

INDEPENDENT STUDY







Aggregates Background



MODULE



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Important Note

You must complete Module D Lesson 1 and Lesson 2 before visiting your state or district laboratory in Module D Lesson 3.

Using This Independent Study Workbook

This self-guided workbook contains the information you need to complete this lesson. Throughout the workbook, instructions are provided that explain how to complete each section. Following the instructions provided ensures that you will successfully complete the independent study (IS) lesson. Make sure that you read all required information, complete exercises, document observations, and answer knowledge check questions as instructed.

Be sure to have your completed workbook available when you attend the Web-conference training (WCT) portion of this module, as the information and your answers, observations, and findings will be reviewed and discussed.

3	Lesson 2 Introduction
G	Welcome to Module D: Aggregates for Transportation Construction Projects, Lesson 2: Aggregates Background. By the end of this lesson, you will be able to:
	 List common uses of aggregates in transportation projects;
	 Define common aggregate-related terms;
	 Describe the most important aggregate properties for general transportation-related applications;
	 Explain the most important aggregate quality characteristics for Portland cement concrete (PCC), asphalt concrete (AC), and pavement support layers; and
	 Identify standard test methods for evaluating aggregates.
	This lesson will take approximately 1 hour to complete.

	Lesson Resources
U	To complete this lesson, you will need the following resources:
	 Your agency's standard specifications for highway and bridge construction (or the equivalent)
	 American Association of State Highway and Transportation Officials (AASHTO) Standard Specifications for Transportation Materials and Methods of Sampling and Testing, 33rd (or most current) Edition and AASHTO Provisional Standards, 2013 (or most current) Edition
	 American Society for Testing and Materials (ASTM) Annual Book of Standards, Volume 04.02 (most current), Concrete and Aggregates
	The following references are not required reading but may be something you wish to consult for additional information regarding the specific study topic.
	 ASTM Volume 04.03 (most current), Road and Paving Materials; Vehicle- Pavement Systems
	 National Cooperative Highway Research Program (NCHRP) Report 557: Aggregate Tests for Hot-Mix Asphalt Mixtures Used in Pavements
	 NCHRP Report 453: Performance-Related Tests of Aggregates for Use in Unbound Pavement Layers
	 NCHRP Report 539: Aggregate Properties and the Performance of Superpave-Designed Hot Mix Asphalt
	 NCHRP Report 555: Test Methods for Characterizing Aggregate Shape, Texture, and Angularity
	 NCHRP Report 405: Aggregate Tests Related to Asphalt Concrete Performance in Pavements
	You will likely have these materials, but if not, they are available in your agency library or materials laboratory.



Instructions

Read the Common Uses of Aggregates in Transportation Projects section and answer the knowledge check questions that follow. Knowledge check questions will be reviewed and discussed during the Web-conference portion of this module.

49	Common Uses of Aggregates in Transportation Projects
U	Aggregates are used for many varied purposes in transportation projects, ranging from providing pavement support to serving as a key component in both Portland cement concrete and asphalt mixtures.
	The specifications governing aggregate selection and properties differ substantially based on the end use. The majority of these aggregates are naturally occurring (igneous, metamorphic, and sedimentary) and undergo various processing steps to comply with specific end-use requirements. Synthetic or manufactured aggregates are also used in some cases to either supplement or replace natural aggregates to meet a specific project goal.
	The most common uses of aggregates in transportation systems are:
	PCC mixtures
	 Asphalt mixtures (hot mix asphalt (HMA), warm mix asphalt (WMA), cold patch, etc.)
	Pavement base coarse (stabilized and unbound)
	Pavement subbase coarse
	Select borrow for fill sections
	Riprap for erosion control
	Surface for gravel roads
	Chip seal

















6	Knowledge Check
	Circle the correct answer.
_	Which of the following are considered primary uses of aggregates in transportation systems?
	a) PCC pavements
	b) HMA pavements
	c) Base coarse materials
	d) All of the above
	e) None of the above

2	Knowledge Check
	Circle the correct answer.
	Which of the following represents the most widely used base coarse material for rigid (PCC) pavements?
	a) Cement treated base
	b) Lean concrete base
	c) Unbound aggregate base
	d) Open graded, unbound permeable base
	e) Asphalt treated base



Instructions

Read the Aggregate Terminology section. This terminology will be used throughout the HMEC.

Aggregate Terminology

There are many terms used to classify and differentiate aggregates. The most common classification schemes are based on aggregate gradation (AASHTO and ASTM). Virtually all classification schemes are related to one or more of the following physical properties:

- Gradation
- Particle shape
- Aggregate type
 - Source or mineralogy
 - Natural or synthetic (manufactured)
 - Virgin or recycled

In this section, we are going to cover common terminology used to describe aggregates. Note that the terms used in Module D are in common use, although some regional differences may exist.



Aggregate Terminology Related to Gradation

Based on common gradation parameters, the following terms are used:

- Coarse aggregate: Aggregate predominantly retained on the 4.75 mm (#4) sieve. The maximum aggregate size used for transportation projects (other than riprap) is generally less than 3 inches.
- Fine aggregate: Aggregate predominantly passing the 4.75 mm (#4) sieve and retained on the 0.075 mm (#200) sieve.
- Fines: Sometimes used to designate fine aggregate or the very fine material that passes the 0.075 mm (#200) sieve.
- Maximum size and nominal maximum size: The following table illustrates and compares the definitions for nominal maximum aggregate size and maximum aggregate size.

	Nominal Max. Size	Maximum Size
ASTM C 125 Concrete	In a Spec – 100% <u>Permitted</u> to Pass	In a Spec – 100% <u>Required</u> to Pass
AASHTO M 323 <i>HMA</i>	One Size Larger Than Size That Retains More Than 10%	Next Size Larger Than Nominal Max.

Note the subtle differences in the definition of maximum and nominal maximum aggregate size. This distinction is very important as these terms are sometimes incorrectly used. For example, in the American Concrete Institute (ACI) 211.1 mix design procedure, the calculations are based on the nominal maximum aggregate size.

The ASTM definition is based on the specification limits for the aggregate product. The AASHTO definition in M 323 for Superpave HMA is based on the actual grading of the aggregate material.













5	Knowledge Check
	Circle the correct answer.
	Coarse aggregates are generally differentiated from fine aggregates based on what standard sieve size?
	a) #200
	b) #16
	c) #4
	d) ¾ inch

5	Knowledge Check
	Circle the correct answer.
	Which of the following terms are used to describe the total production of a rock- crushing operation (generally passing a specific screen size)?
	a) Bank-run
	b) Crushed stone
	c) Crushed gravel
	d) Crusher-run
	e) River gravel



2	Knowledge Check
	Circle the correct answer.
	Which of the following is not considered a manufactured or synthetic aggregate?
	a) Expanded shale
	b) Blast furnace slag
	c) Pumice
	d) Steel slag





Instructions

Read the Important Aggregate Properties section and answer the knowledge check questions that follow. This section lists the most commonly specified aggregate properties and their relative importance for different end uses.

Important Aggregate Properties

The desirable properties of aggregates vary somewhat by the intended use. In other words, aggregates suitable for PCC may not necessarily be the best choice for AC or base coarse materials.

The most frequently specified aggregate properties include the following:

- Gradation
- Strength
- Particle shape
- Surface texture
- Durability (abrasion resistance, hardness)
- Reactivity
- Volumetric stability (coefficient of thermal expansion)

These general categories encompass the majority of aggregate specifications and test protocols.

The following three tables show the relative importance of physical, chemical, and mechanical properties of aggregates based on their intended use.

Relative Importance of Physical Aggregate Properties

The following table shows the relative importance of common physical properties of aggregates based on their end use.

	Property	РСС	AC	Base Coarse
	Particle shape – angularity	М	V	V
	Particle shape – flakiness, elongation	М	V	М
	Particle size – maximum	М	М	М
	Particle size – distribution	М	М	М
	Particle surface texture	М	V	V
	Pore structure, porosity	V	М	U
	Specific gravity, absorption	V	М	М
	Soundness – weatherability	V	М	М
	Unit wt, voids – loose, compacted	V	М	М
	Volumetric stability thermal	М	U	U
	Volumetric stability wet/dry	М	U	М
	Volumetric stability freeze/thaw	V	М	М
	Integrity during heating	U	М	М
	Deleterious constituents	V	М	М
N r L	 V = Very important M = Moderately important J = Unimportant or importance unknow 	n		

Base Coarse

	Relative Importance of Chemical Aggregate Properties						
U	The following table shows properties of aggregates b	ble shows the relative importance of commo gregates based on their end use.		nmon chemical			
	Property	PCC	AC	Base Coarse			
	Solubility	М	U	U			
	Surface charge	U	V	U			
	Asphalt affinity	U	V	М			

Solubility	Μ	U	U
Surface charge	U	V	U
Asphalt affinity	U	V	М
Reactivity to chemicals	V	U	U
Volume stability – chemical	V	М	М
V = Very important			
M = Moderately important			
U = Unimportant or importance unknown			

	Relative Importance of Mechanical Aggregate Properties					
U	The following table shows the relative importance of common mechanical properties of aggregates based on their end use.					
	Property	PCC	AC	Base Coarse		
	Compressive strength	Μ	U	U		
	Toughness – impact resistant	М	М	U		
	Abrasion resistance	Μ	Μ	М		
	Character of products of abrasion	М	М	U		
	Mass stability – stiffness, resilience	U	V	V		
	Polishability	Μ	Μ	U		
	V = Very important M = Moderately important U = Unimportant or importance unknown					

5	Knowledge Check
	Circle the correct answer.
	We have discussed many physical properties of aggregates. Which of the following is the most important property for use in an unbound aggregate base coarse?
	 a) Particle shape b) Coefficient of thermal expansion c) Porosity d) All of the above e) None of the above

Knowledge Check

Circle the correct answer.

Elongated aggregate particles (particle shape) are considered very important for which of the following?

- a) PCC
- b) AC
- c) Base coarse material
- d) Subbase material
- e) All of the above

Knowledge Check

Circle the correct answer.

The compressive strength of aggregates is considered important for which of the following?

- a) PCC
- b) AC
- c) Base coarse material
- d) Subbase material
- e) All of the above



Overview of AASHTO and ASTM Test Procedures

In this section, we are going to look at a number of different test procedures specified by AASHTO and ASTM that are used to characterize the physical properties of aggregates.

In the laboratory session (Lesson 3), you are going to perform or observe a number of the more frequently performed tests.

The AASHTO and ASTM test method numbers are provided for many of the tests listed. As you read through this section, you should prepare a list of these test standards and read through the actual test protocols in the standards books to familiarize yourself with the procedures.



Standard Test Methods for Evaluating Aggregates

Aggregate characterization generally involves performing a number of physical, mechanical, or chemical tests conducted on representative material samples.

The end use of the aggregates and the relative importance of the project may dictate which tests are performed.

In Lesson 3, you will actually be performing or observing many of the tests highlighted in this section. Complete details regarding these and other test procedures will be provided at that time.

The most frequently performed tests are physical characterization. The majority of central and field laboratories perform these on a routine basis. Chemical and mechanical testing is much more specialized and these tests are not as frequently performed.

In this section, we will look at some of the basic aggregate characterization tests.

- Physical properties
 - Grading, shape, angularity, texture
 - Density, pore properties
 - Others depending on intended use
- Chemical properties
 - Affinity for asphalt, bond with PC paste
 - Reactivity
- Mechanical properties
 - Resistance to applied loads (strength and deformation)

Particle Versus Bulk Aggregate Properties	
hough many of the typical aggregate tests are performed on bulk or nbined aggregates, there are circumstances where individual particles are ted.	
 Individual particles (rock, minerals) 	
 Specific gravity (particle density) 	
 Absorption, pore properties 	
 Strength, hardness, toughness (dry, wet) 	
 Volume stability (thermal, moisture) 	
Bulk aggregate (collection of particles)	
 Grading, size, minus #200, cleanliness 	
 Unit weight (bulk density), void content 	
 Moisture content 	



Obtaining Representative Samples

Determining realistic aggregate properties requires tests to be performed on representative samples of either individual or combined aggregates.

Obtaining representative samples consists of using appropriate sampling techniques for the specific application (i.e. stockpiles, conveyor belts, silos, etc.) and splitting the samples to aid in uniformity.

- Sampling of Aggregates, AASHTO T 2 (ASTM D 75)
- Reducing Samples of Aggregates to Testing Size, AASHTO T 248 (ASTM C 702)

It should be noted that in order to obtain representative test results, proper sampling and sample splitting are key.



Grading Aggregates Aggregate grading is performed on either individual or combined aggregate samples. Sieve Analysis, AASHTO T 27 (ASTM C 136) • Individual size fractions - Cumulative percent passing (basis for typical specification) - Cumulative percent retained (for fineness modulus (FM) determination) Maximum aggregate size* Nominal maximum aggregate size* Minus #200 fraction (75 μm) - Dry sieve - Wash loss, AASHTO T 11 *The maximum and nominal maximum aggregate sizes differ by the intended aggregate use and are defined in the following slides. Gradation is one of the most important parameters regardless of intended use. These procedures will be demonstrated in the laboratory portion of Lesson 3.







	Aggregate Cleanliness				
	The cleanliness of the aggregates may have a significant influence on the combined aggregate properties as previously mentioned. The tests indicated above will be demonstrated in the laboratory portion of Lesson 3.				
	Cleanliness of aggregates can also be measured by methods other than minus #200, such as those listed in the slide for fine and coarse aggregates. For graded aggregates, the initial sample is often dry sieved on coarse aggregate sieves first, and then a split of the minus #4 material is tested as a fine aggregate.				
	Fine aggregate methods include sand equivalent, plasticity index, and the methylene blue value test to detect the presence of active clays in the fines.				
	For coarse aggregate material, minus #200 can be determined by washing. Other cleanliness tests include adherent fines and the California cleanliness value (CV).				
	Fine aggregate fraction				
	 Minus #200 sieve (as previously described) 				
	 Sand equivalent, AASHTO T 176 (ASTM D 2419) 				
	 Plasticity index, AASHTO T 90 (ASTM C 4318) 				
	 Methylene blue value, AASHTO T 330 				
	Coarse aggregate fraction				
	 Minus #200 sieve 				
	 Adherent fines, ASTM D 5711 				
	 Cleanliness value (CV), California Test Method 227 				



Fine Aggregate Shape, Angularity, and Texture

Aggregate shape, angularity, and texture affect the interaction of the aggregate particles as well as the void space that exists in both the loose and compacted states. This photo shows the effect of these characteristics in conjunction with slight variations in gradation. The lowest void ratio of the samples shown is the natural sand sample, which tends to be somewhat smooth and rounded in relation to the very angular and rough manufactured sand. As a general rule, the more rounded and smooth the particles are, the easier it is for them to reorient. For instance, consider the workability differences arising from using manufactured sand as opposed to natural sand in a typical paving PCC mixture.

The coarse aggregate shape, angularity and surface texture can be determined according to the following tests:

Uncompacted Void Content of Fine Aggregate (as Influenced by Particle Shape, Surface Texture, and Grading), AASHTO T 304







Coarse Aggregate Shape, Angularity, and Texture

This is an example of crushed stone. Note the angular shape and relatively rough surface texture. These particles do not reorient easily and therefore tend to be relatively stable compared to those in the previous slide. Note that the compactive effort required to achieve good density, and therefore stability, may be quite high.





Coarse Aggregate Shape, Angularity, and Texture

This is an example of blast furnace slag. Note the angular or near cubical shape and the rough and porous surface texture.





Strength and Durability

There are various strength parameters used to describe aggregates. It is difficult to measure strength of individual aggregate particles; and, for many common uses, strength itself does not greatly influence performance. Examples of uses where strength is not overly important include aggregate base and subbase, aggregate for moderate strength Portland cement concrete, and aggregate for asphalt base and binder layers.

Example tensile strengths of aggregate materials are shown here. Aggregate particle strength can be important in some applications, such as for high strength concrete, surface coarses on heavily traveled pavements, and aggregate interlock for load transfer in plain-jointed concrete pavement. Generally, the strength of aggregate is evaluated based on the performance of a mixture containing the aggregate, from previous experience using the aggregate in similar applications, or from subjective petrographic evaluation of the competence of minerals and rocks present.

The following list includes some of the more common strength and durability characteristics of aggregates:

- Compressive strength
 - 4,000 to 70,000 psi
- Tensile strength
 - 30 to 380 psi (Shale)
 - 300 to 1,200 psi (Limestone)
 - 500 to 2,300 psi (Quartzite)
- Hardness
- Impact resistance toughness
- Wear, degradation, abrasion wet or dry
- Polishing pavement friction

Aggregate strength of a bulk sample can be determined according to the following test:

Determining the Resilient Modulus of Soils and Aggregate Minerals, AASHTO T 307



Compressive Strength

The compressive strength of the rocks and minerals comprising typical aggregates vary widely. Although the strengths may vary by a substantial amount, most sources of aggregates may be successfully utilized by identifying the end use and service environment. Strength is not generally measured or specified for aggregates used in construction per se. The following values were determined from rock core strength tests.

- Quartzite 21,000 to 68,000 psi
- Basalt 8,000 to 44,000 psi
- Traprock (Gabbro) 15,000 to 34,000 psi
- Limestone 7,000 to 39,000 psi
- Granite 4,000 to 40,000 psi
- Sandstone 5,000 to 16,000 psi
- Shale 5,000 to 13,000 psi
- Low Strength cinders, coral, lightweight aggregate

The compressive strength of rock cores can be determined by the following test:

Standard Test Methods for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperatures, ASTM D 7012



Hardness

Hardness can be measured in many ways. Weaker, softer aggregate particles may perform well in some applications or climates. However, if the weakness is due to high porosity or the presence of planes of weakness due to clays or mineral cleavage planes, then they may be subject to freezing and thawing or chemical durability problems. Hard aggregates generally provide better abrasion resistance and better wet-weather friction on pavement surfaces.

A number of methods are used to determine the hardness of aggregates. The most frequently used procedures are shown below:

- Scratch Mohs scale of hardness
- Polished stone value British wheel equivalent

Aggregate hardness can be determined according to the following tests:

Moh's Hardness (No AASHTO designation)

Accelerate Polishing of Aggregates Using the British Wheel, AASHTO T 279



Abrasion, Wear, and Degradation

The abrasion, wear, and degradation of both fine and coarse aggregates are important properties for materials when subjected to repeated traffic loadings. Normally, the primary concern is for the degradation of coarse materials. However, fine aggregate, with its much greater surface area, can degrade with a resulting increase in the amount of minus #200 fines. This can happen during aggressive mixing or handling procedures where the particles of fine aggregate abrade one another, for example, during PCC mixing.

Aggregate particles behave differently under wet and dry conditions and therefore are tested under both conditions. The following tests represent the current standards for measuring the abrasion, wear, and degradation characteristics of both coarse and fine aggregates.

- Dry (Los Angeles Abrasion Test)
 - Small coarse aggregate less than 1½ inches

AASHTO T 96 (ASTM C 131)

- Large coarse aggregate greater than ¾ inch
 ASTM C 535
- Wet
 - Micro Deval Test
 - ASTM D 7428 08e1, Standard Test Method for Resistance of Fine Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus
 - AASHTO T 327, Standard Method of Test for Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus
 - ASTM D 6928 10, Standard Test Method for Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus



Durability

Pores in aggregate particles and in rock are distributed in a size range that is related to mineral composition, the method of creation of the rock, and weathering. Another important characteristic is the interconnection structure of the pores in rock or aggregate particles. Pores that are isolated will not transmit fluids as readily as those that are interconnected and can allow water and ions to penetrate or pass through the rock material. Pores that become saturated are more vulnerable to freezing and thawing damage in concrete and to stripping in hot mix asphalt. The faster transmission of ions in aggregate particles can speed alkali-aggregate reactions in concrete and speed corrosion of reinforcing steel in concrete if chlorides are present.





Porosity of Various Aggregates

Pore structure of aggregate particles can range in size from a few angstroms (which is smaller than a water molecule) up to very large holes of 1 mm or more, which can be easily seen without magnification. Pore size distributions have been determined down to 0.01 micrometers by mercury intrusion methods. As a frame of reference, 1 mm = 1,000 micrometers. An example of pore size distributions for several aggregate sources is shown. The sizes of pores that are generally thought to cause freeze-thaw durability problems are those in the 0.1 to 5 micrometer range. Smaller pores are too small for water to freeze in great quantity, and large pores are somewhat like air entrainment in PCC. The larger voids rarely become fully saturated with water.





Volume Stability of Aggregate Particles, Thermal Movement

Volume stability of aggregate particles is most important in PCC but must be considered in base/subbase materials or AC mixtures if large volume changes may occur. Volume change of aggregates or mixtures can be caused by several factors—thermal, moisture, chemical reactions.

- PCC: The range of values for coefficient of thermal expansion (CTE) of PCC ranges from approximately 0.000004 to 0.000007 inch per inch per °F. This value is influenced primarily by aggregate type and to a lesser extent gradation. Limestone aggregates tend to have a lower CTE (.000002); however, some coarse grained limestone can be as high as .000004. Quartz and aggregates with high silica contents are generally much higher (approximately 0.0000065). The CTE of PCC is determined by AASHTO T 336, "Coefficient of Thermal Expansion of Hydraulic Cement Concrete."
- AC: Aggregate thermal properties are not considered important in AC mixtures. Under most temperature conditions, asphalt is flexible and thermal movements are generally accommodated. However, at very cold temperatures, much of the flexibility is lost and thermal cracking can result in asphalt pavement as it cools and contracts.
- Base: Thermal expansion is not important in unbound base and subbase materials. However, cement stabilized bases may be subject to the same considerations as PCC at higher cement contents and strengths (i.e. lean concrete bases).



Volume Stability of Aggregate Particles, Chemical Reactions

Volume changes may occur due to chemical reactions (i.e. alkali silica reactions in PCC) or physical processes, such as volume increases due to water freezing in the pore structure of the aggregates (i.e. D-cracking in PCC).

Steel slag (different from blast furnace slag) aggregates can experience significant volume changes due to hydration reactions (ASTM D 4792) in which free calcium and magnesium expand in volume when exposed to water.

For this reason, steel slag should not be used in concrete. For use in HMA and in bases/subbases, the slag must be wetted, cured, and aged to make sure the expansion reactions have run their full course before use in construction— particularly construction in a confined space.



Volume Stability of Aggregate Particles, Moisture-Induced Movement

Volume changes may also be the result of moisture fluctuations. Although these changes may be present in AC and bases/subbases, their effects are minimal and are usually disregarded. However, in the case of PCC, these changes may be significant and are assessed by the following methods.

- AASHTO T 160 (ASTM C 157) Length Change of Hardened Hydraulic Cement Mortar and Concrete
- AASHTO T 334-08 (2012) Estimating the Cracking Tendency of Concrete



Activity: State Standards Compared to AASHTO/ASTM Standards

In Lesson 1, you compiled a table of information regarding your agency standards. Now, look up and review the various AASHTO and ASTM standards listed in this lesson. You should take the time now to flag them for easy reference during Lesson 3. Note that when you begin preparation for Lesson 3, there will be additional specifications to review.

In this activity, you will compare your State standards with the AASHTO/ASTM standards. In the majority of cases, they will be the same since most agencies base their specifications on AASHTO or ASTM. Since not all standards will be the same, note any differences regarding test protocols and recommended values. These differences may be in regard to general aggregate properties or they may be specific to intended use, i.e. PCC, AC, bases, and subbases. Document these variations in the following table. There are a few examples included.

The table is not intended to be all inclusive of your agency standards but will provide a framework for what is requested in this exercise.

Material Property	Intended Use	AASHTO or ASTM Requirement	Agency Requirement	Notes
Gradation – Minus #200	Base coarse	AASHTO T11	Hydrometer analysis	Determine characteristics of minus #200 material
Cleanliness	НМА	AASHTO 176 (Sand Equivalent) or AASHTO T 330 (Methylene Blue Index)	Methylene blue	Predicts stripping potential of AC/aggregate
Volume Stability – CTE	РСС	AASHTO T 336	Not performed in laboratory, estimated based on aggregate type	Used to estimate CTE of the PCC

Intended Use	AASHTO or ASTM Requirement	Agency Requirement	Notes
	Intended Use	Intended Use AASHTO or ASTM Requirement Image:	Intended Use AASHTO or ASTM Requirement Agency Requirement Image: Constraint of the second secon

Material Property	Intended Use	AASHTO or ASTM Requirement	Agency Requirement	Notes

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Material Property	Intended Use	AASHTO or ASTM Requirement	Agency Requirement	Notes

Material Property	Intended Use	AASHTO or ASTM Requirement	Agency Requirement	Notes

