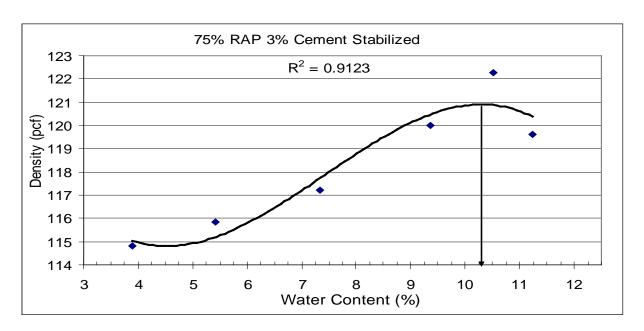


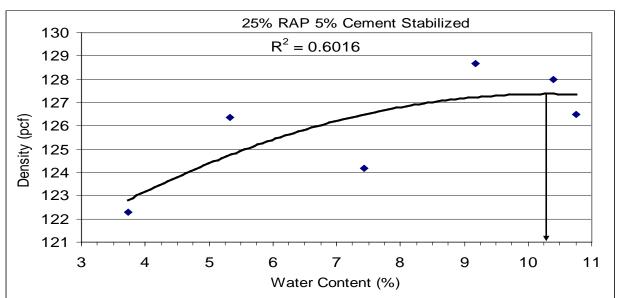


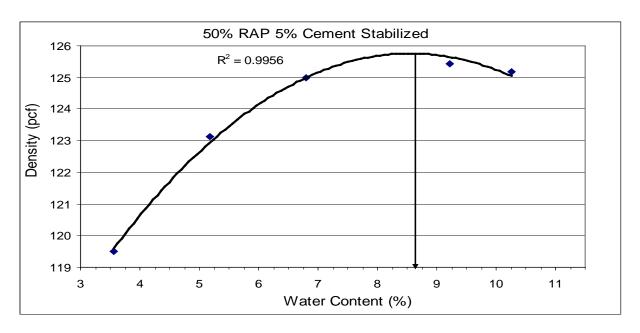


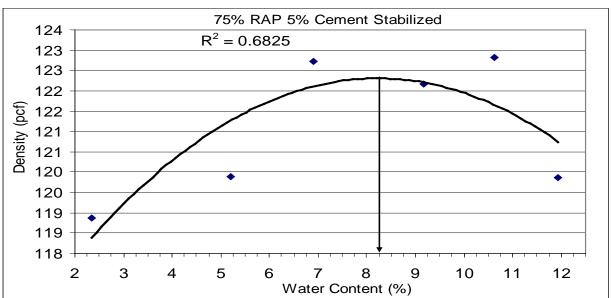


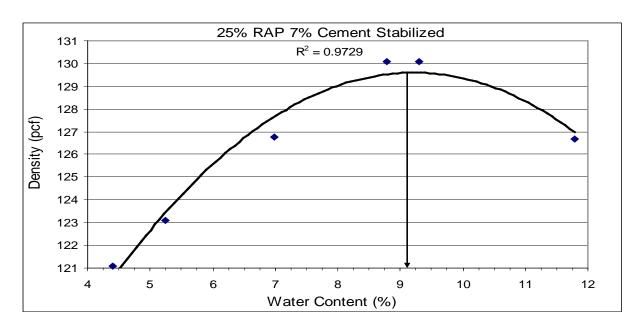


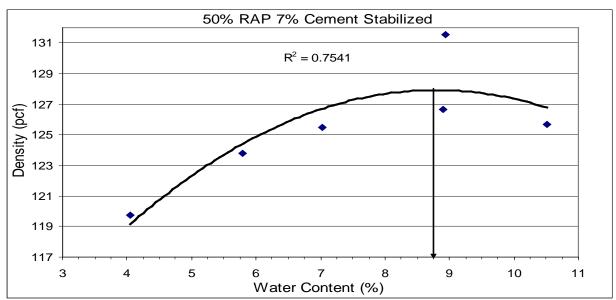


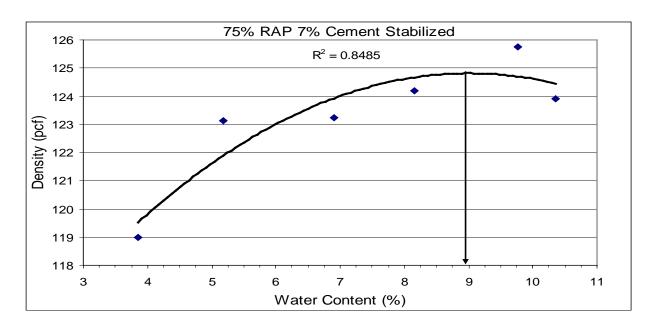


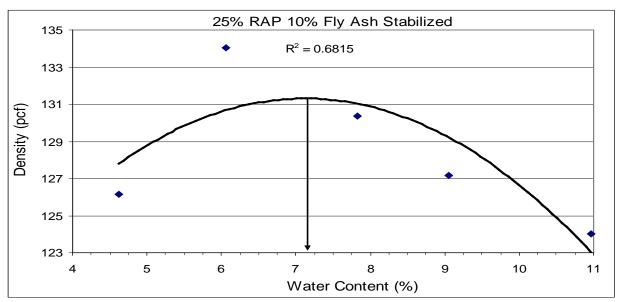


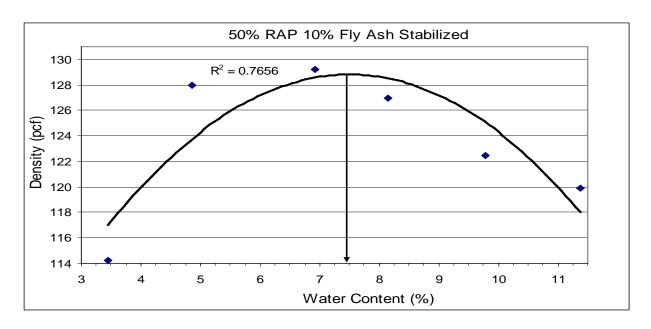


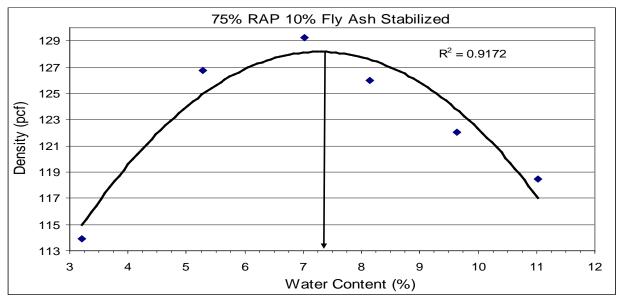


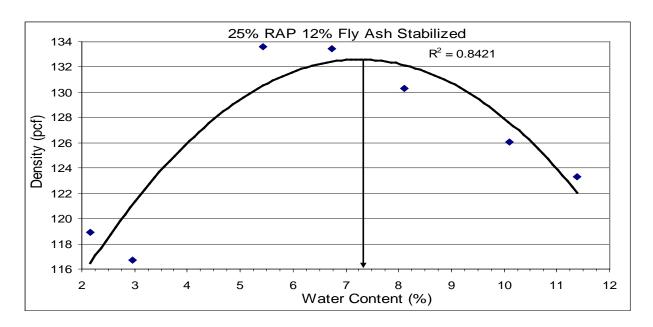


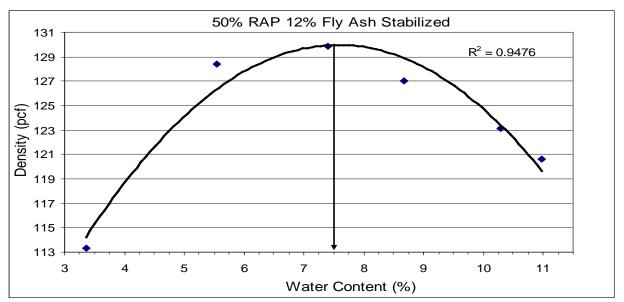


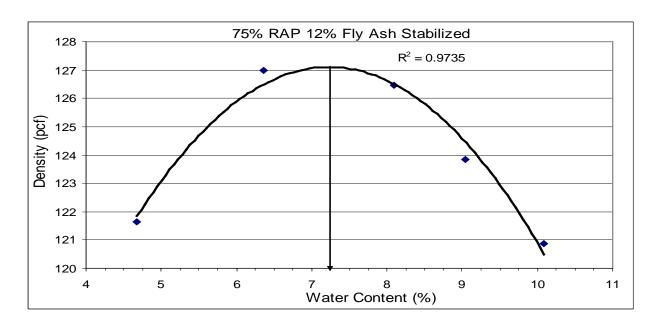


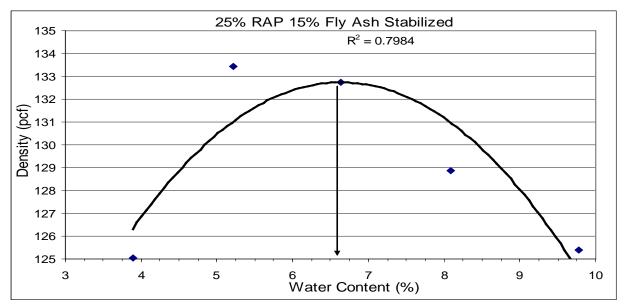


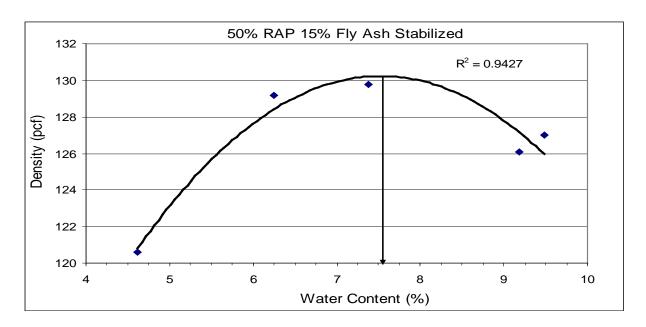


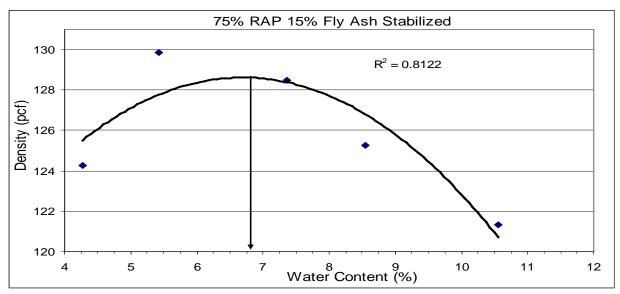




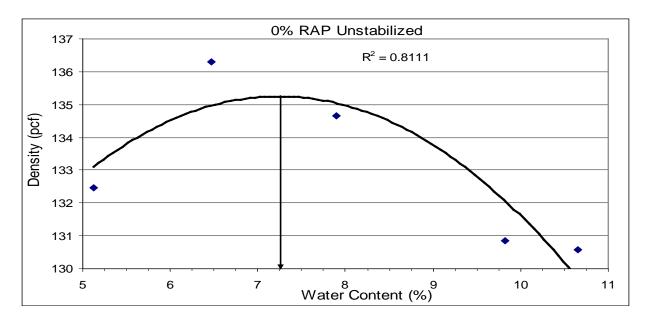


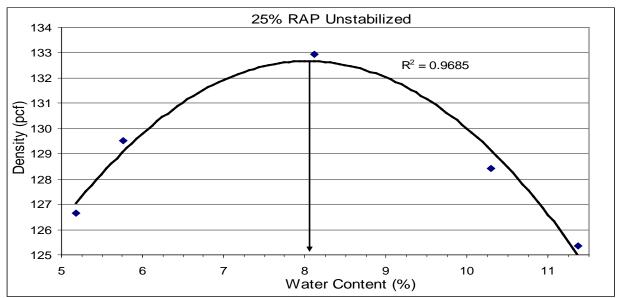


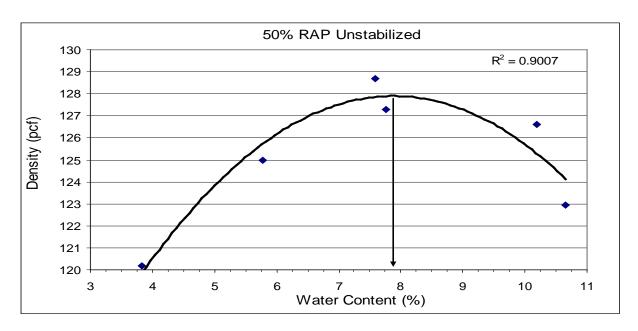


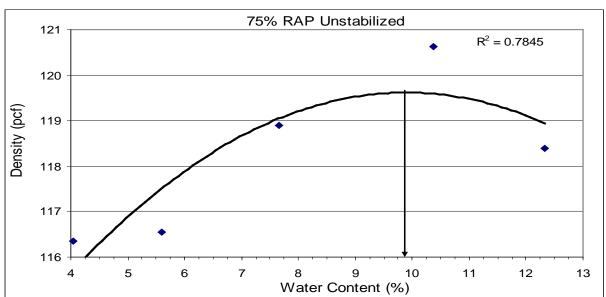


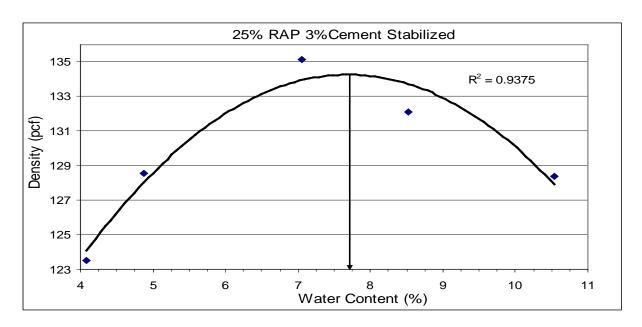
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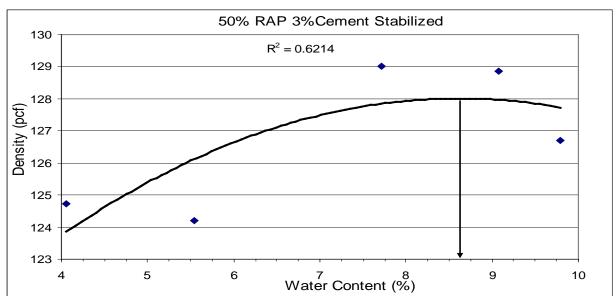


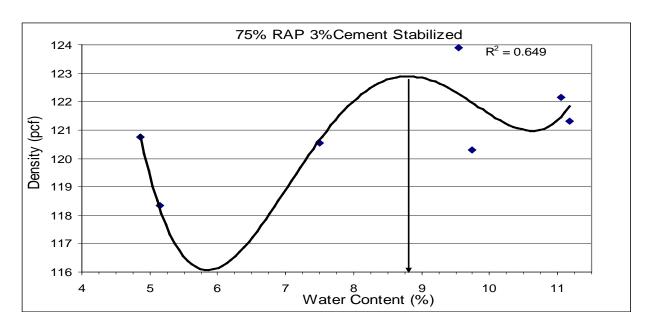


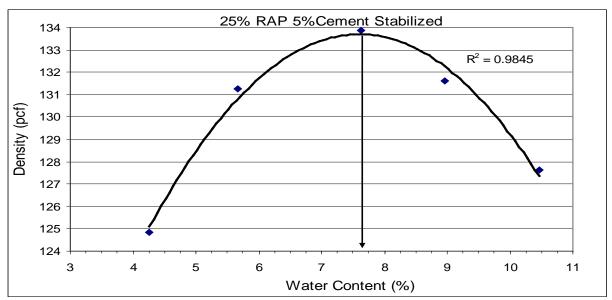


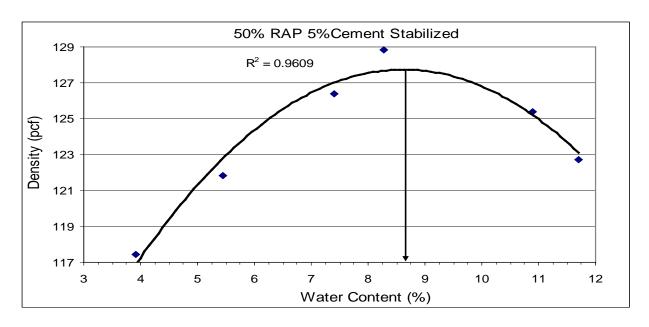


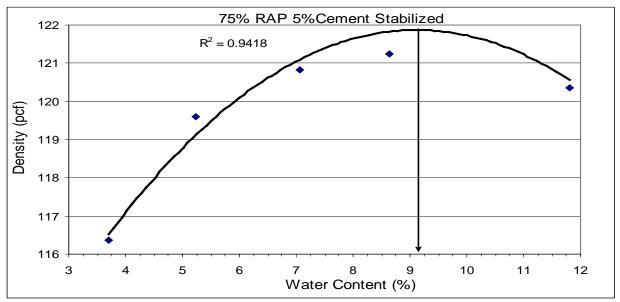


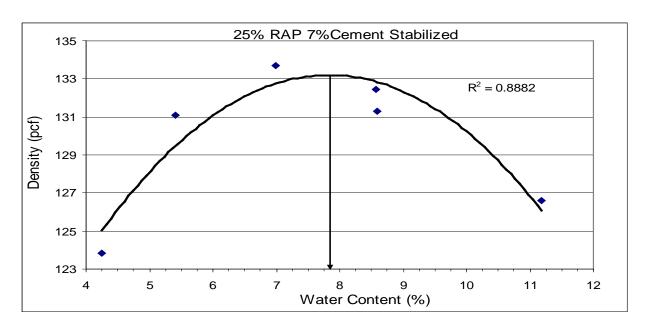


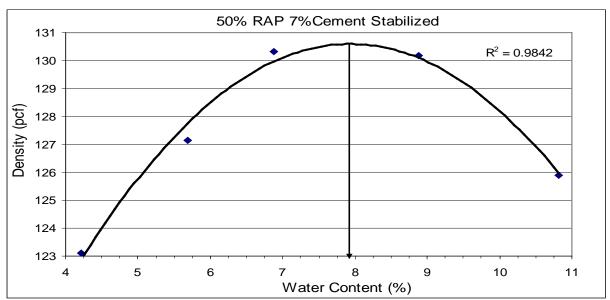


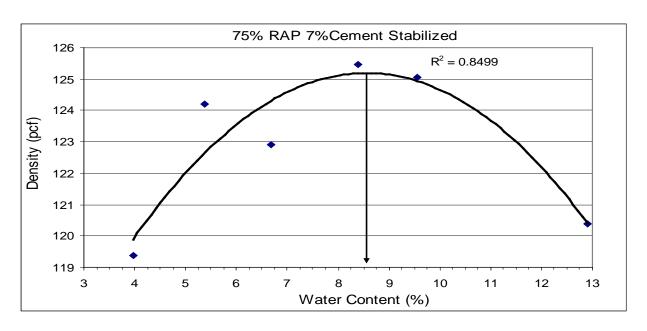


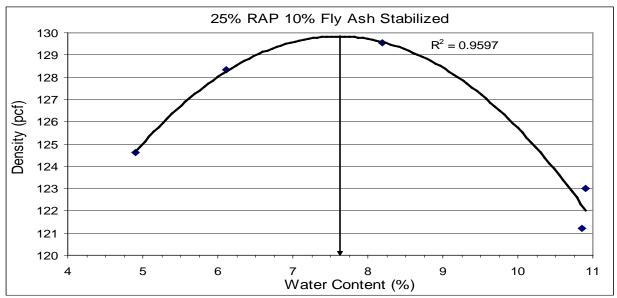


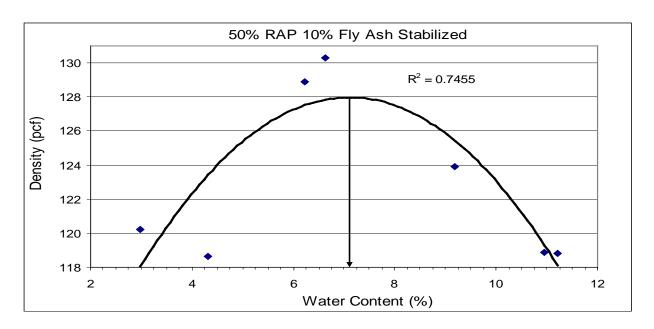


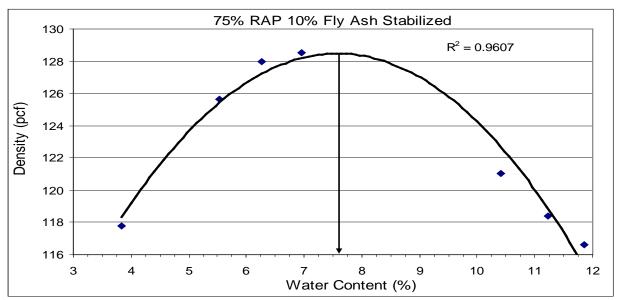


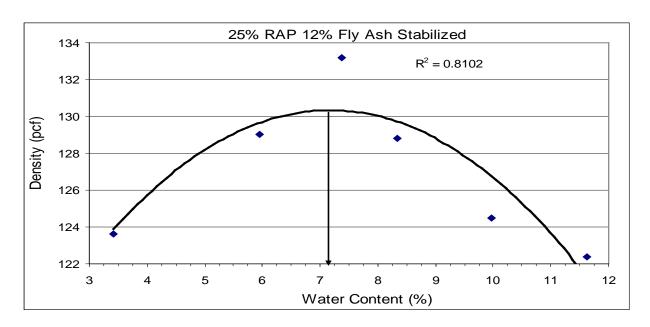


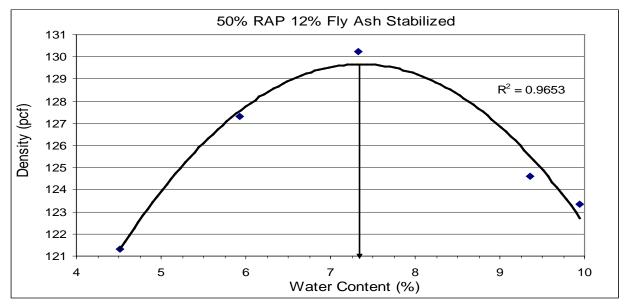


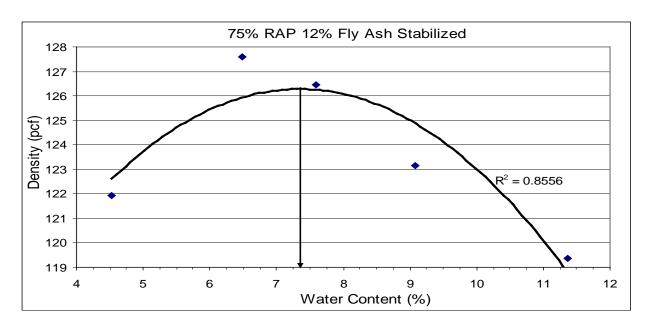


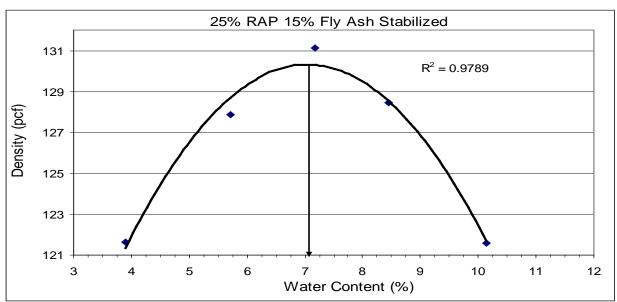


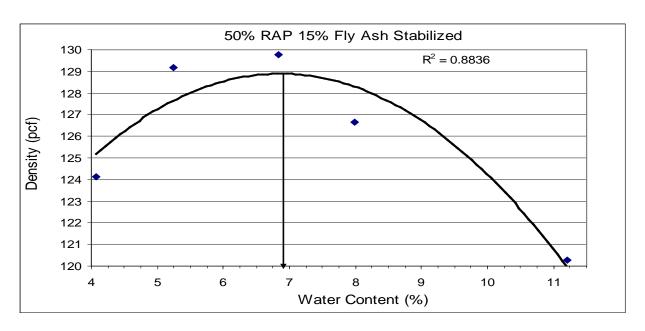


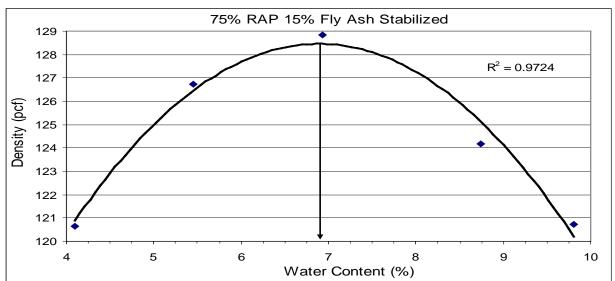




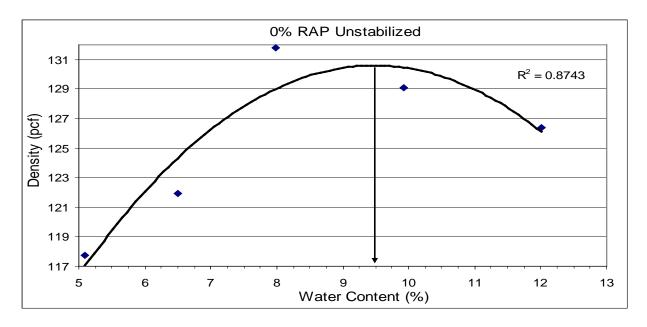


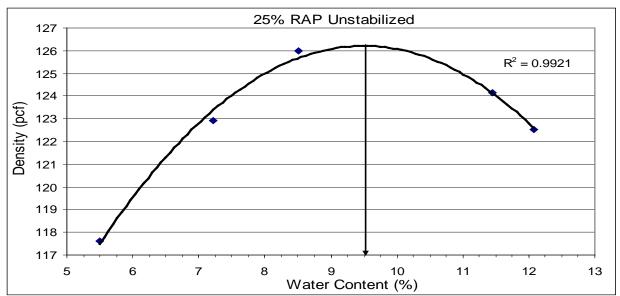


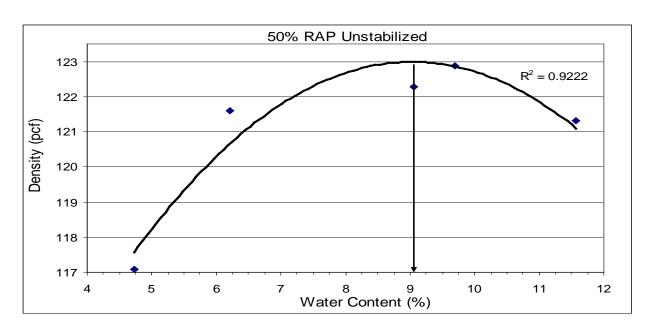


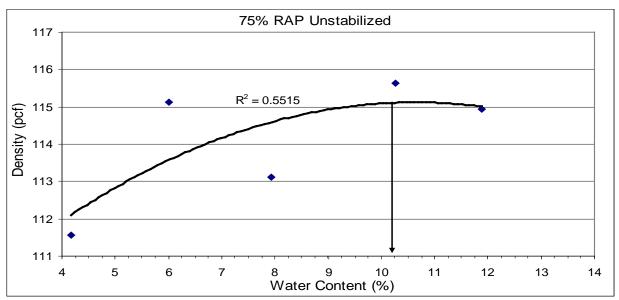


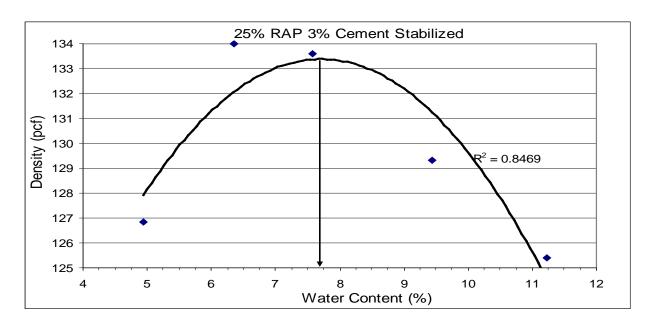
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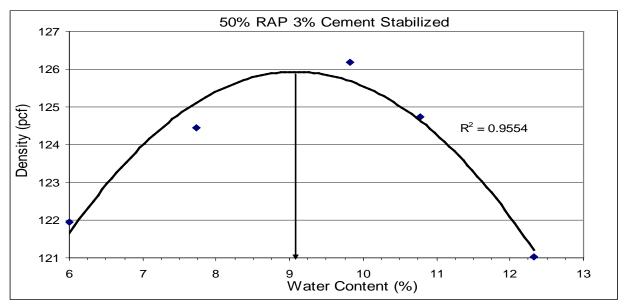


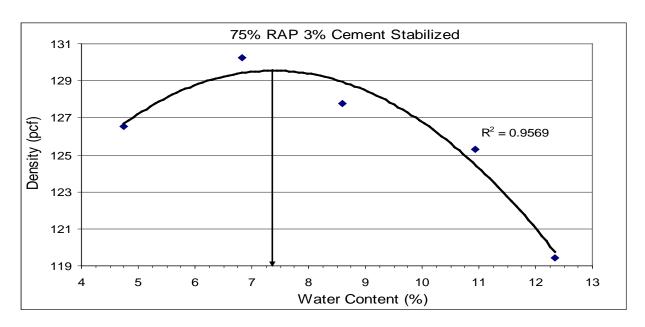


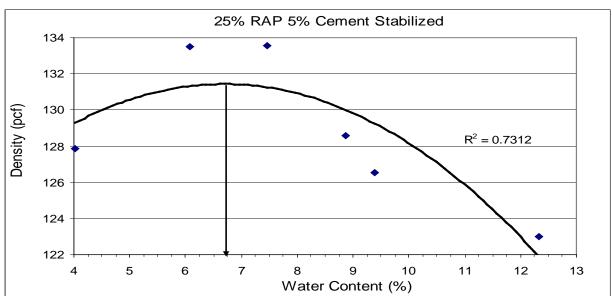


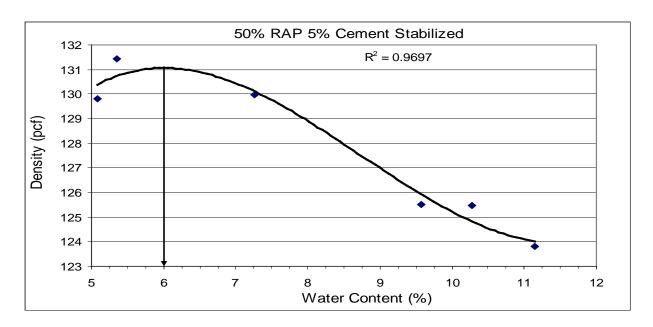


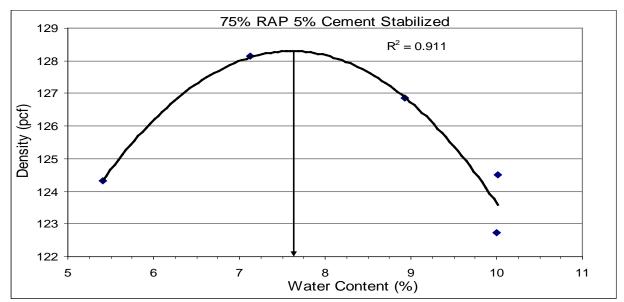


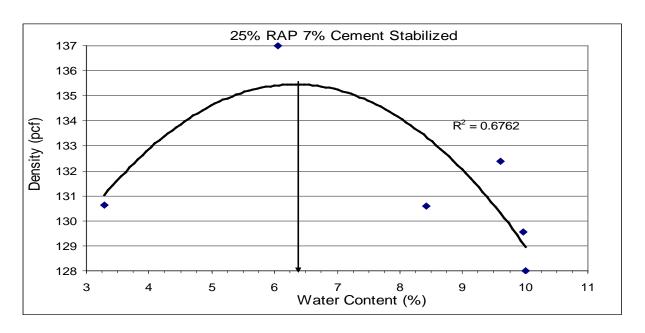


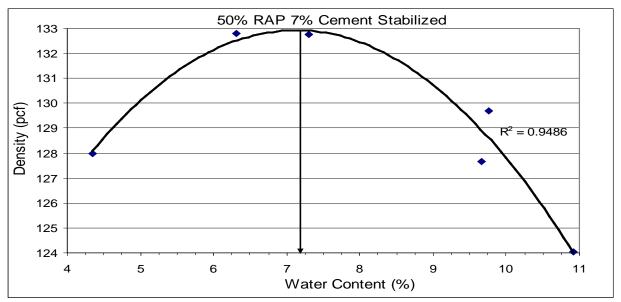


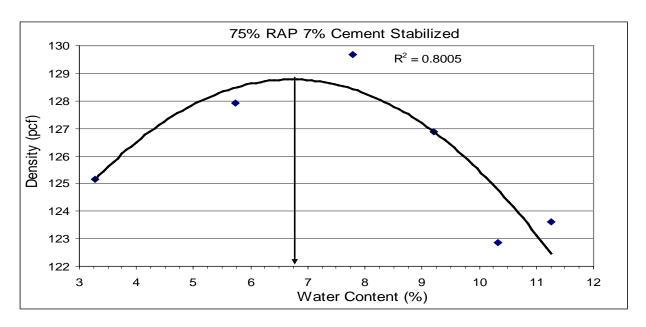


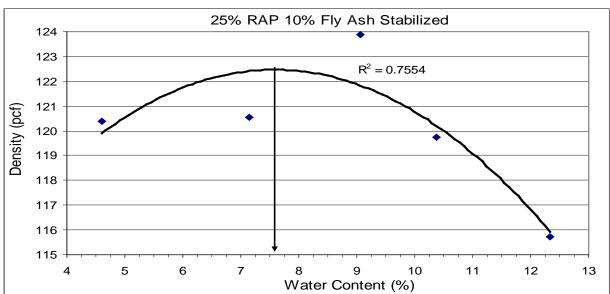


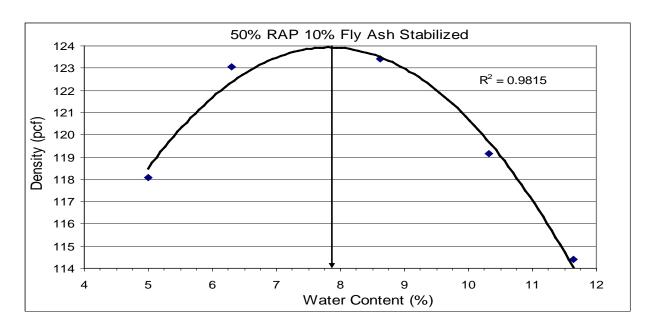


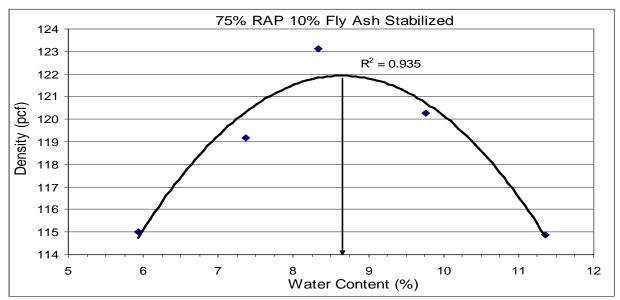


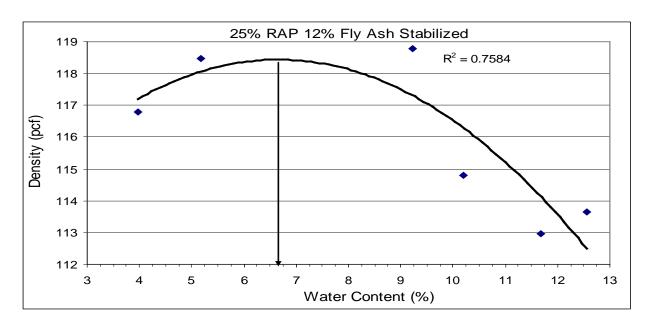


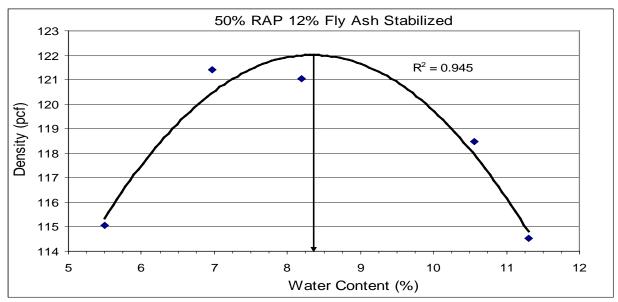


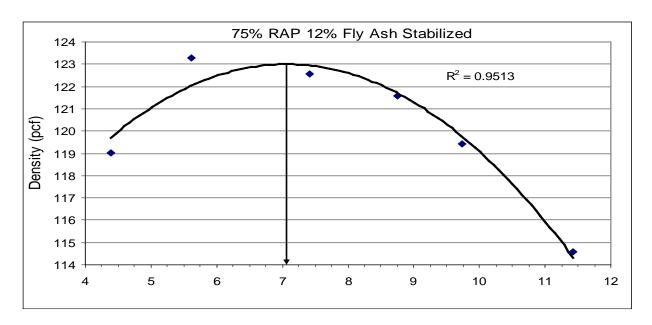


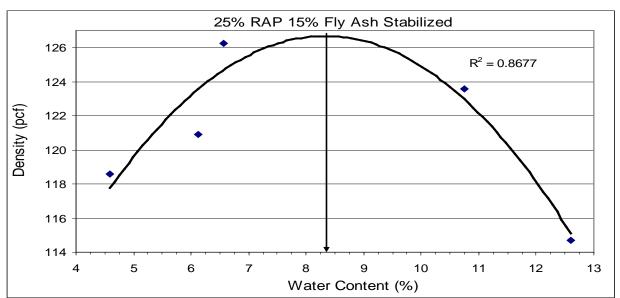


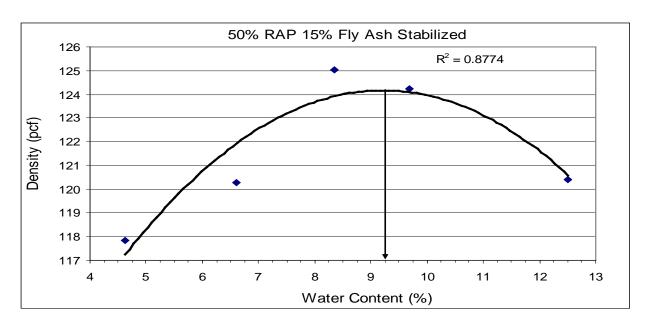


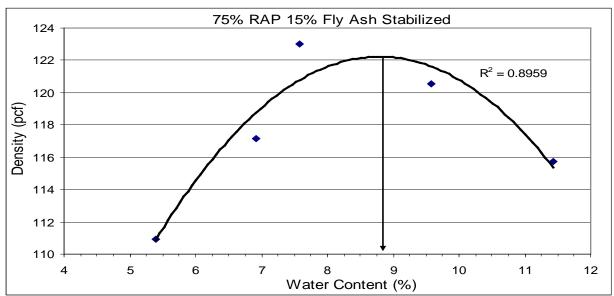




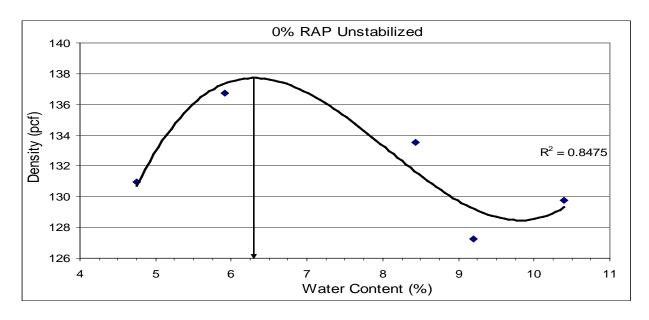


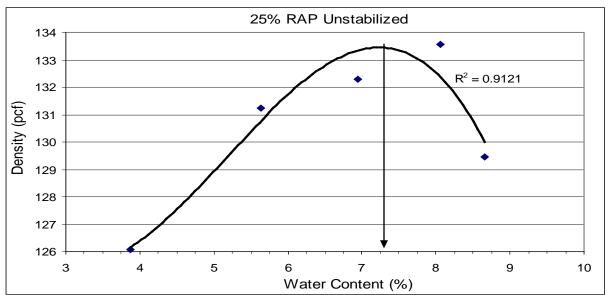


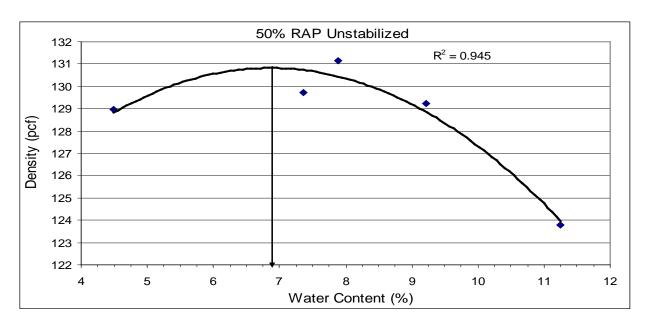


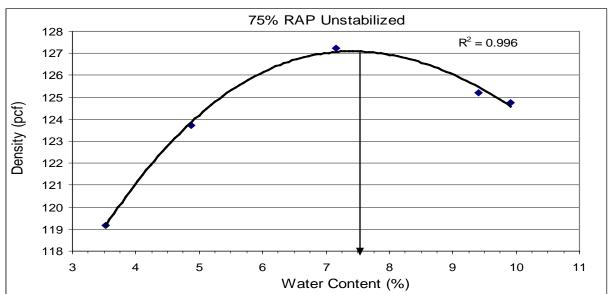


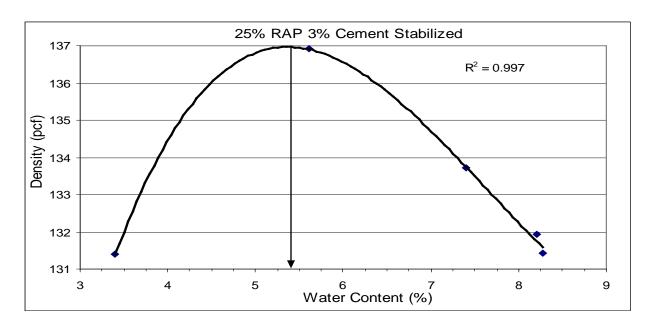
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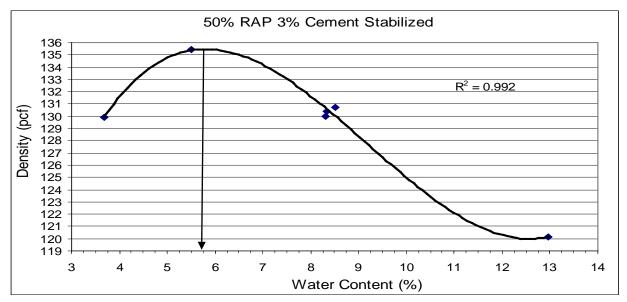


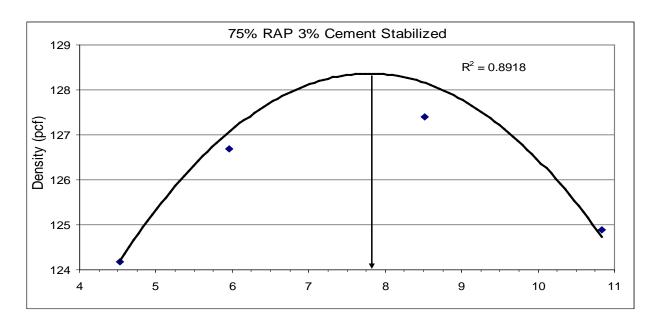


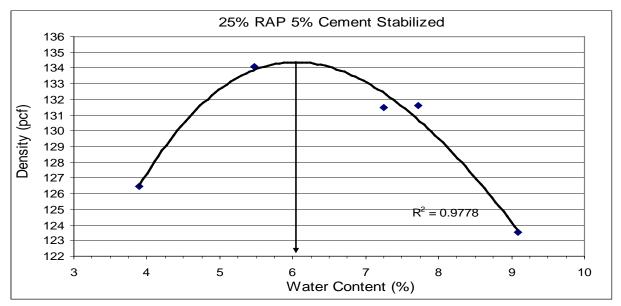


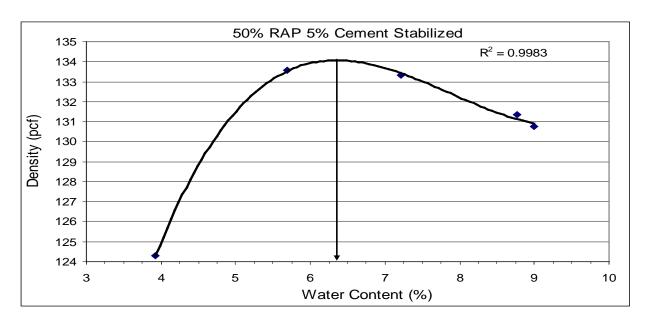


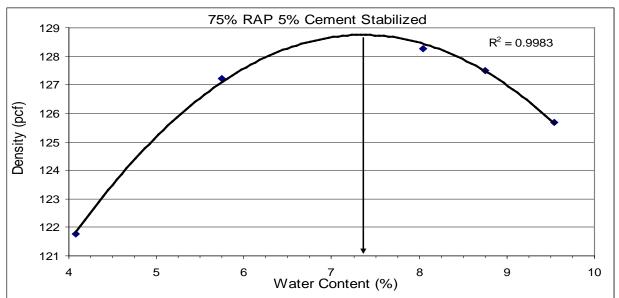


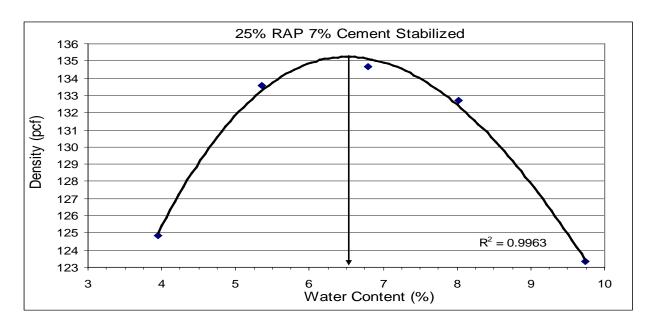


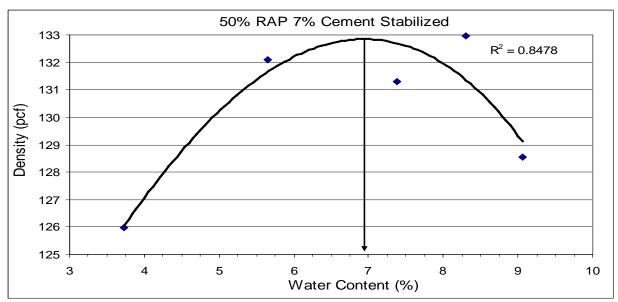


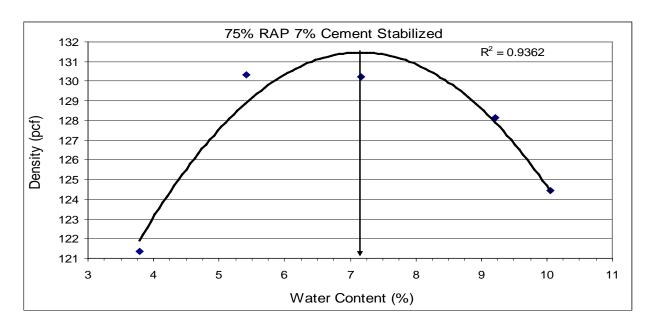


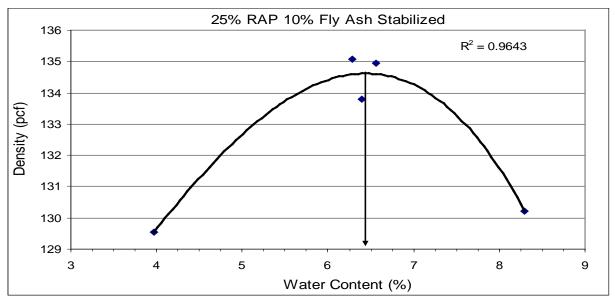


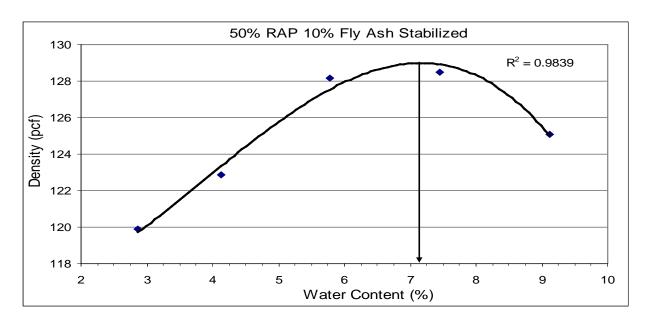


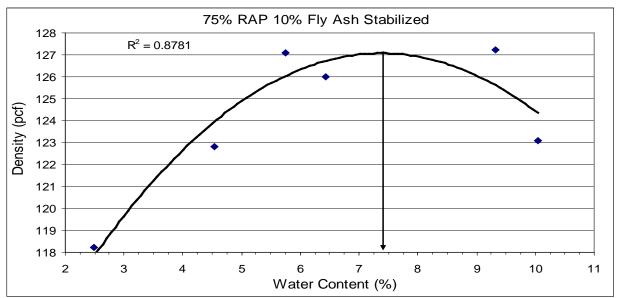


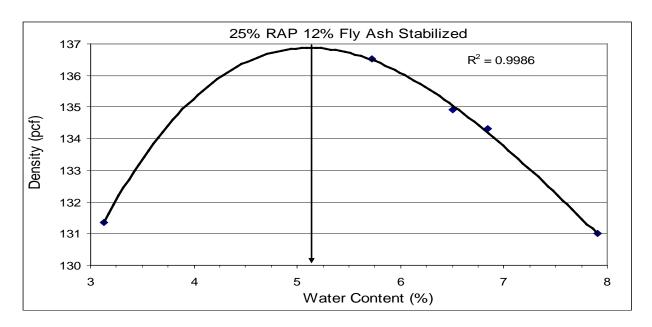


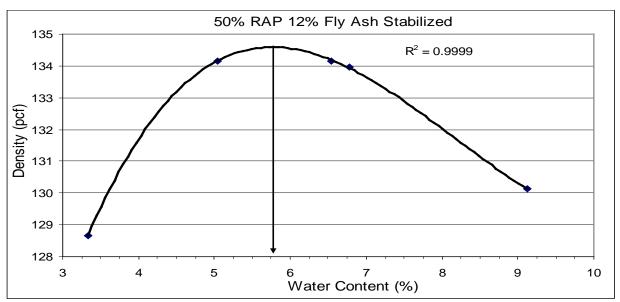


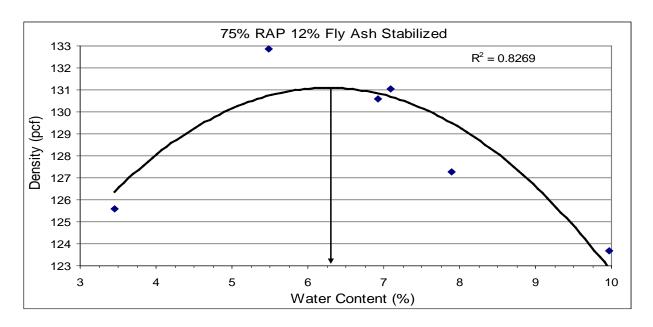


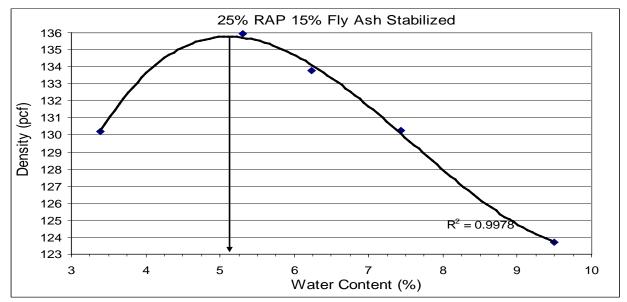


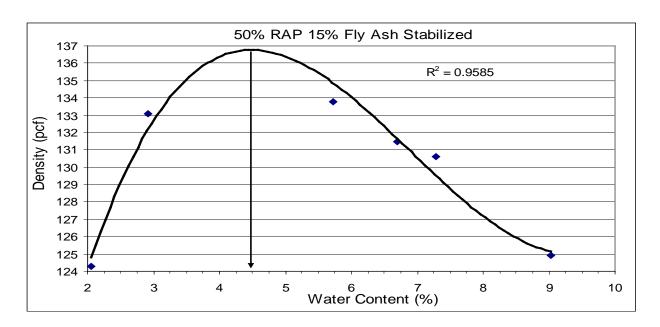


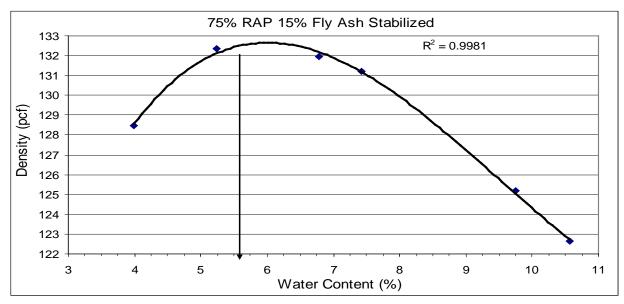




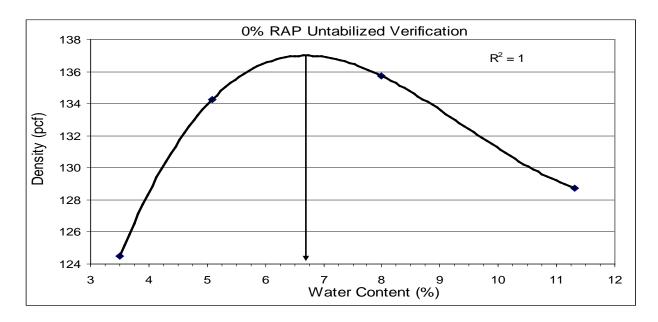








Poor Clean Verification



Appendix B

Optimum moisture content and maximum dry density

Optimum moisture and density with the shifts for pile changes in the poor clean material. The highlighted values are the old ones before the pile problems.

GOOD SOURCE - CLEAN GRADATION

Dorcontago					Stabi	ilized		
Percentage of RAP		Unstabilized		Cement			Fly Ash	
OFTIAL			3%	5%	7%	10%	12%	15%
75%	MC (%) =	9.30	10.25	8.25	9.00	7.28	7.25	6.67
7370	ρ (lb/ft^3) =	120.20	120.90	122.25	124.80	128.00	127.20	129.00
50%	MC (%) =	7.70	9.20	8.60	8.75	7.50	7.50	7.52
30%	ρ (lb/ft^3) =	118.90	125.00	125.50	128.00	129.00	130.00	130.30
25%	MC (%) =	9.00	9.75	10.30	9.20	7.20	7.30	6.60
2370	ρ (lb/ft^3) =	128.00	128.00	127.50	129.50	131.40	133.00	132.80
0%	MC (%) =	8.25						
070	ρ (lb/ft^3) =	132.50						

GOOD SOURCE - DIRTY GRADATION

					Stabi	ilized		
Percentage of RAP		Unstabilized		Cement			Fly Ash	
OTTO			3%	5%	7%	10%	12%	15%
75%	MC (%) =	9.80	8.85	9.20	8.60	7.60	7.35	6.80
7370	ρ (lb/ft^3) =	119.75	122.80	121.80	125.50	128.50	126.30	128.50
50%	MC (%) =	7.90	8.60	8.60	7.90	7.10	7.30	6.80
30%	ρ (lb/ft^3) =	128.00	128.00	128.00	130.60	128.00	129.50	128.90
25%	MC (%) =	8.10	7.70	7.60	7.80	7.60	7.20	7.10
23/0	ρ (lb/ft^3) =	132.50	134.50	133.60	133.10	129.90	130.20	130.00
0%	MC (%) =	7.25						
U%	ρ (lb/ft^3) =	135.30						

POOR SOURCE - CLEAN GRADATION

D					Stab	lized		
Percentage of RAP		Unstabilized		Cement			Fly Ash	
OFTICAL			3%	5%	7%	10%	12%	15%
75%	MC (%) =	9.49	6.85	7.04	6.25	8.06	6.58	8.11
73/0	ρ (lb/ft^3) =	116.63	131.11	130.00	130.40	123.52	124.53	123.82
50%	MC (%) =	7.72	7.76	5.12	6.05	6.65	7.08	7.89
30%	ρ (lb/ft^3) =	126.06	129.04	134.26	136.31	127.09	125.04	127.29
25%	MC (%) =	7.40	6.04	5.26	4.99	5.92	5.22	6.47
23/0	ρ (lb/ft^3) =	131.02	138.38	136.41	140.56	127.08	122.93	131.23
0%	MC (%) =	6.70						
070	ρ (lb/ft^3) =	137.00						

POOR SOURCE - DIRTY GRADATION

					Stab	lized		
Percentage of RAP		Unstabilized		Cement			Fly Ash	
011011			3%	5%	7%	10%	12%	15%
75%	MC (%) =	7.40	7.75	7.30	7.20	7.35	6.25	6.50
7370	ρ (lb/ft^3) =	127.00	128.50	128.70	131.50	127.10	131.20	132.60
50%	MC (%) =	7.00	5.80	6.30	7.00	7.25	5.80	4.50
30%	ρ (lb/ft^3) =	131.00	135.50	134.00	132.90	129.00	134.80	136.80
25%	MC (%) =	7.25	5.50	6.10	6.50	6.50	5.20	5.15
25%	ρ (lb/ft^3) =	133.50	137.00	134.50	135.30	134.60	137.00	135.80
0%	MC (%) =	6.25						
070	ρ (lb/ft^3) =	137.50						

POOR SOURCE - CLEAN GRADATION Pre pile problems

D					Stabi	llized		
Percentage of RAP		Unstabilized		Cement			Fly Ash	
OTTAL			3%	5%	7%	10%	12%	15%
75%	MC (%) =	10.25	7.40	7.60	6.75	8.70	7.10	8.75
73/0	ρ (lb/ft^3) =	115.20	129.50	128.40	128.80	122.00	123.00	122.30
50%	MC (%) =	9.05	9.10	6.00	7.10	7.80	8.30	9.25
30%	ρ (lb/ft^3) =	123.00	125.90	131.00	133.00	124.00	122.00	124.20
25%	MC (%) =	9.50	7.75	6.75	6.40	7.60	6.70	8.30
25%	ρ (lb/ft^3) =	126.30	133.40	131.50	135.50	122.50	118.50	126.50
0%	MC (%) =	9.50						
0%	ρ (lb/ft^3) =	130.50						

 $\label{eq:Appendix C} \textbf{Appendix C}$ Unconfined compressive strength soaked and un-soaked cement.

		U	CS Samp	ole	UCS D	•		Suction nple	UCS We	_	
Material	Cem ent %	Α	В	С	Avg.	cv	D	E	Avg.	cv	Ratio %
GC 25%	3	680	174	178	344	84	427	351	389	14	113%
RAP	5	306	262	280	282	8	603	675	639	8	226%
IVAI	7	249	244	278	257	7	788	788	788	0	306%
GC 50%	3	123	153	138	138	11	342	370	356	6	259%
RAP	5	323	229	374	309	24	401	504	452	16	147%
IVAI	7	341	415	466	407	15	788	716	752	7	185%
CC 750/	3	173	205	177	185	10	278	284	281	2	152%
GC 75% RAP	5	223	213	274	237	14	449	482	466	5	197%
IVAF	7	460	385	382	409	11	515	462	489	8	120%
CD 250/	3	301	371	385	352	13	673	499	586	21	166%
GD 25% RAP	5	581	534	550	555	4	1011	1082	1046	5	189%
KAF	7	1082	698	963	915	21	1504	1536	1520	1	166%
OD 500/	3	222	232	237	230	3	434	443	438	1	190%
GD 50% RAP	5	462	403	375	413	11	632	630	631	0	153%
KAF	7	1066	1066	1146	1093	4	1162	1042	1102	8	101%
OD 75%	3	133	173	129	145	17	266	319	293	13	202%
GD 75% RAP	5	200	250	207	219	12	443	453	448	1	205%
KAP	7	359	370	393	374	5	543	537	540	1	144%
	3	300	308	278	295	5	501	632	567	16	192%
PC 25% RAP	5	606	585	576	589	3	1098	1210	1154	7	196%
	7	786	987	786	853	14	1233	1098	1166	8	137%
	3	136	151	186	158	16	322	356	339	7	215%
PC 50% RAP	5	445	380	312	379	18	566	762	664	21	175%
	7	525	619	602	582	9	1050	1090	1070	3	184%
	3	193	144	130	156	21	384	265	325	26	208%
PC 75% RAP	5	257	281	231	256	10	346	388	367	8	143%
	7	510	455	596	520	14	531	604	567	9	109%
	3	468	425	469	454	6	1257	1154	1206	6	266%
PD 25% RAP	5	708	939	731	793	16	1281	1377	1329	5	168%
	7	1035	971	725	910	18	1544	1353	1448	9	159%
	3	478	368	416	421	13	725	692	709	3	168%
PD 50% RAP	5	648	607	728	661	9	697	747	722	5	109%
	7	736	665	787	730	8	1130	1027	1078	7	148%
	3	260	261	242	254	4	443	335	389	20	153%
PD 75% RAP	5	411	461	355	409	13	618	530	574	11	140%
	7	612	614	650	625	3	655	634	645	2	103%

Unconfined compressive strength soaked and un-soaked fly ash.

		UC	S Samp	le	UCS Dry	·	Tube S Sam		UCS W		
Material	Fly Ash %	А	В	O	Avg.	cv	D	Е	Avg.	cv	Ratio %
	10	1552	1353	1695	1533	11	791	796	793	0	52%
GC 25% RAP	12	795	N/A	995	895	16	795	797	796	0	89%
	15	1058	875	1154	1029	14	798	791	795	1	77%
	10	351	311	464	375	21	783	792	788	1	210%
GC 50% RAP	12	412	369	304	362	15	779	784	781	0	216%
	15	370	424	421	405	8	793	789	791	0	195%
	10	254	281	309	281	10	614	633	623	2	222%
GC 75% RAP	12	307	376	321	335	11	590	543	566	6	169%
	15	363	495	376	412	18	796	773	785	2	191%
	10	552	571	615	579	6	262	258	260	1	45%
GD 25% RAP	12	610	693	635	646	7	240	253	246	4	38%
	15	662	445	630	579	20	333	320	326	3	56%
	10	363	342	532	412	25	168	190	179	9	44%
GD 50% RAP	12	671	605	537	604	11	184	242	213	19	35%
	15	428	510	643	527	21	268	243	255	7	48%
	10	223	312	293	276	17	117	107	112	6	41%
GD 75% RAP	12	345	342	304	330	7	124	127	125	2	38%
	15	513	381	543	479	18	185	199	192	5	40%
	10	561	534	580	559	4	636	552	594	10	106%
PC 25% RAP	12	682	652	607	647	6	641	625	633	2	98%
	15	844	955	1066	955	12	774	634	704	14	74%
	10	301	374	361	345	11	453	521	487	10	141%
PC 50% RAP	12	368	434	409	404	8	615	523	569	12	141%
	15	581	601	596	593	2	731	716	723	2	122%
	10	208	177	269	218	21	358	401	380	8	174%
PC 75% RAP	12	275	341	364	327	14	500	532	516	4	158%
	15	343	411	474	409	16	471	621	546	19	133%

	10	92	66	76	78	17	547	331	439	35	562%
PD 25% RAP	12	177	166	149	164	9	382	343	362	8	221%
	15	172	188	151	170	11	424	359	392	12	230%
	10	35	29	38	34	14	309	447	378	26	1113%
PD 50% RAP	12	61	71	52	61	15	295	298	296	1	482%
	15	154	156	166	159	4	406	274	340	27	214%
	10	37	40	40	39	5	327	384	356	11	915%
PD 75% RAP	12	63	56	64	61	7	277	444	360	33	594%
	15	65	71	53	63	14	272	416	344	30	547%

Moisture sensitivity for cement and fly ash dielectric constants.

		Tube S Sam					Tube S Sam			
Material	Cement %	Α	В	Avg.	cv	Fly Ash %	А	В	Avg.	cv
GC 25%	3	4.48	6.14	5.31	22.11	10	4.77	4.20	4.49	9.04
RAP	5	6.14	3.01	4.57	48.45	12	4.97	5.19	5.08	3.16
IXAI	7	2.98	5.15	4.06	37.70	15	6.03	5.75	5.89	3.28
GC 50%	3	4.84	3.07	3.95	31.72	10	5.75	3.87	4.81	27.66
RAP	5	3.27	2.55	2.91	17.33	12	4.81	3.44	4.13	23.53
IXAF	7	4.93	4.23	4.58	10.91	15	5.81	6.34	6.08	6.13
CC 750/	3	3.79	3.77	3.78	0.37	10	4.68	5.92	5.30	16.54
GC 75% RAP	5	2.93	2.81	2.87	2.96	12	4.72	4.45	4.58	4.22
NAF	7	2.65	5.05	3.85	43.96	15	5.96	2.71	4.33	53.09
CD 250/	3	8.07	4.57	6.32	39.14	10	11.07	10.04	10.56	6.92
GD 25% RAP	5	6.71	6.87	6.79	1.60	12	10.87	11.28	11.08	2.60
KAP	7	5.34	5.94	5.64	7.52	15	8.52	7.83	8.18	5.94
OD 500/	3	5.58	5.83	5.71	3.14	10	6.50	7.60	7.05	11.03
GD 50% RAP	5	5.97	4.67	5.32	17.27	12	13.56	8.35	10.96	33.60
KAP	7	6.13	7.77	6.95	16.69	15	9.29	9.17	9.23	0.97
OD 750/	3	8.23	10.39	9.31	16.40	10	7.66	8.36	8.01	6.18
GD 75% RAP	5	5.59	7.19	6.39	17.63	12	11.53	6.83	9.18	36.15
KAP	7	5.05	6.80	5.93	20.84	15	5.63	8.49	7.06	28.63
DO 050/	3	6.24	6.19	6.22	0.53	10	5.14	6.81	5.97	19.73
PC 25% RAP	5	8.39	6.68	7.54	16.07	12	4.33	5.39	4.86	15.52
KAP	7	3.73	6.77	5.25	40.92	15	6.38	3.71	5.04	37.48
DO 500/	3	7.94	7.77	7.86	1.50	10	6.11	8.23	7.17	20.84
PC 50% RAP	5	4.25	3.72	3.98	9.35	12	6.23	6.86	6.54	6.84
KAP	7	4.72	4.11	4.41	9.83	15	3.43	4.21	3.82	14.31
DO 750/	3	4.84	6.45	5.65	20.20	10	7.71	4.28	5.99	40.43
PC 75% RAP	5	6.80	7.46	7.13	6.55	12	6.13	5.18	5.65	11.84
KAP	7	4.62	3.81	4.21	13.65	15	7.89	9.17	8.53	10.67
DD 050/	3	6.56	4.52	5.54	26.04	10	4.77	7.75	6.26	33.68
PD 25%	5	2.99	3.97	3.48	19.78	12	4.10	6.90	5.50	36.00
RAP	7	4.93	3.01	3.97	34.23	15	5.87	7.89	6.88	20.77
DD 500/	3	3.17	2.93	3.05	5.57	10	9.03	5.04	7.03	40.08
PD 50% RAP	5	3.91	4.57	4.24	11.12	12	6.99	7.21	7.10	2.19
KAP	7	3.36	4.45	3.91	19.79	15	9.09	10.49	9.79	10.06
DD 750/	3	4.43	6.01	5.22	21.49	10	4.87	4.99	4.93	1.72
PD 75%	5	3.49	3.69	3.59	4.07	12	8.99	7.95	8.47	8.69
RAP	7	3.69	3.51	3.60	3.67	15	6.93	11.41	9.17	34.49

Appendix D

Weights in grams of specimens after 12 day moisture conditioning test on optimums. The samples without letters are the averages.

Specimen ID	Cycl	Cvcle	Cycle	Cvcle	Cycle	Cvcle	Cycle	Cvcle	Cvcle	Cycle	Cvcle	Cycle
	e 1	2	3	4	5	6	7	8	9	10	11	12
Poor Clean 75% RAP	2005	1997.	1991.	1992.	1985.	1989.	1994.	1995.	1995.	1994.	1992.	1991.
5% CEM A	.0	0	7	6	3	4	6	9	4	4	4	7
Poor Clean 75% RAP	2003	1995.	1989.	1991.	1986.	1982.	1987.	1986.	1982.	1974.	1971.	1971.
5% CEM B	.9	5	9	1	8	2	6	9	7	9	7	0
Poor Clean 75% RAP	2002	1994.	1989.	1990.	1987.	1983.	1989.	1988.	1988.	1987.	1988.	1987.
5% CEM C	.2	0	4	8	4	8	4	9	7	5	0	0
Poor Clean 75% RAP	1987	1976.	1974.	1974.	1971.	1970.	1973.	1972.	1971.	1971.	1971.	1967.
5% CEM D	.1	2	4	2	9	4	5	3	3	9	0	6
Poor Clean 75% RAP	2005	1998.	1993.	1994.	1992.	1989.	1994.	1994.	1994.	1992.	1992.	1993.
5% CEM E	.1	6	1	0	6	8	7	9	9	9	5	0
Poor Clean 75% RAP	1997	1991.	1985.	1985.	1984.	1982.	1986.	1985.	1985.	1986.	1985.	1984.
5% CEM F	.9	3	0	7	0	3	4	9	9	7	7	9
Poor Clean 75% RAP	2000	1992.	1987.	1988.	1984.	1983.	1987.	1987.	1986.	1984.	1983.	1982.
5% CEM	.2	1	3	1	7	0	7	5	5	7	6	5
Poor Clean 50% RAP	2056	2053.	2052.	2052.	2050.	2047.	2052.	2054.	2054.	2052.	2053.	2052.
5% CEM A	.6	2	9	7	1	2	3	1	7	7	2	3
Poor Clean 50% RAP	2050	2044.	2044.	2042.	2039.	2035.	2040.	2040.	2040.	2039.	2038.	2035.
5% CEM B	.4	8	4	7	3	9	4	9	4	7	9	7
Poor Clean 50% RAP	2008	2002.	1998.	1997.	1993.	1991.	1994.	1996.	1996.	1993.	1993.	1990.
5% CEM C	.6	0	1	0	6	6	5	5	4	1	4	5
Poor Clean 50% RAP	2061	2056.	2055.	2053.	2050.	2048.	2053.	2054.	2055.	2054.	2054.	2053.
5% CEM D	.5	0	6	7	7	0	4	0	6	5	5	7
Poor Clean 50% RAP	2063	2057.	2055.	2049.	2047.	2044.	2047.	2050.	2050.	2049.	2046.	2043.
5% CEM E	.3	8	2	4	8	2	8	4	0	4	6	3
Poor Clean 50% RAP	2023	2021.	2018.	2017.	2015.	2013.	2016.	2016.	2018.	2016.	2016.	2013.
5% CEM F	.8	3	7	0	6	2	6	7	1	1	7	9
Poor Clean 50% RAP	2044		2037.	2035.	2032.	2030.	2034.	2035.	2035.	2034.	2033.	2031.
5% CEM	.0	2	5	4	9	0	2	4	9	3	9	6
Poor Clean 25% RAP	2071	2066.	2062.	2050.	2058.	2056.	2056.	2056.	2053.	2051.	2053.	2051.
3% CEM A	.2	0	5	3	1	8	7	2	7	5	8	6
Poor Clean 25% RAP	2077	2071.	2065.	2045.	2044.	2042.	2042.	2040.	2036.	2034.	2034.	2031.
3% CEM B	.8	5	4	8	9	3	0	2	9	9	7	6
Poor Clean 25% RAP	2066	2060.	2057.	2043.	2048.	2050.	2047.	2046.	2044.	2042.	2043.	2041.
3% CEM C	.7	0	0	5	5	3	7	6	0	0	5	2
Poor Clean 25% RAP		2077.	2071.	2057.	2062.		2063.	2061.	2059.		2056.	2054.
3% CEM D	.3	6	5	8	6	7	2	6	2	3	0	5
Poor Clean 25% RAP	2082	2058.	2049.	2038.	2043.	2046.	2044.	2042.	2040.	2038.	2039.	2038.
3% CEM E	.4	1	7	7	5	0	8	2	8	3	7	5
Poor Clean 25% RAP		2056.	2051.	2038.	2046.		2044.	2043.	2040.	2037.	2040.	2038.
3% CEM F	.2	8	2	8	2	0	2	0	5	0	7	4

			I	I · -					I · -			
Poor Clean 25% RAP		2065.	2059.	2045.	2050.		2049.	2048.	2045.	2043.	2044.	2042.
3% CEM	.3	0	6	8	6	9	8	3	9	2	7	6
Poor Clean 25% RAP	2004	1978.	1970.	1963.	1958.	1958.	1956.	1956.	1954.	1953.	1950.	1951.
12% FA A	.5	8	5	1	6	8	5	0	5	3	9	9
Poor Clean 25% RAP	2057	2036.	2032.	2026.	2023.	2025.	2023.	2022.	2020.	2020.	2019.	2019.
12% FA B	.6	9	1	7	7	4	5	7	2	2	4	0
Poor Clean25% RAP	2059	2042.	2034.	2028.	2026.	2028.	2025.	2025.	2023.	2022.	2025.	2021.
12% FA C	.7	9	7	6	8	0	6	5	7	8	2	8
Poor Clean 25% RAP	2055	2031.	2021.	2015.	2012.	2009.	2008.	2003.	2000.	1998.	2000.	1996.
12% FA D	.5	5	7	7	1	7	3	8	7	6	7	9
Poor Clean 25% RAP	2065	2048.	2043.	2038.	2037.	2036.	2034.	2035.	2032.	2032.	2035.	2034.
12% FA E	.5	8	8	7	3	6	7	4	4	5	7	8
Poor Clean 25% RAP	2003	1982.	1973.	1966.	1966.	1967.	1963.	1963.	1961.	1960.	1961.	1962.
12% FA F	.5	3	4	9	3	5	8	8	1	9	8	1
Poor Clean 25% RAP	2041	2020.	2012.	2006.	2004.	2004.	2002.	2001.	1998.	1998.	1999.	1997.
12% FA	.1	2	7	6	1	3	1	2	8	1	0	8
Poor Clean 50% RAP	2004	1990.	1980.	1978.	1976.	1975.	1973.	1972.	1971.	1974.	1972.	1966.
12% FA A	.8	8	1	0	9	1	8	4	1	2	9	3
Poor Clean 50% RAP	2034	2029.	2021.	2019.	2019.	2019.	2017.	2016.	2013.	2016.	2016.	2014.
12% FA B	.7	2	4	1	2	7	5	0	6	6	9	5
Poor Clean 50% RAP	2043	2037.	2031.	2031.	2030.	2028.	2028.	2029.	2026.	2029.	2029.	2026.
12% FA C	.4	1	6	6	7	8	9	0	3	8	3	3
Poor Clean 50% RAP	2047	2042.	2036.	2036.	2035.	2035.	2033.	2032.	2029.	2032.	2031.	2029.
12% FA D	.7	0	7	8	2	0	1	0	5	3	3	2
Poor Clean 50% RAP	2026		2019.	2017.	2019.	2017.	2015.	2017.	2016.	2017.	2016.	2014.
12% FA E	.7	8	1	1	9	3	8	1	8	9	9	6
Poor Clean 50% RAP	2012		2002.	2000.	2002.	2001.	1999.	1996.	1997.	1999.	1999.	1995.
12% FA F	.8	8	8	0	4	4	3	5	6	7	0	8
Poor Clean 50% RAP	2028		2015.	2013.	2014.	2012.	2011.	2010.	2009.	2011.	2011.	2007.
12% FA	.4	3	3	8	1	9	4	5	2	8	1	8
Poor Clean 75% RAP	1961	1956.	1954.	1949.	1948.	1945.	1944.	1941.	1940.	1938.	1935.	1933.
10% FA A	1.6	4	2	4	1940.	4	3	1941.	8	8	4	5
	_				1954.				_	1941.		
10% FA B	.6	3	3	1900.	1954.	3	8	9	5	1941.		2
				-							0	
Poor Clean 75% RAP		1957.	1950.	1945.	1945.	1941.	1940.	1938.	1935.		1932.	1930.
10% FA C	.1	2	6	2	2	6	9	4	5	5	6	2
Poor Clean 75% RAP	1990		1979.	1975.	1976.	1975.	1971.	1972.	1970.	1967.	1964.	1961.
10% FA D	.7	4	4	5	9	1	6	3	6	0	6	8
Poor Clean 75% RAP	1971		1965.	1960.	1960.	1958.	1955.	1953.	1951.	1950.	1949.	1945.
10% FA E	.6	4	4	2	1	5	1	1	8	9	0	4
Poor Clean 75% RAP	1940		1935.	1927.	1925.		1921.	1918.	1916.		1910.	1906.
10% FA F	.3	5	3	8	1	5	7	2	3	4	3	3
Poor Clean 75% RAP	1967	1961.	1957.	1951.	1951.	1949.	1946.	1944.	1942.		1938.	1935.
10% FA	.3	0	2	9	6	1	6	5	9	0	3	4
Good Dirty 25% RAP		2008.	1998.	1995.	1994.		1985.	1984.	1981.		1971.	1969.
3% CEM A	.8	6	3	2	1	9	5	2	5	0	6	9
Good Dirty 25% RAP	1994	1990.	1978.	1974.	1974.	1970.	1968.	1967.	1965.	1965.	1960.	1960.
	l		l	l	l	l	l	l	l	L	l	l

3% CEM B	.8	7	5	5	5	9	3	7	9	5	7	3
Good Dirty 25% RAP	1972	1969.	1961.	1957.	1957.	1952.	1949.	1947.	1945.	1942.	1936.	1934.
3% CEM C	1.1	7	2	9	1	2	7	3	0	0	7	7
Good Dirty 25% RAP	1962	1958.	1954.	1952.	1953.	1950.	1948.	1947.	1945.	1945.	1942.	1940.
3% CEM D	.7	7	9	9	3	7	8	1	5	2	1	3
Good Dirty 25% RAP	2006		1983.	1973.	1968.	1963.	1955.	1947.	1942.	1934.	1920.	1915.
3% CEM E	.2	3	6	9	9	1	5	7	7	0	3	9
Good Dirty 25% RAP	1997	1989.	1978.	1971.	1965.	1957.	1950.	1944.	1940.	1934.	1924.	1917.
3% CEM F	.8	1	2	3	9	5	8	8	0	7	7	5
Good Dirty 25% RAP	1991	1985.	1975.	1971.	1969.	1964.	1959.	1956.	1953.	1949.	1942.	1939.
3% CEM	.1	9	8	0	0	1	8	5	4	9	7	8
Good Dirty 50% RAP	2146	_		2129.	_		_	2135.	2129.	2125.	2131.	2137.
5% CEM A	.0	4	7	6	5	4	6	3	2	5	2	0
Good Dirty 50% RAP		2121.	2116.	2121.	2117.		2113.				2118.	2125.
5% CEM B	.6	3	3	9	6	5	1	6	7	0	4	2123.
		2135.	2131.	2133.	2130.		2129.	2140.	2131.	2127.	2133.	2137.
Good Dirty 50% RAP												
5% CEM C	.5	6	2	5	2	1	3	5	9	8 2079.	6	2
Good Dirty 50% RAP		2098.	2093.		2089.		2087.		2082.		2081.	2084.
5% CEM D	.6	9	0	3	7	6	3	4	4	5	5	8
Good Dirty 50% RAP	2089		2051.	2048.	2044.	2034.	2025.	2021.	2016.	2009.	2004.	2003.
5% CEM E	.4	8	5	0	2	7	6	7	2	0	7	5
Good Dirty 50% RAP	2078		2054.	2053.	2050.	2052.	2046.	2049.	2044.	2038.	2041.	2041.
5% CEM F	.5	5	6	8	3	8	9	9	1	3	2	8
Good Dirty 50% RAP	2118		2095.	2097.	2093.	2095.	2088.	2092.	2086.	2082.	2085.	2088.
5% CEM	.8	9	7	0	1	2	0	6	6	5	1	3
Good Dirty 75% RAP	2055		2042.	2038.	2041.	2038.	2042.	2039.	2036.	2037.	2040.	2037.
7% CEM A	.1	5	7	2	3	4	2	7	6	8	3	1
Good Dirty 75% RAP	2039		2024.	2020.	2021.	2017.	2017.	2015.	2014.	2014.	2015.	2010.
7% CEM B	.2	8	0	4	1	2	9	3	0	0	7	0
Good Dirty 75% RAP	2027	2015.	2014.	2010.	2012.	2008.	2012.	2009.	2007.	2009.	2013.	2009.
7% CEM C	.9	5	5	8	9	7	4	6	8	9	9	4
Good Dirty 75% RAP		2018.			2014.		2013.		2009.	2009.	2013.	2013.
7% CEM D	.3	3	3	2	6	8	5	9	3	2	3	2
Good Dirty 75% RAP	2039		2028.	2022.	2027.	2021.	2027.	2023.	2020.	2022.	2026.	2022.
7% CEM E	.8	3	1	7	6	5	1	7	2	1	2	4
Good Dirty 75% RAP	2019		2007.	2002.	2007.	2002.	2005.	2003.	2000.	2002.	2004.	2002.
7% CEM F	.9	2	4	6	1	8	9	4	5	5	5	1
Good Dirty 75% RAP		2023.	2022.	2018.		2016.		2016.	2014.		2019.	2015.
7% CEM	.2	4	3	2	8	7	8	9	7	9	0	7
Good Dirty 75% RAP	1814		1785.	1776.	1773.	1770.	1766.	1751.	1749.	1740.	1736.	1735.
10% FA A	.9	6	2	2	8	6	7	2	9	0	2	8
Good Dirty 75% RAP	1844		1816.	1807.	1804.	1799.	1794.	1792.	1795.	1789.	1786.	1785.
10% FA B	.0	8	2	5	7	2	9	9	0	7	2	8
Good Dirty 75% RAP	1862		1823.	1810.	1810.	1803.	1799.	1798.	1799.	1792.	1791.	1790.
10% FA C	.2	4	6	6	4	4	2	7	1	7	3	4
Good Dirty 75% RAP		1818.	1810.	1800.	1800.		1792.	1791.	1788.		1782.	1782.
10% FA D	.0	5	2	9	2	2	3	5	0	1	7	4

Good Dirty 75% RAP	1823	1799.	1796.	1786.	1788.	1781.	1778.	1777.	1779.	1773.	1770.	1768.
10% FA E	.9	8	17 30.	9	1700.	7	7	1	0	8	5	8
Good Dirty 75% RAP	1740		1692.	1676.	1666.	1655.	1645.	1635.	1631.	1623.	1619.	1609.
10% FA F	.2	8	7	3	7	3	8	8	5	9	3	0
Good Dirty 75% RAP	1820		1787.	1776.	1774.	1767.	1762.	1757.	1757.	1750.	1747.	1745.
10% FA	.7	8	3	4	0	7	9	9	1	7	7	4
Good Dirty 50% RAP	1836	1811.	1794.	1789.	1777.	1767.	1756.	1751.	1745.	1741.	1740.	1734.
10% FA A	.5	9	8	7	6	9	6	9	8	3	4	9
Good Dirty 50% RAP	1908	1882.	1869.	1858.	1849.	1841.	1832.	1828.	1822.	1817.	1814.	1810.
10% FA B	.1	7	7	7	4	1	4	7	7	8	4	9
Good Dirty 50% RAP	1919		1890.	1884.	1875.	1870.	1861.	1855.	1845.	1841.	1843.	1838.
10% FA C	.3	5	4	2	7	3	7	4	8	9	4	4
Good Dirty 50% RAP	1881	1863.	1849.	1845.	1838.	1832.	1827.	1823.	1820.	1817.	1818.	1814.
10% FA D	.2	7	9	0	8	1	6	4	0	0	3	7
Good Dirty 50% RAP	1884		1842.	1831.	1823.	1820.	1812.	1803.	1792.	1788.	1789.	1785.
10% FA E Good Dirty 50% RAP	.5 1828	7 1779.	7 1758.	8 1741.	8 1732.	0 1719.	6 1706.	4 1697.	8 1682.	6 1676.	1 1674.	3 1666.
10% FA F	.0	1779.	3	7	9	17 19.	5	8	4	7	4	6
Good Dirty 50% RAP	1876	1849.	1834.	1825.	1816.	1808.	1799.	1793.	1784.	1780.	1780.	1775.
10% FA	.3	3	3	2	4	4	6	4	9	6	0	1
Good Dirty 25% RAP	1912	1889.	1877.	1876.	1869.	1863.	1861.	1860.	1854.	1852.	1847.	1846.
12% FA A	.4	5	0	5	6	4	1	1	2	2	0	6
Good Dirty 25% RAP	1895	1857.	1826.	1793.	1779.	1763.	1757.	1753.	1746.	1743.	1736.	1733.
12% FA B	.2	7	9	3	6	2	2	2	1	8	4	8
Good Dirty 25% RAP	1911	1889.	1876.	1874.	1867.	1860.	1860.	1862.	1855.	1853.	1848.	1846.
12% FA C	.0	7	9	1	4	1	1	8	8	5	7	9
Good Dirty 25% RAP	1910	1891.	1878.	1875.	1868.	1861.	1860.	1860.	1853.	1848.	1843.	1841.
12% FA D	.3	0	9	4	0	0	3	1	8	9	6	3
Good Dirty 25% RAP 12% FA E	1921 .0	1904. 4	1892. 3	1891. 0	1883. 9	1874. 4	1874. 5	1868. 6	1860. 8	1859. 6	1851. 3	1849. 4
Good Dirty 25% RAP	1871	1852.	1837.	1834.	1823.	1815.	1812.	1805.	1795.	1792.	1775.	4 1772.
12% FA F	.5	7	0	5	4	2	2	3	8	8	6	0
		1880.		1857.						1825.		
12% FA	.6	8	8	5	7	6	6	0	8	1	1	0
Good Clean 75% RAP	1987	1978.	1978.	1975.	1975.	1970.	1977.	1976.	1976.	1978.	1976.	1979.
7% Cem A	.4	7	4	1	6	3	5	1	8	1	7	3
Good Clean 75% RAP	2073	2062.	2061.	2056.	2054.	2049.	2052.	2050.	2049.	2061.	2049.	2052.
7% Cem B	.5	7	2	7	6	5	4	3	3	4	6	1
Good Clean 75% RAP	2074	2067.	2067.	2064.	2063.	2056.	2059.	2058.	2059.	2050.	2059.	2062.
7% Cem C	.4	7	0	1	0	3	5	6	6	5	8	1
		2075.	2072.	2067.			2064.	2064.	2064.		2064.	2067.
7% Cem D	.2	4	7	4	9	7	2	2	0	9	0	2
		2050.	2047.	2045.			2041.	2041.	2041.		2040.	2045.
7% Cem E	.8	4	6	3	2	3	8	4	6	9	9	0
Good Clean 75% RAP		2031.	2029.	2026. 0	2025. 0	2019. 7	2021. 5	2022.	2022. 6	2023.	2022.	2024. 5
7% Cem F	.6		3	_	_			2025	_	2036.		
Good Clean 75% RAP	2053	ZU44.	2042.	2039.	2037.	2032.	2036.	2035.	2035.	2036.	2035.	2038.

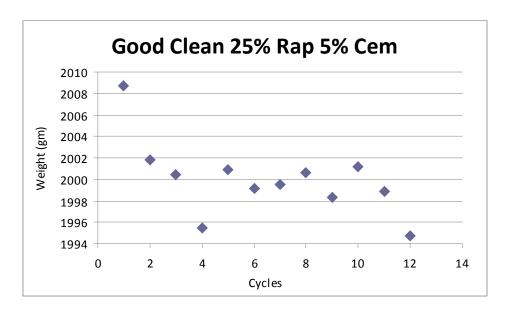
7% Cem	.5	3	7	1	9	8	2	5	7	8	5	4
Good Clean 50% RAP	2020		2010.	2007.	2004.	2004.	2010.	2009.	2010.	2012.	2012.	2009.
7% Cem A	.8	0	9	7	8	8	6	7	3	0	3	2
Good Clean 50% RAP	2016	-	2007.	2005.	1999.	1999.	2005.	2004.	2004.	2004.	2005.	2002.
7% Cem B	.8	0	7	2	0	0	0	1	1	8	7	0
Good Clean 50% RAP	1983	1980.	1975.	1975.	1971.	1971.	1976.	1974.	1975.	1977.	1978.	1974.
7% Cem C	.1	8	4	9	1	1	2	5	4	7	8	7
Good Clean 50% RAP	2035		2023.	2020.	2017.		2022.	2020.	2020.	2022.	2023.	2019.
7% Cem D	.2	2	7	3	6	6	7	1	0	2	1	9
Good Clean 50% RAP	2032	2031.	2026.	2024.	2021.	2021.	2025.	2025.	2025.	2028.	2029.	2026.
7% Cem E	.9	0	3	7	9	9	6	6	6	9	5	2
Good Clean 50% RAP	1988	1984.	1982.	1980.	1975.	1975.	1980.	1978.	1980.	1980.	1981.	1978.
7% Cem F	.5	2	0	8	4	4	6	3	2	6	0	6
Good Clean 50% RAP	2012	2009.	2004.	2002.	1998.	1998.	2003.	2002.	2002.	2004.	2005.	2001.
7% Cem	.9	2	3	4	3	3	5	1	6	4	1	8
Good Clean 25% RAP	2020	2013.	2012.	2006.	2011.	2010.	2010.	1977.	1975.	1977.	1974.	1971.
5% Cem A	.7	9	8	2	5	1	4	6	7	4	3	3
Good Clean 25% RAP	2037	2027.	2027.	2020.	2027.	2025.	2026.	2011.	2008.	2011.	2010.	2005.
5% Cem B	.3	1	3	6	8	9	6	3	0	7	1	2
Good Clean 25% RAP	1992	1986.	1983.	1979.	1984.	1983.	1981.	1984.	1982.	1984.	1982.	1978.
5% Cem C	.2	6	4	8	3	5	8	6	0	6	9	9
Good Clean 25% RAP	2003	1996.	1996.	1990.	1995.	1994.	1996.	1997.	1995.	1997.	1994.	1991.
5% Cem D	.6	4	3	9	6	7	4	4	2	3	0	9
Good Clean 25% RAP	2012	2005.	2004.	2000.	2005.	2003.	1978.	2005.	2003.	2006.	2004.	1999.
5% Cem E	.5	5	1	0	8	3	3	3	1	8	6	9
Good Clean 25% RAP	1986	1981.	1978.	1975.	1980.	1977.	2003.	2027.	2025.	2029.	2027.	2021.
5% Cem F	.1	6	8	1	5	4	6	8	9	0	2	2
Good Clean 25% RAP	2008		2000.	1995.	2000.	1999.	1999.	2000.	1998.	2001.	1998.	1994.
5% Cem	.7	9	5	4	9	2	5	7	3	1	9	7
Good Clean 75% RAP	1775	1743.	1725.	1720.	1700.	1694.	1682.	1667.	1655.	1651.	1647.	1633.
12% FA A	.7	1	0	8	5	6	4	1	4	6	4	2
Good Clean 75% RAP 12% FA B	1812 .8	1786. 7	1768. 5	1770. 6	1751. 4	1744. 1	1734. 8	1713. 3	1698. 7	1680. 6	1675. 4	1569. 4
Good Clean 75% RAP		1631.	1606.	1594.	1569.		1530.	1498.	1485.	1477.	1469.	1529.
12% FA C	.6	8	8	0	0	6	5	4	3	1477.	1409.	8
	1818		1762.	1758.	1737.		1715.	1699.	1693.		1685.	1618.
12% FA D	1.1	1704.	3	6	17.57.	2	8	2	7	2	3	3
Good Clean 75% RAP	1751	-	1696.	1685.	1666.	1652.	1639.	1614.	1595.	1589.	1579.	1620.
12% FA E	.2	5	0	4	2	3	5	8	7	3	4	9
Good Clean 75% RAP		1733.	1708.	1708.	1690.	1680.	1673.	1658.	1646.		1634.	
12% FA F	.1	1	2	1	9	1	6	7	1	5	9	7
Good Clean 75% RAP		1732.	<u>-</u> 1711.	1706.	1685.	1675.	1662.	1641.	1629.		1615.	1597.
12% FA	.8	7	1	3	9	0	8	9	2	7	3	4
Good Clean 50% RAP	1573	1439.	1341.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA A	.9	1	1									
Good Clean 50% RAP	1496	1307.	1152.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA B	.3	6	9									
			i						1		1	1

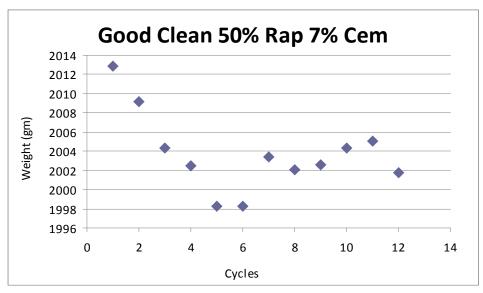
Good Clean 50% RAP	1633	1515.	1408.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA C	.3	6	9									
Good Clean 50% RAP	1553		1238.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA D Good Clean 50% RAP	.8 1763	5 1732.	3 1718.	Х	Х	Х	Х	Х	Х	Х	Х	х
12% FA E	.5	8	5	^	^	^	^	^	^	^	^	^
Good Clean 50% RAP	1579	1472.	1335.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA F	.6	1	2									
Good Clean 50% RAP	1600		1365.	Х	Х	Х	Х	Х	Х	Х	Х	Х
12% FA Good Clean 25% RAP	.1 1750	5 1714.	8 1674.	1658.	1644.	1633.	1626.	1607.	1595.	1594.	1586.	1573.
12% FA A	1730	4	6	8	1	5	2	2	5	2	7	8
Good Clean 25% RAP	1739	1692.	1648.	1620.	1603.	1562.	1556.	1535.	1521.	1520.	1516.	1513.
12% FA B	.2	5	5	9	4	0	5	9	4	0	7	0
Good Clean 25% RAP	1722	1665.	1614.	1579.	1552.	1531.	1518.	1490.	1475.	1472.	1462.	1457.
12% FA C Good Clean 25% RAP	.9 1792	9 1737.	9 1686.	7 1656.	0 1630.	9 1616.	9 1609.	5 1585.	2 1572.	3 1569.	0 1567.	4 1565.
12% FA D	.0	6	7	1030.	3	9	7	8	3	7	8	5
Good Clean 25% RAP	1781	1743.	1686.	1658.	1637.	1623.	1610.	1592.	1581.	1580.	1575.	1574.
12% FA E	.7	8	6	8	2	2	2	1	9	7	8	3
Good Clean 25% RAP	1763		1676.	1655.	1627.	1615.	1602.	1587.	1573.	1572.	1568.	1560.
12% FA F Good Clean 25% RAP	.3 1758	1 1712.	6 1664.	1 1638.	7 1615.	0 1597.	7 1587.	5 1566.	6 1553.	4 1551.	5 1546.	5 1540.
12% FA	.2	9	7	2	8	1	4	5	3	6	3	8
Poor Dirty 75% RAP	1995	1986.	1981.	1976.	1977.	1973.	1973.	1969.	1971.	1966.	1967.	1963.
5% CEM A	.0	7	5	0	3	2	3	3	7	8	3	2
Poor Dirty 75% RAP	1991	1986.	1981.	1978. 7	1979. 7	1976. 2	1978.	1974.	1973. 3	1972. 4	1970.	1967. 4
5% CEM B Poor Dirty 75% RAP	.4 1999	1 1991.	6 1988.	1984.	1987.	∠ 1981.	5 1983.	4 1978.	3 1979.	4 1977.	3 1976.	4 1972.
5% CEM C	.6	3	6	8	1	4	9	9	4	3	6	2
Poor Dirty 75% RAP	1964	1953.	1951.	1945.	1947.	1942.	1942.	1937.	1937.	1934.	1934.	1930.
5% CEM D	.8	9	2	8	1	6	2	2	8	0	7	1
Poor Dirty 75% RAP 5% CEM E	2000	1993. 1	1990. 1	1985. 2	1985. 4	1978. 5	1980. 9	1976. 0	1973. 9	1972. 5	1971. 6	1968. 2
Poor Dirty 75% RAP	1971		-	1954.	1956.			1945.	1945.		1944.	
5% CEM F	.4	0	9	9	3	2	0	8	9	8	5	5
Poor Dirty 75% RAP	1987		1975.	1970.	1972.	1967.	1968.	1963.	1963.		1960.	1957.
5% CEM	.1	0	8	9	2	4	1	6	7	3	8	1
Poor Dirty 50% RAP 3% CEM A	2124 .9	1976. 0	1939. 2	1925. 4	1915. 1	1911. 6	1903. 5	1899. 2	1892. 0	1875. 2	1871. 9	1865. 3
Poor Dirty 50% RAP	2076		1870.	1864.	1856.	1854.	1848.	1844.	1841.	1834.	1825.	1819.
3% CEM B	.5	3	5	7	6	6	9	2	2	7	3	5
Poor Dirty 50% RAP		1950.	1926.	1924.	1906.	1898.	1883.	1877.	1868.		1853.	
3% CEM C	.9	8	3	2	0	2	4	4	1	2	8	1
Poor Dirty 50% RAP 3% CEM D	2137 .1	1958. 9	1945. 6	1942. 7	1933. 7	1933. 2	1926. 0	1922. 9	1914. 1	1905. 9	1896. 0	1891. 1
Poor Dirty 50% RAP		9 1877.	1845.	1833.	1815.	1804.	1792.	1788.	1784.		1752.	1745.
. 551 51119 5576 11741	2010	.011.	1040.	.000.	.010.	.007.	52.	50.	5	., 55.	1702.	1 , 40.

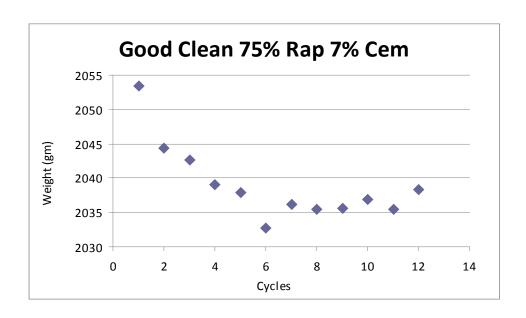
3% CEM E	.9	3	6	2	3	4	2	7	1	1	4	3
Poor Dirty 50% RAP	.9 2083	3 1895.	1868.	∠ 1860.	3 1848.	4 1839.	∠ 1829.	1826.	1819.	1810.	1806.	1803.
3% CEM F	.5	8	0	0	1040. 5	8	7	6	5	1010.	0	2
Poor Dirty 50% RAP	2099	1927.	1899.	1891.	1879.	1873.	1864.	1859.	1853.	1841.	1834.	1828.
3% CEM	.8	0	2	7	2	6	0	8	2	2	2	3
Poor Dirty 25% RAP	2125		2003.	1993.	1993.	1983.	1973.	1954.	1943.	1933.	1922.	1911.
3% CEM A	.3	2006.	2003.	1993.	1993.	8	1973.	8	1943.	7	1922.	4
	.3 2076		1940.	1925.	1922.	1907.	1901.	1884.	1874.		1851.	1836.
Poor Dirty 25% RAP 3% CEM B	.7	1942. 7	1940.	1925.	3	1907.	6	9	1074.	5	2	5
Poor Dirty 25% RAP	2103	1969.	1972.	1952.	3 1953.	1939.	1927.	1904.	1887.	1872.	1859.	1838.
3% CEM C	.3	1969. 7	0	1952. 7	1955.	7	6	1904.	0	9	9	9
			1969.	1954.	1950.	1937.	1926.	1906.	1891.	1880.	1870.	1853.
Poor Dirty 25% RAP 3% CEM D	2103 .8	1969.	0	1954. 5	1950.	1937.	1926.	1906.	7	4	8	0
	.o 2121	1994.	1996.	1979.	4 1976.	1959.	1949.	1932.	1922.	1904.	1893.	1878.
Poor Dirty 25% RAP 3% CEM E	.1	1994.	1996.	0	1976.	1959.	3	1932.	3	0	2	2
	2106			1973.	1976.	4 1965.		1945.			1922.	
Poor Dirty 25% RAP 3% CEM F		1984.	1984.	1973.	1976.	1965.	1956. 9		1939. 7	1927.	3	1910.
	.1	-	8		•		×	0		0	3 1886.	8
Poor Dirty 25% RAP 3% CEM	2106	1977.	1977. 5	1963.	1962.	1948. 9	1939. 2	1921.	1909.	1896. 6		1871.
	.1	9		1	0			3	8	-	6	5
Poor Dirty 75% RAP	1802	1718.	1701.	1678.	1662.	1646.	1637.	1625.	1620.	1612.	1608.	1602.
15% FA A	.2	0	3	1	1	5	0	1	8	8	7	6
Poor Dirty 75% RAP	1734		1617.	1586.	1579.	1559.	1539.	1533.	1518.	1510.	1506.	1496.
15% FA B	.5	4	4	6	0	7	2	0	9	3	6	9
Poor Dirty 75% RAP	1695	1616.	1580.	1545.	1516.	1494.	1471.	1457.	1447.	1433.	1424.	
15% FA C	.7	0	2	1	8	5	5	9	7	7	8	2
Poor Dirty 75% RAP	1749	1670.	1638.	1602.	1578.	1562.	1527.	1507.	1486.	1473.	1463.	1441.
15% FA D	.2	1	7	8	0	3	5	4	9	1	2	8
Poor Dirty 75% RAP	1797	1720.	1711.	1695.	1686.	1678.	1669.	1662.	1653.	1642.	1638.	1632.
15% FA E	.7	5	2	3	5	6	3	6	3	1	5	7
Poor Dirty 75% RAP	1795	1744.	1731.	1712.	1704.	1693.	1688.	1682.	1679.	1672.	1673.	1668.
15% FA F	.3	6	0	7	3	4	6	9	2	9	7	7
Poor Dirty 75% RAP	1762	1685.	1663.	1636.	1621.	1605.	1588.	1578.	1567.	1557.	1552.	1541.
15% FA	.4	4	3	8	1	8	9	2	8	5	6	5
Poor Dirty 50% RAP		1660.	1634.	1615.	1609.	1595.	1591.	1582.	1582.		1566.	1559.
15% FA A	.4	7	9	7	7	5	1	5	0	8	0	3
Poor Dirty 50% RAP	1797		1741.	1722.	1714.	1701.	1697.	1687.	1683.	1674.	1663.	1660.
15% FA B	.0	6	0	6	1	7	7	0	0	7	1	1
Poor Dirty 50% RAP	1786		1727.	1712.	1706.	1693.	1688.	1680.	1681.		1664.	1660.
15% FA C	.4	3	3	6	2	6	5	6	0	7	5	3
Poor Dirty 50% RAP	1822		1770.	1756.	1747.	1734.	1729.	1717.	1714.		1703.	1698.
15% FA D	.9	2	9	9	3	8	3	5	9	8	0	9
Poor Dirty 50% RAP			1749.	1736.	1728.	1720.	1717.	1709.	1710.		1695.	1692.
15% FA E	.1	1	2	3	0	2	6	9	3	7	7	9
Poor Dirty 50% RAP	1736		1668.	1655.	1651.	1635.	1636.	1629.	1630.	1623.	1615.	1609.
15% FA F	.9	1	8	9	0	4	4	7	3	9	0	6
Poor Dirty 50% RAP	1781		1715.	1700.	1692.	1680.	1676.	1667.	1666.	1660.	1651.	1646.
15% FA	.0	5	4	0	7	2	8	9	9	6	2	9

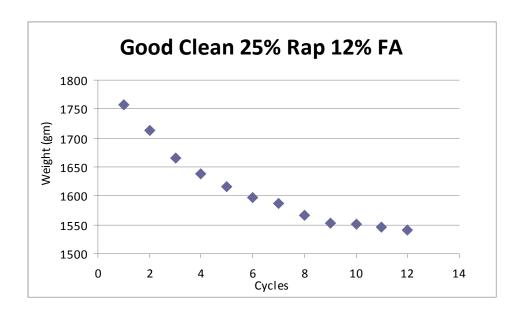
Poor Dirty 25% RAP	1855	1815.	1805.	1797.	1787.	1782.	1784.	1772.	1775.	1769.	1754.	1750.
15% FA A	.2	7	4	9	5	8	9	6	7	3	0	2
Poor Dirty 25% RAP	1849	1793.	1774.	1766.	1763.	1759.	1757.	1747.	1746.	1743.	1720.	1715.
15% FA B	.1	7	3	5	0	9	3	3	3	0	6	6
Poor Dirty 25% RAP	1781	1729.	1714.	1705.	1700.	1694.	1694.	1680.	1680.	1677.	1662.	1657.
15% FA C	.8	3	2	8	0	7	3	0	0	6	9	3
Poor Dirty 25% RAP	1818	1777.	1764.	1750.	1744.	1737.	1742.	1728.	1730.	1724.	1701.	1692.
15% FA D	.6	0	0	2	1	0	1	6	5	6	9	4
Poor Dirty 25% RAP	1842	1812.	1797.	1785.	1779.	1771.	1770.	1761.	1764.	1759.	1744.	1740.
15% FA E	.1	3	3	6	0	4	6	3	4	4	2	5
Poor Dirty 25% RAP	1801	1753.	1735.	1726.	1715.	1702.	1687.	1678.	1678.	1674.	1658.	1657.
15% FA F	.6	9	9	8	4	7	3	2	4	9	8	1
Poor Dirty 25% RAP	1824	1780.	1765.	1755.	1748.	1741.	1739.	1728.	1729.	1724.	1707.	1702.
15% FA	.7	3	2	5	2	4	4	0	2	8	1	2

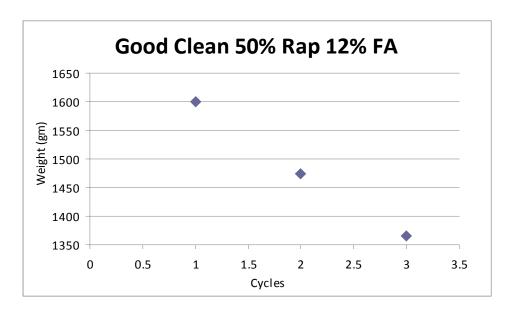
Good Clean TST samples

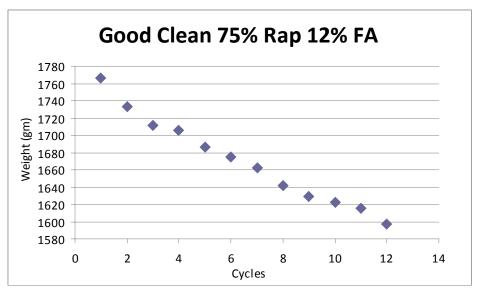


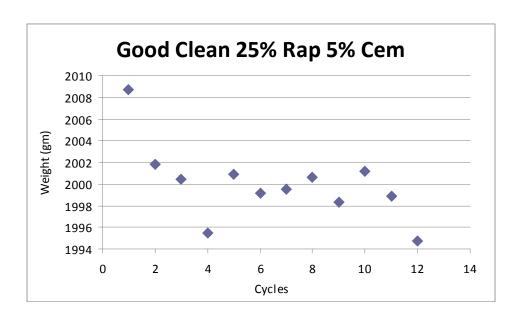


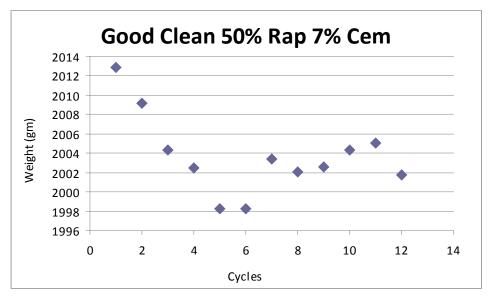


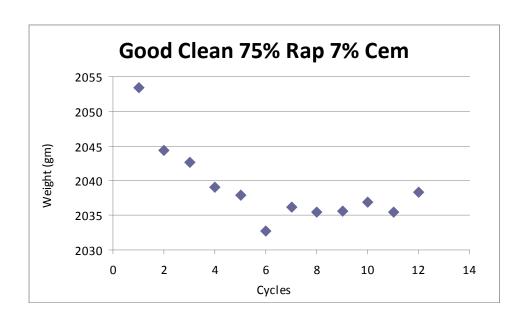


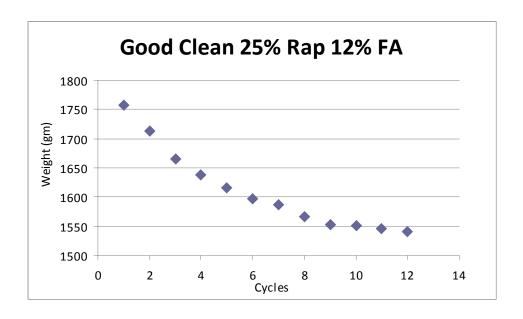


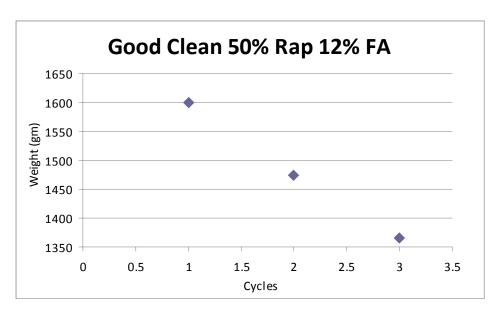


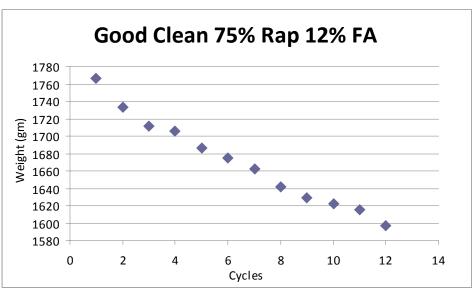




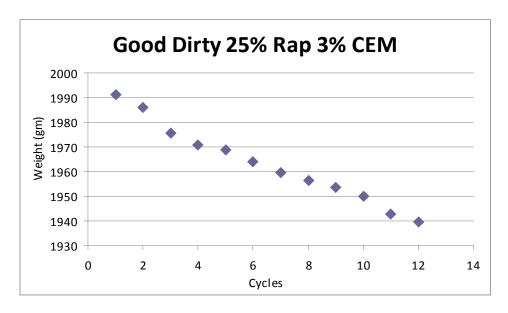


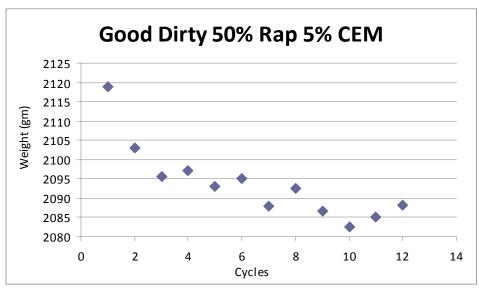


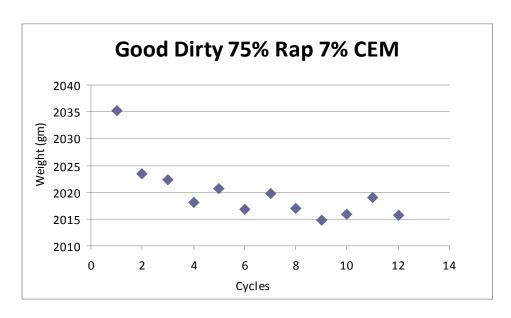


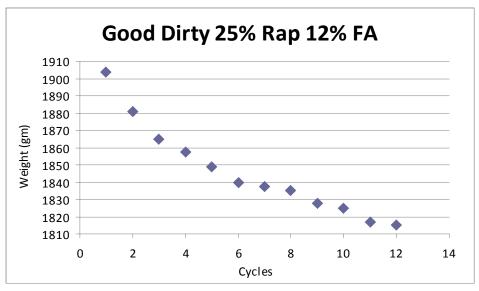


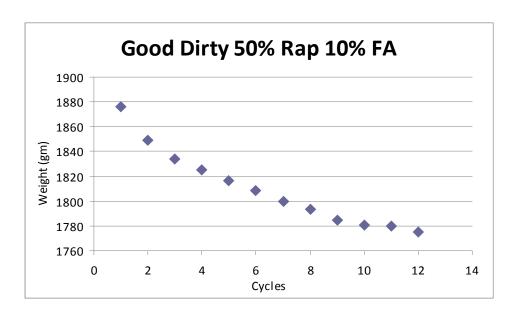
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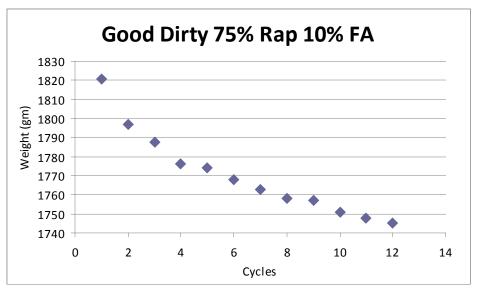


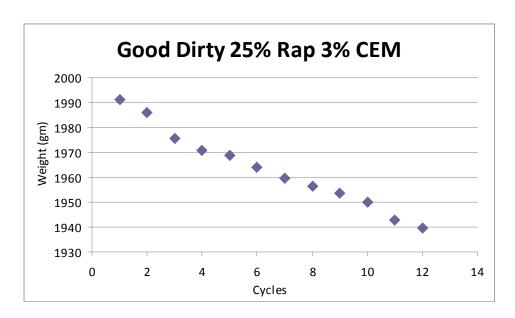


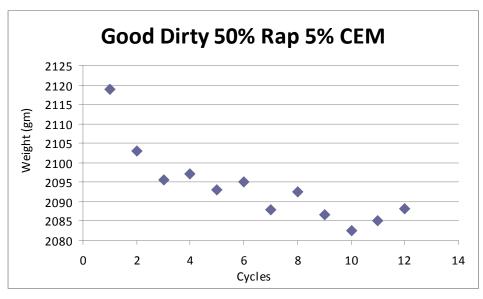


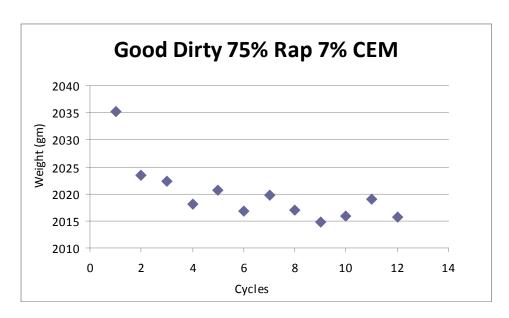


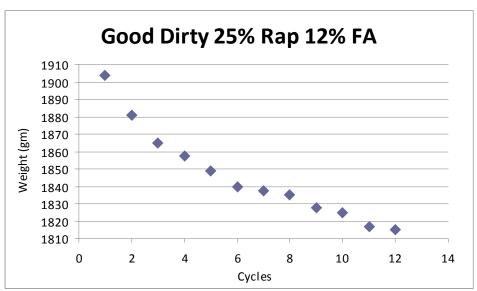


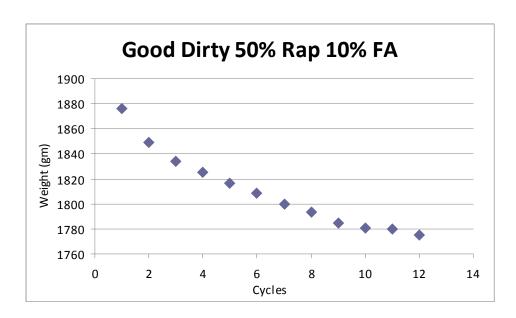


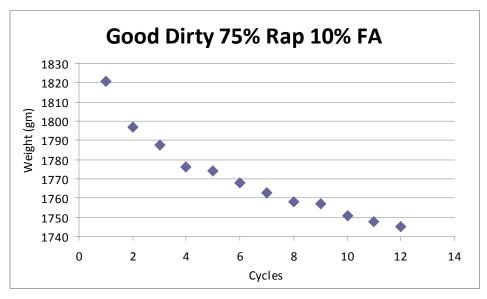




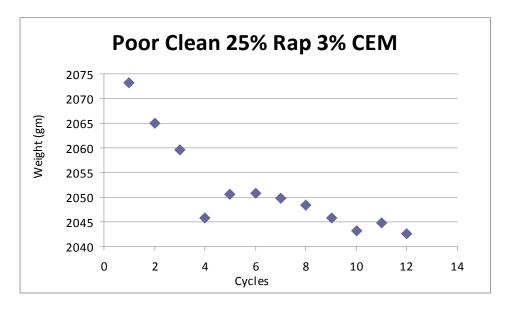


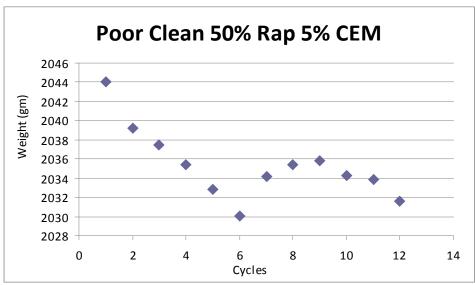


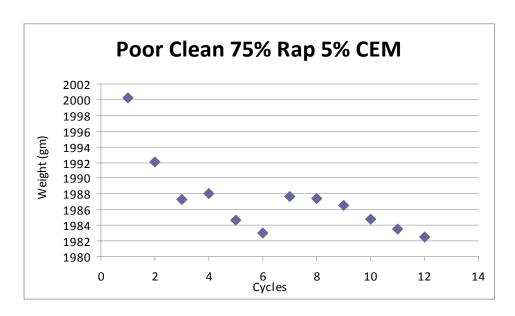


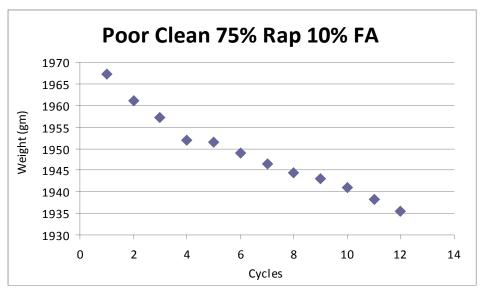


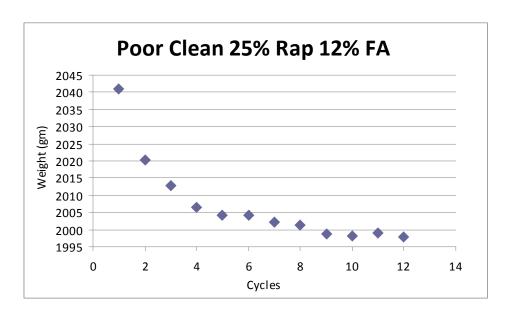
Poor Clean

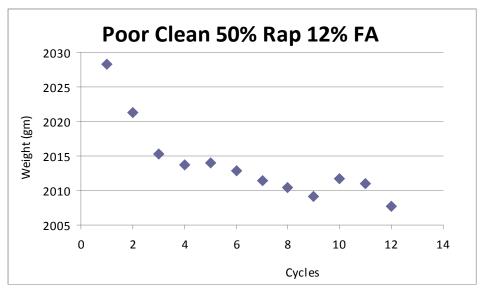


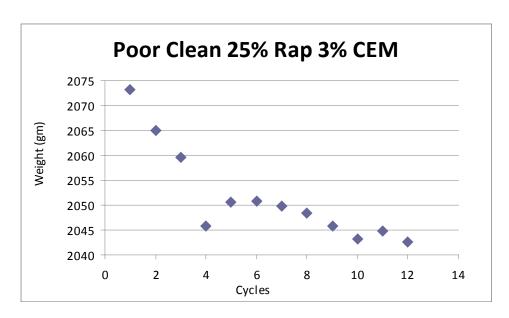


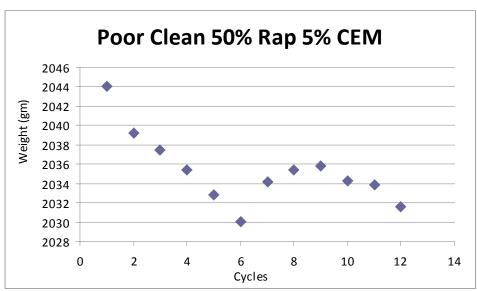


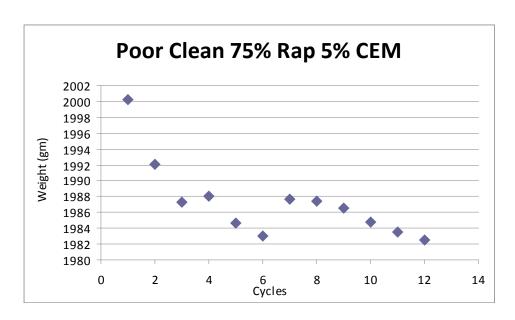


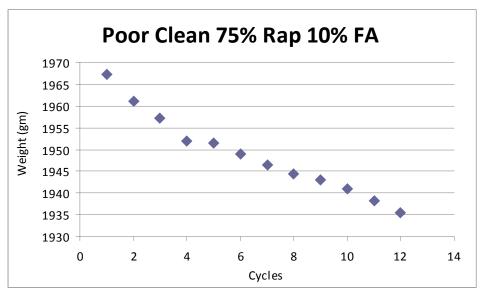


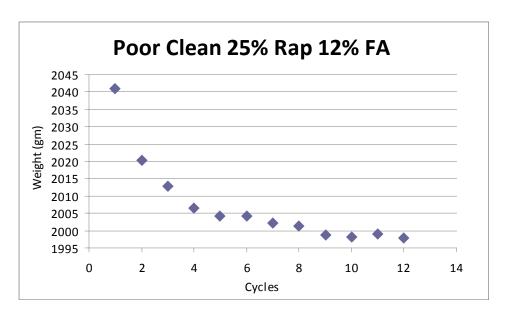


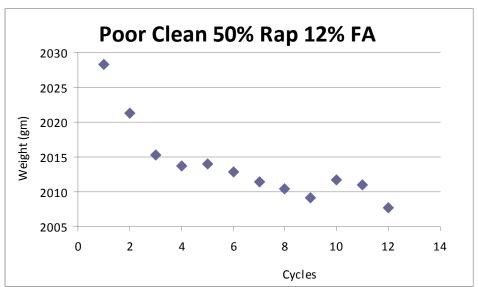












Poor dirty

