SPS Traffic Site Evaluation – Pilots Summary and Lessons Learned

Federal Highway Administration Turner-Fairbank Highway Research Center Pavement Performance Division, HRDI-13 6300 Georgetown Pike McLean, Virginia 22101

May 2, 2002



U.S. Department of Transportation **Federal Highway Administration**



Long-Term Pavement Performance Serving your need for durable pavements

1 Executive Summary	1
2 Introduction	2
3 Standards	3
3.1 Trucks	3
3.2 Static Weights	4
3.3 Spacing	4
3.4 Temperatures	
3.5 Speeds	8
3.6 Equipment Readiness	9
3.6.1 Weight calibration	10
3.6.2 Classification	11
3.7 Pavement Smoothness	11
3.7.1 Short wavelength	12
3.7.2 Long wavelength	13
4 Logistics	13
4.1 Coordination	14
4.2 Schedules and Duration	14
4.3 Trucks and Drivers	15
4.4 Staffing	16
4.4.1 Data Collection	16
4.4.2 Smoothness	17
4.5 Go/ No Go Decisions	17
5 Analysis	18
5.1 Smoothness	19
5.2 Weight Criteria	19
6 Summary	22
7 Site Specific Results	23
7.1 Arizona	23
7.2 Florida	24
7.3 Maryland	
7.4 Michigan	
7.5 Texas	
8 Raw data	29
8.1 Arizona	29
8.2 Florida	30
8.3 Maryland	43
8.4 Michigan	
8.5 Texas	

List of Tables

Table 3-1 WIM System Calibration Tolerances	
Table 3-2 Computed axle spacing means and confidence intervals in feet	4
Table 3-3 Computed axle spacing means and confidence intervals expresses in	
error	
Table 3-4 Smoothness estimate	
Table 7-1 Overall AZ site 0600 results	
Table 7-2 Overall FL site - Piezo-electric WIM results	
Table 7-3 Overall FL site - Bending Plate WIM results	
Table 7-4 Overall MD 0500 site results	
Table 7-5 Overall MI 0100 site results	
Table 7-6 MI site results with selected trucks	
Table 7-7 Overall TX 0100 results	
Table 8-1 AZ Static Test Truck measurements	
Table 8-2 AZ raw data for LTPP lane	
Table 8-3 FL Static Test Truck Measurements	
Table 8-4 FL Piezo Lane 1 – Raw data for Truck 1	
Table 8-5 FL Piezo Lane 1 – Raw data Truck 2	
Table 8-6 FL Piezo Lane 2 – Raw data Truck 1	
Table 8-7 FL Piezo Lane 2 – Raw data Truck 2	
Table 8-8 FL Bending Plate Lane 1 – Raw data Truck 1	
Table 8-9 FL Bending Plate Lane 1 – Raw data Truck 1	
Table 8-10 FL Bending Plate Lane 4 – Raw data Truck 1	
Table 8-11 FL Bending Plate Lane 4 – Raw data Truck 2	
Table 8-12 MD Static Test Truck Measurements	
Table 8-13 MD Raw data for LTPP lane	
Table 8-14 MI Static Test Truck Measurements	
Table 8-15 MI Raw data for LTPP lane	
Table 8-16 TX Static Test Truck Measurements	
Table 8-17 TX Raw data for LTPP lane	

List of Figures

Figure 3-1 Temperature variation AZ, FL, TX	6
Figure 3-2 Temperature variation - MD, MI	6
Figure 3-3 GVW sensitivity to temperature	7
Figure 3-4 Piezos with different temperature sensitivities	8
Figure 3-5 Speed distributions - AZ, FL n.b., MD	8
Figure 3-6 Speed distributions - FL s.b., MI, TX	9
Figure 3-7 Tandem axle weight variability with speed	9
Figure 4-1 Flow diagram of SPS traffic pooled fund activities	15
Figure 5-1 AZ speed/temperature combinations	20
Figure 5-2 FL speed/temperature combinations	
Figure 5-3 MD speed/temperature combinations	21
Figure 5-4 MI speed/temperature combinations	21
Figure 5-5 TX speed/temperature combinations	

SPS Site Evaluations – Pilots Summary and Lessons Learned	May 2, 2002
	Page iv
Figure 6-1 Method for slab length determination	
c c	

1 Executive Summary

The Long-Term Pavement Performance program has embarked on a nationwide effort to improve the quality and quantity of traffic data available. The effort is focused on the Specific Pavement Studies experiments involving new construction and pavements with asphalt overlays, SPS-1, -2, -5 and -6. Due to concerns about the availability of research quality loading data for analysis purposes, the LTPP program has proposed to assume the responsibility for traffic collection using the FHWA pooled fund mechanism. States may pay to have as much or as little of their SPS traffic collection done by LTPP rather than using internal agency resources.

As is customary for LTPP data collection activities, a series of protocols and guidelines has been developed in order to acquire uniformly collected, research quality data. The guidelines include a suggested performance based equipment specification for equipment replacement, a performance specification for validating equipment operation in the field and a pavement smoothness criteria. In addition, a field protocol addressing the calibration and validation procedures has been prepared.

Over the summer and fall of 2001, LTPP staff oversaw a series of pilots to verify that the performance specifications and field procedures were feasible. The pilots looked at both piezo and bending plate sensor systems installed in asphalt and PCC pavements. Included in the pilots was one site to test the reinstallation process and a site to do a side by side comparison of the principal sensor systems, piezo and bending plate.

As a result of the pilots the following occurred:

- Verification that the equipment performance specifications are achievable with current practice and technology.
- Determination that the smoothness specification is too restrictive for actual field conditions and requires revision.
- Confirmation that the recommendation of bending plate sensors in smooth PCC with bending plate sensors does produce research quality data.
- Validation of the recommended field practices including the conditions for vehicles, speeds and temperatures.

2 Introduction

The Specific Pavement Studies (SPS) projects in experiments 1, 2, 5, 6 and 8 are the subject of a concentrated effort to obtain research quality traffic loading data. For the SPS-1, -2, -5, and -6 projects this effort focuses on acquiring weight data which meets the performance standards for gross vehicle weight and axle information in ASTM E 1318 Type I WIM systems¹. In contrast, the SPS-8 experimental sections are expected to have minimal truck traffic. The inclusion of this experiment is intended to provide a resource for verifying light loads or for obtaining information on sites which have a greater number of trucks than originally expected when the site was built.

The effort to upgrade traffic data collection for selected SPS experiments began in 1999 with input from the TRB LTPP Committee Traffic Expert Task Group (Traffic ETG). They noted that scarcity of traffic data as well as lack of knowledge about the limited data available constituted a deficiency in the ability of the LTPP program to get the maximum value out of the investment in the SPS-1, -2, -5 and 6 experiments. As a result, FHWA directed that a change in the traffic data collection process for these experiments be considered. The recommendation adopted was for a centrally run program for equipment installation, maintenance and data collection with a recommended equipment standard. States have the opportunity to participate in a pooled fund study that implements the recommended program. The fund is structured so that a state may select which, if any, of the services it wishes to use.

As with any other LTPP data collection effort managed by FHWA, the pooled fund study required the development of equipment and pavement guidelines and data collection protocols. Inputs to the guidelines and protocols included the professional experience of the practitioners who constitute the Traffic ETG, FHWA staff and contractors and ASTM E-1318. It was the consensus of the Traffic ETG that the most likely installation to meet the specified accuracy requirements for research needs over a range of operating conditions was bending plate in portland cement concrete. The guidelines reflect that consensus. The suggested equipment guidelines are, however, performance based rather than equipment specific. The pavement guidelines include a sample grinding specification and smoothness specifications that address both short wavelength and long wavelength conditions. All of the guidelines were made available to the traffic data collection community prior to writing the data collection protocols. The data collection protocols developed include practices for initial site evaluation, on-going site validation, and installation and construction acceptance conditions.

To test the various protocols and guidelines, five pilot studies were conducted in the summer and fall of 2001. The pilots concentrated on site validation. It was shown that the documents developed were essentially correct as written. The accuracy requirement for weights for Type I equipment in ASTM E-1318 can be met with equipment currently in operation. Pavement smoothness makes a difference. The sensitivity of piezo ceramic sensors to temperature variation was observed. Greater variability of piezo sensors when compared to bending plate sensors under side by side testing was observed. The need to have a consistent means for

¹ Highway Weigh-In-Motion (WIM) Systems with User Requirements and Test Methods, 1994.

measuring and documenting the quality of WIM data being collected was a significant finding as the pilots progressed. It was not possible to evaluate a new installation beginning with the contracting process so some uncertainty remains in the recommendations for those phases of data collection.

3 Standards

The requirements for a site to be accepted for research data collection are shown in Table 3-1. Since the weight data is the only information that is required for use in pavement analysis work, those conditions are the only ones that a site must meet. It was the recommendation of the Traffic ETG that failures with respect to vehicle speed or axle spacing were of more importance to classification issues than weight issues. Since the thrust was weight data collection, assuming accurate classification, the speed and spacing information was identified as a diagnostic measure rather than a performance criterion to define research quality weight data.

SPS-1, -2, -5, -6 and -8 Sites	95 Percent Confidence Limit of Error
Loaded single axles	<u>+</u> 20 percent
Loaded tandem axles	<u>+</u> 15 percent
Gross vehicle weights	+10 percent
Vehicle speed	<u>+</u> 1 mph [2 km/hr]
Axle spacing length	± 0.5 ft

Table 3-1 WIM System Calibration Tolerances

3.1 Trucks

Site validation can be done using test trucks of the traffic stream. The traffic stream provides a better representation of site conditions. However, the logistics of providing reliable static comparison weights at the vast majority of the target sites precluded adoption of this as the recommended method. Adoption of the test truck method resulted in a two truck minimum fleet. Two trucks were considered the minimum to insure that a truck with unusual characteristics did not result in flawed data after the validation was completed.

For purposes of site to site comparisons, a 3S2 (5-axle tractor, semi-trailer combination) was selected as the standard vehicle. It needs to have an air suspension system on all tandem axles. The air suspension should be in good condition. This is qualitative, as no quantitative measure for this aspect has been defined to date. This vehicle should be loaded to approximately 80,000 pounds with a load that will not shift. This truck was selected as being the most critical to pavement evaluations with the suspension giving it the best chance of passing. The second vehicle should represent the predominant truck contributing to pavement loading at the site. It may be a 3S2 with the same weight and a different suspension, a heavier vehicle in states with higher or seasonal load limits, another 3S2 with an air suspension and a much lighter load, or a different axle configuration or another vehicle entirely. The ASTM E-1318 recommendation of a 2-axle vehicle as the second truck was not adopted. The one light 2-axle vehicle used behaved entirely differently from the other two vehicles, a fact attributed primarily to site roughness. The merit of excluding dump trucks was partially vindicated by

one site. However, the dump used at another site did not produce results that much different from the 3S2 used at the site.

3.2 Static Weights

Locating certified scales within a reasonable distance of the sites has not been a problem.

To counter the impact of shifting loads (even though they are not supposed to) and the effect of fuel usage on weight, iterative vehicle weighing is done before and after the validation process. To overcome the inability to determine where a weight change occurred in the absence of axle scales a multi-pass weighing procedure was adopted. It involves moving the truck across a platform scale so that individual axle weights can be either measured or calculated. Three separate measurements of gross vehicle weight (GVW) are also required. This helps to account for variability in the static weights. It also helps to determine any problems with trucks prior to commencing a validation session. A standard for a successful pre-validation static weighing session was set as the standard deviation of the GVW being one-third or less of the tolerance required for the equipment to pass the weight criterion (three and a third percent). For the post-validation static weighing only a single GVW measurement is required if the change from the pre-validation weight is less than 1,000 pounds.

3.3 Spacing

The spacing criterion was taken directly from ASTM E-1318. As shown in Table 3-2 the criterion appears reasonable as a test of within group (i.e. tandem) spacing. However, between group spacings, with two exceptions all exceed the 0.5 foot criterion, five of them by nearly a factor of two. Changing the spacing computations to include all spacings does not increase the number of sites meeting the criterion, as shown by the number of shaded cells.

State	Within Group	Between Group	All spacings
AZ	0.16 ± 0.45	0.74 ± 1.18	0.45 ± 1.05
FL n.b. piezo	-0.12 ± 0.10	0.16 ± 0.36	0.02 ± 0.38
FL s.b. piezo	-0.12 ± 0.10	0.16 ± 0.34	0.00 ± 0.38
FL n.b. bending plate	-0.15 ± 0.30	0.01 ± 0.98	-0.07 ± 0.74
FL s.b. bending plate	-0.03 ± 0.31	0.71 ± 1.33	0.34 ± 1.20
MD	-0.04 ± 0.11	-0.25 ± 0.37	-0.14 ± 0.34
MI	0.10 ± 0.60	0.30 ± 1.60	0.32 ± 1.61
TX	0.02 ± 0.32	0.14 ± 0.86	0.08 ± 0.65

Table 2.2 Commuted		and confidence intervals in f	
1 able 3-2 Computed	axie spacing means	s and confidence intervals in f	eet

Consideration should be given to the continued inclusion or revision of the spacing criterion. Either greater emphasis should be put on spacing calibration to obtain good classification results or a wider confidence interval identified. Failure to meet the spacing criterion is currently a non-fatal flaw. The impact of changes and the relationship to a larger tolerance needs to be addressed in terms of implications for classification failures, their diagnosis and multiple state classification algorithms. Alternatively, the criterion could be applied only to axle groups. It is interesting to note that two of the sites that failed due to variability of

weight with speed also failed to meet the criterion for within group axle spacing. The allowable variability, 0.5 feet, is the distance traveled in slightly under one-hundredth of a second at 35 mph or a fraction over five thousandths of a second at 65 mph.

One alternative that has been proposed is expressing the spacing failures as a percentage. If 0.5 feet is take as a percentage of the expected spacing for a tandem axle group, then the equivalent percentage is approximately 12 percent. Table 3-3 illustrates the application of this criterion. As compared to Table 3-2 the number of failures has been significantly reduced. This raises the question as to what the percentage is that should be used if a percentage error option is adopted.

State	Within Group	Between Group	All spacings
AZ	2.9 ± 8.1	2.9 ± 4.2	2.9 ± 6.5
FL n.b. piezo	-2.7 ± 2.3	0.5 ± 1.6	-1.1 ± 3.7
FL s.b. piezo	-2.9 ± 2.4	0.6 ± 1.5	-1.1 ± 3.9
FL n.b. bending plate	-3.4 ± 6.8	-0.1 ± 3.8	-1.8 ± 6.4
FL s.b. bending plate	-0.8 ± 7.2	2.6 ± 4.5	0.9 ± 6.9
MD	-1.0 ± 2.5	-1.15 ± 1.4	-1.0 ± 2.0
MI	2.0 ± 13.2	2.0 ± 10.1	2.0 ± 10.9
TX	0.2 ± 0.4	1.1 ± 2.2	0.6 ± 1.2

 Table 3-3 Computed axle spacing means and confidence intervals expresses in percentage error

3.4 Temperatures

One set of analyses examines the sensitivity of the scale system to changes in pavement temperature. The range of temperatures on site for any given day is effectively random. Overcast conditions, rain, start and end times and rush hour operations all affect the observed frequency and range. However, the goal of at least a 30 degree Fahrenheit range of pavement temperatures during a site visit is not unreasonable. Figure 3-3 and Figure 3-4 show support for both continuous temperatures and the minimum 30 degree range. With in each vertical line is a 15 degree range. Two consecutive ranges where data exists highlight temperature sensitivity problems.

Because the field test can not control actual pavement temperatures, the grouping process is dependent on the actual temperature observations obtained. The recommended grouping process splits the day's test runs into three groups. The difference between the high and low temperatures produces the range that is then divided into 3 evenly spaced bins. The number of points in each group is not necessarily equal. There are some instances in the pilots where only two bins could be justified due to the sample sizes resulting from a three bin split.

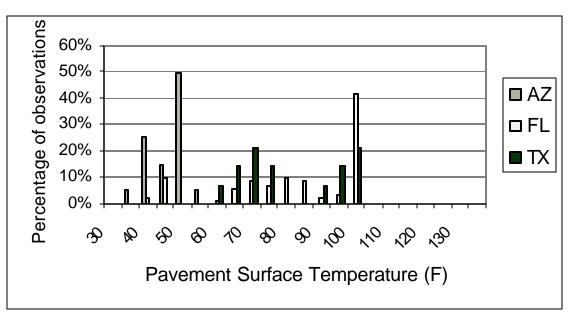
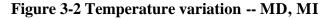
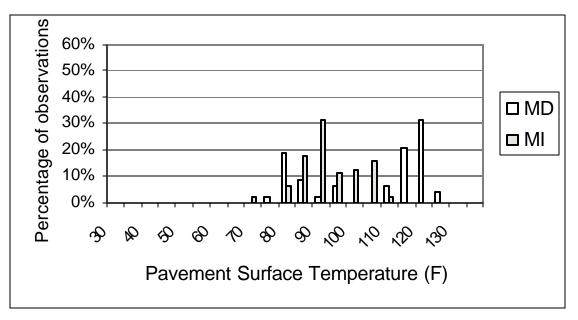


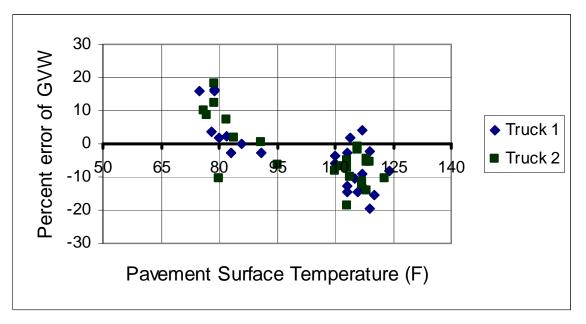
Figure 3-1 Temperature variation -- AZ, FL, TX

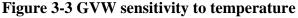
It was noted in measuring temperatures at several sites that a five to seven degree differential was possible between the shoulder where the temperature was measured and the center of the lane in the middle of the sensor. However no temperature variation was observed along the wheel path. Since temperature is a qualitative rather than a quantitative variable this is not a major issue. It does however emphasize the need to take all temperature measurements for a weighing session at the same location.





Temperature sensitivity is an issue with piezo sensors as shown in Figure 3-3. Continuing to get the widest possible range of temperatures remains important. For sites undergoing semiannual validation both hot and cold conditions can be captured. For the initial evaluation, the best indicator of a potential continuing temperature sensitivity problem is to validate on a day where a reasonably wide range of temperatures including an extreme are possible.





As the graph in Figure 3-4 shows this sensitivity does not automatically come with a piezo system. The sensors on either side of the road in Florida behaved very differently, possibly due to the mix of piezo sensor types in the northbound lane and the method by which temperature corrections were applied.

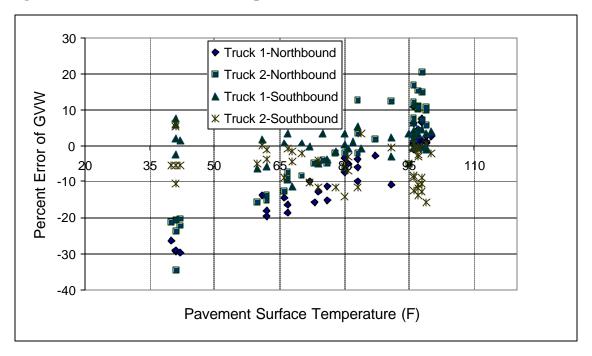
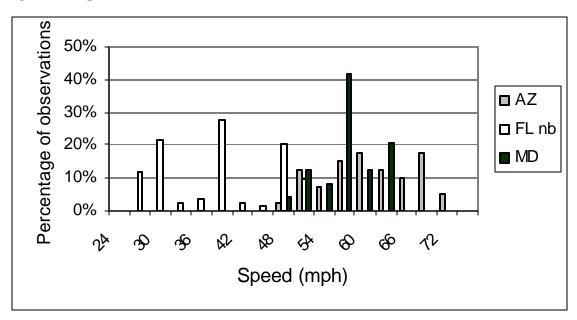


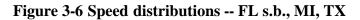
Figure 3-4 Piezos with different temperature sensitivities

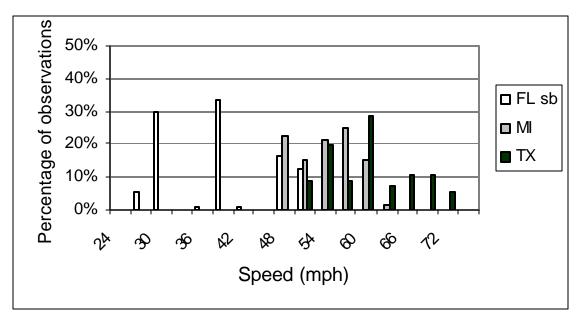
3.5 Speeds

Another set of analyses examines the sensitivity of the scale system to changes in vehicle speed. The collection requirement is that the range covers eighty percent of the commonly observed truck speeds. The interval between test run values must be 5 to 10 mph and have at least a 10 mph range from low to high. If the interval between each pair of values is more than 10 mph, it is expected that 4 speed groups rather than 3 will be used.

Figure 3-5 Speed distributions -- AZ, FL n.b., MD







Obtaining a range of speed remains important as observed in Figure 3-7 where variability in error increases with speed.

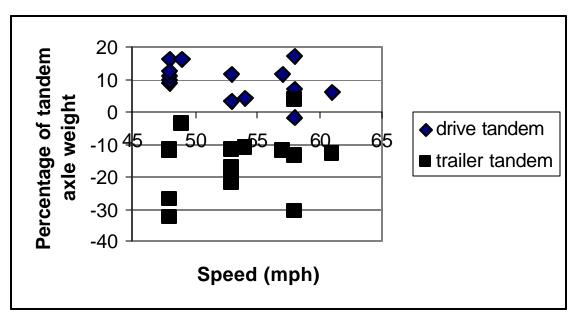


Figure 3-7 Tandem axle weight variability with speed

3.6 Equipment Readiness

Confirmation of equipment readiness comes from the state. Prior to the site visit, information on typical speed distributions and classification information is needed from the state. For the pilots this information was not formally obtained. For future visits getting this data in

advance is necessary. Additionally information on the exact location of working sensors where more than one set of sensors is in the roadway and the location of items like pull boxes would reduce the time spent on site.

It is expected that the equipment to be tested is operational. In the case of site evaluations following acceptance testing on replacement or new construction, the interval between acceptance and evaluations should be short enough that equipment could be presumed working. There is an acceptance test under the pooled fund study guidelines that newly installed equipment run for 15 consecutive days with less than 3 hours of down time.

Semi-annual preventive maintenance at the sites such as flushing drains, checking pull boxes and connections and wear in sensors or pavement deformation would contribute significantly to operational readiness and knowledge about working conditions.

Data validation, equipment diagnostics and calibration checks performed in the office at least monthly will provide an early indicator of problems.

3.6.1 Weight calibration

Each WIM system tested should be calibrated prior to the verification tests. Here should is the operative word. The assumption is that a calibration occurred in conjunction with the installation or system upgrade. For systems that require temperature adjustments to their calibration factors, a complete set of (calibrated) temperature adjustments should be in-place prior to the start of verification testing and/or the start of data collection for LTPP purposes. As a point of fact for all site evaluation cases a quick calibration check was done and calibration was performed before conducting the evaluation. Many sites had either never been calibrated or no record of the last calibration was available.

When doing a site evaluation, the first and possibly only LTPP visit to the site, provision must be made for calibrating the site. The preferred calibration practice for existing sites is to do a complete set of runs as for a validation, calibrate and then run the formal validation series. The initial validation series is necessary to provide a measure of the quality of the already collected WIM data. If the manufacturer has an alternative method, that can be used. In the case of continuing validations, running the full series before adjustment is required because there will be no other record (Sheet 16) as to exactly how the data drifted since the last validation visit. Whether or not a calibration is required at a site, a few runs at varying speeds with both trucks should be done to check the need for calibration before starting validation. Such a practice will identify immediately schedule and resource changes needed.

When sites must be calibrated a quick check after the calibration occurs should be done to verify the corrections.

The current field manual states that validation checks should be done with auto-calibration on or off according to actual data collection practice. It has been suggested that evaluation of piezo sensors should be done with auto-cal off. This does not replicate operating conditions and does not allow all systems to employ temperature correction surrogates they may use. While doing validations with auto-cal off provides a better representation of the equipment's function, it will not necessarily reflect the characteristics of the data provided over the monitoring period.

3.6.2 Classification

The vehicle classification algorithm to be used at each LTPP test site should be supplied by the SHA responsible for the operation of the roadway at that site. The expectation is that highway agencies have done all of the work in their equipment acceptance testing and site installation to verify the algorithms embedded in the equipment and their suitability to the site. Typically, the state of the practice is to have performed a complete multi-hour test and evaluation of that classification algorithm to ensure that it accurately classifies trucks at that site. However, the results of these pilots raise questions about the level of accuracy of the classification algorithm should have a higher level of emphasis than currently in the guidelines.

The current on-site test is not designed to fully test the algorithm. Instead, it is designed to ensure that the installed equipment is functioning correctly and that no mistakes have been made in the installation of a previously approved algorithm on that particular set of data collection electronics. The field test involves manually classifying vehicles crossing the WIM scale and comparing those classifications with the scale output.

The classifier is considered to be working acceptably when:

- 1) No more than 2 percent of the vehicles recorded are reported as "unclassified" by the WIM scale.
- 2) The number of classification errors involving <u>truck</u> classifications is less than 2 percent.

At those sites where a crude classification test was done, error rates were seven and a half percent or higher. This highlights a problem the data collection staff did not expect to encounter, potentially poor quality in the classification data currently provided to the LTPP program. In an ESAL based design methodology this may have less of an impact than in a method that depends on axle groups by vehicle class.

It was determined when doing the first site that it was not sufficient to merely check classification of random trucks. At that site a number of pickups hauling trailers were classified as tractor-trailers. At another site the data collection crew was surprised by the fact that a 6-bin rather than a 13-bin scheme was used. No on-site staff had a copy of the correspondence or the 6-bin scheme thus no classification verification could be done.

As a result of the experience with classification checks, the existing procedures do not appear sufficient to verify that accurate classification is occurring. A measure and a method to check each site on the initial visit should be developed.

3.7 Pavement Smoothness

Two types of failure possibilities were considered for smoothness: short wavelength and long wavelength. The ASTM E-1318 smoothness standard was considered and rejected as the LTPP criterion for short wavelength evaluation. The use of the sweep approach, the 20 foot

straight edge and the need for lane closure all led to developing a less intrusive and time consuming standard. The FHWA LTPP team minimizes the number of lane closures due to their impact on the travelling public and the constraints on state resources. As a result a high speed profiler alternative was developed for site evaluation and validation use. The proposed alternative collected 25mm profile and then used software to simulate a straightedge and a disk. For site construction acceptance a variant of the ASTM straightedge and disk method was proposed using a 12 foot straight edge and fixed wheel path and transverse locations was proposed.

For long wavelength analysis a vehicle simulation process is being used to generate data sets. The outcome of that process is to be an index that qualitatively describes the likelihood that a 3S2 vehicle passing over a calibrated system will meet the weight criteria.

3.7.1 Short wavelength

In developing the two methods (profiler and straightedge) it was assumed that they would produce the same or correlated answers. In fact they do not and one of the questions to be resolved is how to reconcile the results of two different methods designed to qualitatively evaluate the same thing.

All sites evaluated to date fail the all or nothing criterion, even those which meet the weight criteria. The question then arises, how rough is too rough or at what point is a failure reflected in variability with speed.

The Class I profiler alternative requires operators and equipment that comply with LTPP profiling protocols. The equipment is expected to record data at 25 mm intervals while travelling 80 kmph. Software was developed to simulate a 3.65 m (12 foot) straightedge and a 150 mm in diameter, 3 mm thick disk (6 inches in diameter and one-eighth inch thick) being moved along the wheel paths. Locations where the "disk" could be slipped under the straightedge were considered failures.

The construction acceptance method involves an actual 3.65 m straightedge placed at overlapping offsets along both wheel paths and at selected transverse locations and a 150 mm in diameter, 3 mm thick disk to slide under it. The same failure criterion was used; being able to freely pass the disk under the straightedge constitutes a failure point.

Side by side tests of the profiler software and the actual straightedge were conducted in Arizona, Texas and at a pair of LTPP locations in Illinois where traffic control was already available. Initial attempts to match profiler and straightedge evaluations at 90 percent or better have failed. In the best case the percentage of matches is only 60 percent. It has been shown that the straightedge and the high-speed profiler cannot be considered interchangeable² and the two tests may not be considered interchangeable for future evaluations. One factor in the difference is more placement locations in the software than in the field. Modifying the software for that circumstance does not however improve the comparisons. A second is the properties of the actual straightedge. The straightedge must be

² Short Wavelength Pavement Smoothness Testing at Pilot Project Locations, *Draft*, December 2001.

between 19 and 75 mm wide in the measurement plane with a maximum out-of-trueness of less than ± 0.40 mm/m at the bottom surface in both dimensions. Along the entire length of the straightedge the potential difference can be as much as a third of the thickness of the disk. It is possible to construct such a straightedge and to get a disk that isn't warped in the cutting process. Varying the size of the virtual disk has not provided a means to improve the correlation between the methods.

The all or nothing criterion using an actual straight edge for construction is likely to be retained despite its lower sensitivity to roughness. At the AZ site it only identified a tenth of the locations the profiler did. However for new pavements, with traffic control in place for possible grinding, running a profiler is likely to be a complicated process. For profiler results, the far more commonly used measure, some boundary conditions will have to be established for desirable, acceptable, and unacceptable. It may be possible to establish them without collecting significant amounts of additional field data by using the methods of the long wavelength study.

Table 3-4 shows two possible measures of failure using the current divotometer software. The first is the average number of failures or divots per profiler pass. The other is the number of failures that are found in all profiler passes along a given path. The break point for expected failure due to speed, the smoothness surrogate, falls somewhere between the Maryland and Michigan conditions.

State	AZ	FL n.b.	FL s.b.	FL n.b.	FL s.b.	MD	MI	TX
Sensor Type	bending	piezo	piezo	bending	bending	piezo	bending	bending
	plate			plate	plate		plate	plate
Average divots	220.8	8.3	9.3	8.3	9.3	31.4	119	24.4
Replicate divots	152	5	4	5	4	22	90	11
Weight result	FAIL	FAIL	FAIL	PASS	PASS	FAIL	FAIL	PASS
Failure reason	Speed	Temp.	Тоо			Temp	Speed	
			variable					
Pavement	mixed	AC	AC	AC	AC	AC	PCC	AC

Table 3-4 Smoothness estimate

3.7.2 Long wavelength

The recommendation of a criterion and its comparison against actual field results is due after completion of this summary in early Spring 2002.

4 Logistics

The field procedures as initially designed assumed a single day activity with a 4-person crew on site including the drivers. Actual site evaluations including travel time, and provision for delays have become 5-day activities including travel, training of new crew at each site, loading and weighing trucks, calibration and the actual data collection. Staffing as currently organized is a minimum of two drivers, a WIM equipment operator, a data collector for weights, a collector of speeds and temperatures and an odd jobs person. Neither the duration nor the staff sizes are likely to be significantly reduced for future site evaluations without the changes suggested in this section. It is clear that for the semi-annual evaluations that are part of the routine monitoring requirement a major effort must be made to reduce the resource requirements.

4.1 Coordination

One of the most time consuming parts of each visit is the coordination between the parties involved. Identifying the interested parties and principal points of contact is crucial to efficiently conducting evaluations. A combination of detailed communication, advance planning, training and identification of Go/No Go conditions is needed for every successful visit. Coordination must occur in advance as well as on-site and be responsive to unexpected conditions.

It is suggested that a new activity, a site visit prior to the initial evaluation, be added to reduce the amount of time spent on site and to gather information that could indicate that scheduling a visit is inappropriate. Among the useful information would be classification algorithms and speed distributions for the site. This would also provide the opportunity to brief agency personnel on field activities and expectations early on for familiarity with LTPP practices.

It is important that someone knowledgeable as to the WIM system controller type and its operation be on-site during pre-validation calibration.

4.2 Schedules and Duration

There are currently two elements to schedule, the visit itself and on-site activities with a third recommended, the pre-site visit. Given the lead times on trucks and travel site evaluations and validations should be scheduled no less than thirty days prior to their occurrence. The suggested initial pre-validation visit and associated coordination should occur at least six and preferably eight weeks prior to a proposed evaluation so that any minor deficiencies can be corrected.

The schedule of on-site activities for a site evaluation needs to allow for at least a day and a half to include loading and weighing trucks, briefing all on-site staff, training time particularly for drivers, collect the data and make provision for a range of temperatures. If a straightedge evaluation is done, another half day needs to be included. This time can be reduced by using the same trucks and drivers at multiple sites, briefing state personnel at the pre-site visit and checking speeds and classification for gross errors at the pre-site visit. In the current air travel environment it is unlikely that the two days per site of travel time will be significantly reduced with out doing back to back evaluations. Working back to back evaluations to include weekends is not a satisfactory solution unless travel is shifted to weekends. The difficulty in getting state personnel for weekend work and the hours of service rules for drivers will act as constraints.

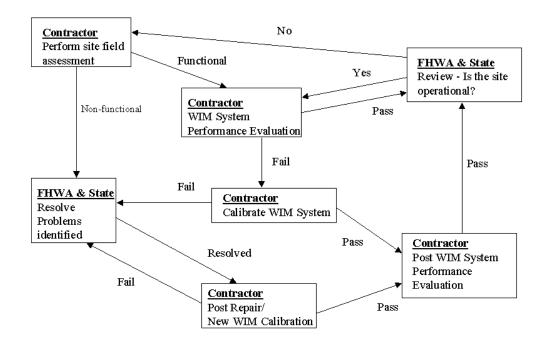


Figure 4-1 Flow diagram of SPS traffic pooled fund activities

As shown in Figure 4-1 the process of site evaluation and validation is iterative.

4.3 Trucks and Drivers

For the sites done to date, two trucks were generally sufficient to do all data collection in a single day. Longer turn around routes will need more trucks if the cost estimating assumption of a day on site is to remain valid.

The site by site process of acquiring trucks, finding loads and hiring drivers was very time consuming. The strong support of all the states involved in the pilots in either providing all vehicles, loads and drivers or facilitating the process with local contacts and knowledge simplified the validation process. However, to put the evaluation and validation process into cost efficient production will require longer term contracts for drivers, trucks and loads. The two to three man-days of effort required for site specific truck acquisition exceed the estimated level of administrative effort per site.

Training of drivers is an essential part of collecting quality data. There are two aspects where this is essential, static weighing and speed differentiation during data collection. Problems have been observed when drivers fail to release their brakes on the scales. This leads to more repetitions of the weighing process to get consistent values. The second aspect is driving a constant target speed so bins are clear and not intermingled. Driving safely will always take priority over the research objectives. As shown in Figure 5-1 through Figure 5-5 not all drivers were able to consistently differentiate speeds. In part, a wider range of speeds needed and less traffic interference facilitated these efforts. These results support the

recommendations for the target ranges and bin sizes. Additionally, the practice of running at the highest target speed first then the middle and finally the low should be continued. The failure to do so for one site led to the analysis using two temperature bins instead of three. Pavement temperature changed faster than speeds could be changed given the length of the turnarounds at most sites.

4.4 Staffing

As originally envisioned the data collection staff was composed solely of LTPP contractors. Experience has shown that this is not prudent. Furthermore the staff size on which the cost estimates were based could be achieved with some modest changes in data collection activities.

4.4.1 Data Collection

Data collection includes successfully using on-site equipment. In five site visits, six separate installations have been encountered (Florida had two). Bringing outside staff up to date on every system is a poor use of limited contract resources. None of the equipment is owned by LTPP and to date all of it has required some level of calibration. The calibration of state equipment, the actual turning of the dials, by LTPP contractors will require making non-LTPP users aware that they are responsible for how the resulting data is interpreted and used. The preferred scenario is having state resources on site to access the equipment, down load data for the session and perform any necessary calibration adjustments. Having agency representation on site has the added benefit of eliminating the need for LTPP contractors to keep track of, own and learn multiple versions of software.

Part of the data collection process involves static weighing of trucks. With experienced personnel this only requires one individual other than the drivers.

The weight data collection process can be done by recording everything or simply the applicable record number. Recording everything is possible when supported by the software and when the trucks are spaced far enough a part to allow for the time to collect all the information. In some instances it is easier to platoon the trucks and just get record numbers. In either case vehicle record numbers are mandatory, as is a download of the day's records for the collection period to check for transcription errors.

The classification verification process needs to be done as a blind test. If all vehicles are done in an hour, then only one trained individual is needed. If a vehicle by vehicle comparison is desired to diagnose possible problems then two individuals are needed, one to read out record numbers and the other to manually classify the stream while recording those numbers. The two person approach is also needed where classification checking is going to be an intermittent activity. In addition a procedure is needed that more fully describes the verification of quality classification data.

The need to collect both temperature and speed data independent of the installed equipment adds another person to the data crew size. This individual is not really busy all the time but without extensive experience and a second person to support, intermittent classification activities cannot be used for much. It is suggested that once the speed reported by the equipment has been verified as reasonably accurate no speed data be collected on the test trucks themselves. Since speed is a qualitative rather than quantitative analysis variable this should not affect the results. The second recommendation to eliminate the need for this person is to install thermocouples in the pavement to record temperatures. For some equipment they are already in place. For others this is a simple activity to provide instrumentation that can be used on routine site visits. This serves two purposes and possibly three. Firstly, it removes a labor requirement. Secondly, it makes for a safer work site since personnel can be moved completely off the shoulders to the equipment box and potential behind a barrier. Thirdly, depending on the location of the installation it can provide a resolution of the observed temperature differential issue. Temperature is a qualitative analysis variable. However, in cases where temperature sensitivity is observed, knowing when it happens as a function of conditions of sensor conditions is more desirable than as a function of shoulder conditions.

4.4.2 Smoothness

Running the profiler for smoothness checking can be done with a standard LTPP crew of one including any temporary or permanent marking and all of the necessary runs. When a straightedge is used at least two people are required. They must mark the paths to be checked in addition to actually using the straightedge and disk. Since one person is needed to place the disk and straight edge, a second individual to track placements and record the data is extremely useful as well as being able to provide relief. A three person crew is optimal since it halves the time needed by a two person crew. This is important to shorten road closure time and increase the time available for test runs.

To date all profiling has been done in conjunction with a site visit. In the case of one site where trucks visibly bounced and the WIM section had obvious distresses a pre-visit profiling could have eliminated this site as a possible pilot. To eliminate further site visits where smoothness is clearly going affect results, profiling should be done before the first evaluation. It is not necessary to profile during an evaluation unless the results are expected to change significantly due to temperature or interim maintenance activities.

4.5 Go/ No Go Decisions

Due to the resource requirements a more formal system of Go/ No Go decision making should be employed. For pilots this has been willingness by the states to provide a site. For production work the decision must be based on equipment and site conditions, and staff availability. Go/ No Go decisions are made at two points, initial scheduling and on-site. For initial scheduling to occur the following is recommended:

- 1. Advance coordination and an approved plan.
- 2. Identification of an on-site agency representative.
- 3. Equipment must be operational.
- 4. Data on the speed range must be reasonably available.
- 5. Verification of the classification algorithm.
- 6. A smoothness evaluation, at a minimum short wave length and preferably long wave length as well has been conducted that indicates the site has a reasonable chance of meeting the criteria.

- 7. Trucks and loads and any necessary loading equipment must be available.
- 8. Trained data collection crew must be available.

On-site Go/No go decisions should be limited to ones based on unexpected equipment failure, weather or safety which cannot be addressed during any slack in the schedule.

5 Analysis

As more pilot studies were done the reporting evolved. Even though pilots are expected to be in some sense unique, the time involved in analyses was substantial and could not be performed in the field. The ability to both facilitate the process and account for the vagaries encountered at each site would significantly improve response and taking corrective action. Where the current field manual suggests that a site meeting the criteria needs only a memo to that effect, indications are that a complete document of the analysis is preferred for all sites. It is suggested that such a report contain the following information:

- 1. Site location and pavement type.
- 2. WIM system vendor and controller type.
- 3. Sensor type
- 4. System calibration parameters
- 5. Period of testing and weather conditions.
- 6. Truck types, suspensions and weights (gross and by axle).
- 7. Raw data including WIM ID number.
- 8. The overall results by criterion.
- 9. The smoothness evaluation.
- 10. A speed temperature matrix.
- 11. The results as a function of temperature.
- 12. The results as a function of speed.
- 13. Graphs of GVW for:
 - the entire population by truck
 - by temperature group
 - by speed group
- 14. Graphs of single axles for:
 - the entire population by truck
 - by temperature group
 - by speed group
- 15. Graphs of tandem axles for:
 - the entire population by truck
 - by temperature group
 - by speed group
- 16. And, for sites that do not meet the criteria, graphs which illustrate each problems. The number and composition of such graphs must be determined on a case by case basis.

If the site fails to meet the criterion the recommendation should be bending plate in smooth PCC unless viable corrective actions exist or the site is going out of study within 18 months. If the site fails due to smoothness grinding may be an option but whether that or new PCC is the selected solution, reinstallation of sensors can be expected.

5.1 Smoothness

The current divotometer software for short wave smoothness evaluation can be applied in the field. What is needed is better summary reporting for the actual criterion that is adopted as a result of the smoothness investigation. Currently the only thing reported is the number of divots in each pass by pass. A summary across multiple passes would be useful.

5.2 Weight Criteria

While the standard spreadsheet template format that evolved works well, a more automated process would be desirable. At a minimum the spreadsheet format should be standardized to encompass all the reasonable variations in input and have templates for all basic graphs in place. At the upper end a piece of software capable of taking text output from the equipment and producing a general report on site would be the goal. In either case, for future evaluations reporting at the conclusion of the visit should be the goal.

In spite of the difficulties in getting the desired 3x3 relatively even distribution of speed and temperature combinations the requirement should not be dropped. As the FL evaluation showed, it is possible to achieve the necessary points. For the initial or a single site evaluation the widest possible range and relatively even distribution between groups should continue to be the goal, principally for use in diagnostics of failing sites. For sites on a twice yearly verification schedule three speeds and 2 temperatures may be more rational as an outcome of time constraints. For sites on a biannual verification schedule picking the hottest and coldest months for the year will give at least 3 groups in the course of the year and under optimal conditions might yield 5 or even 6.

Figure 5-1 through Figure 5-5 show the various speed and pavement temperature distributions actually experienced. As can be seen in the figures, getting tight groups for speeds is not a given. The skill and training of the drivers as well as the traffic and roadway conditions contribute to the precision of the speeds.



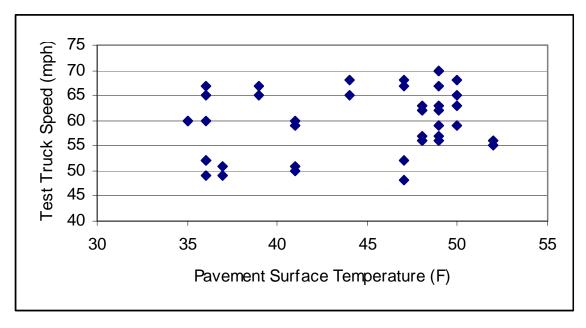
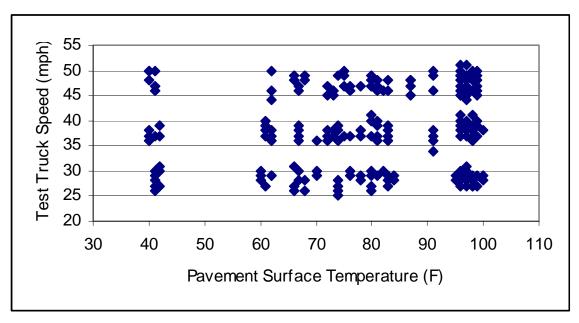
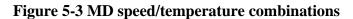


Figure 5-2 FL speed/temperature combinations





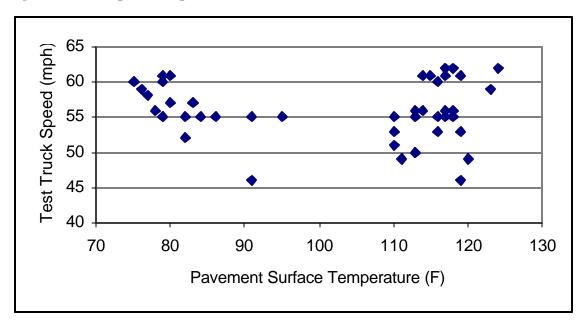
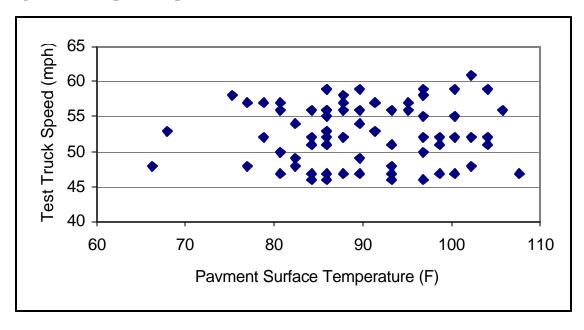


Figure 5-4 MI speed/temperature combinations



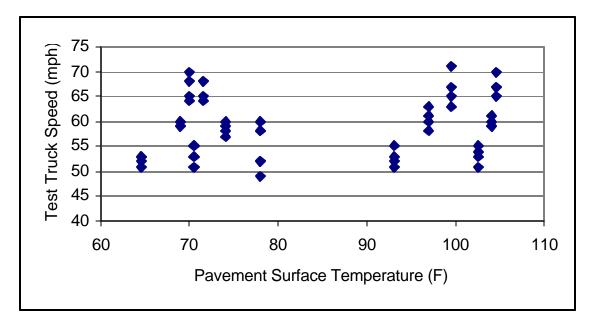


Figure 5-5 TX speed/temperature combinations

6 Summary

The pilot studies were successful in demonstrating the availability of a practical truck weight data validation procedure and the reasonableness of the standard defined for research quality data. The following conclusions have been drawn from those activities:

- The ASTM E-1318 Type I WIM system based performance specification is achievable with current technology and practices.
- The recommendation of bending plate sensors in smooth PCC will meet the performance specification.
- The minimum two trucks with a 3x3 matrix of temperatures and speed is sufficient to determine if the weight data is of research quality.
- Smoothness is an important factor in determining variability of weights.
- The current acceptance criteria for straight edge and profiler measurements is inappropriate.
- The straight edge and the profiler smoothness methodologies are not equivalent.
- A better definition of smooth enough must be developed for both the straight edge and profiler evaluations.
- Attention needs to be paid to PCC/AC interfaces where they exist on the site.
- A Go/No Go standard needs to be established to determine which sites are in fact suitable for evaluation and which will not produce research quality data without prior remedial work.
- A greater emphasis on vehicle classification and a measure of the quality of that aspect of the data is needed.
- The ability to do the analysis of data on site is needed.

An additional issue centers around a element which is difficult to test empirically, the desired length of slab. At the PCC sites tested the 500 foot length which was based on Figure 6-1 was found to be reasonable minimum. Consideration should be given to making the 500 foot slab a minimum rather than the fixed length specification.

Figure 6-1 Method for slab length determination

Slab Length for Minimum WIM Error Length SD = nV/fSettling Distance (SD) 75 ft n = No. cycles 75 ft V = Speed scale = Frequency 700 5 cycles, 1.5Hz 600 Recommended design 500 Slab length (ft) 400 Settling distance 300 4 cycles 2Hz 200 100 0 20 40 60 0 80 100 D. Cebon Traffic Speed (mph) 28 March 2001

7 Site Specific Results

The section provides the overall summaries for each of the pilot studies done.

7.1 Arizona

This initial analysis is based on test runs conducted during the evening of October 31 and the morning of November 1, 2001 at test site 040600 on I 40 west of Flagstaff, AZ. This SPS-6 site is at milepost 202 on the eastbound, right hand lane of a divided four-lane facility. The traffic monitoring equipment at this location includes inductance loops and bending plates on all lanes. These sensors are installed within a PCC pavement about 500ft in length. The roadway outside this short section is asphalt. Immediately prior to the test runs, the bending plate and inductance loops on the LTPP lane were replaced and calibrated. No auto-calibration was used during test runs. The two trucks used for initial calibration and for the subsequent testing included:

- 1. A five-axle tractor semi-trailer with a low-bed trailer loaded to near 70,000 lbs.
- 2. A five-axle tractor semi-trailer with flat-deck trailer loaded to just over 77,000 lbs.

Both five-axle units were equipped with air suspensions on tractor and trailer. The low-bed trailer on truck #1 had a standard tandem axle group with an additional lift axle that was in the raised position.

There are two attributes of this site that make it less than ideally suited for WIM. First, the location is near the top of a grade of about one mile that made it difficult for the test trucks to achieve the speeds at which the majority of the truck traffic was operating (70mph). The site itself is on a positive grade of approximately 2%. Secondly, the AC/PCC interface located about 400 feet prior to the bending plate is not smooth and produced very pronounced suspension movements in both test trucks. Although the suspension movement was largely damped by the time the trucks reached the bending plates, there were cyclical movements of the trailer platforms themselves that occurred independent of the suspension and these continued beyond the scale location. Unfortunately both trailers were loaded such that the mass was concentrated midway between the two axle groups. This caused the steel trailer deck to act as a spring whenever the road surface was less than smooth and undoubtedly had an effect on the forces recorded by the bending plates.

Each truck made a total of 20 passes over the WIM scale at speeds ranging from approximately 50 to 70 miles per hour. Pavement surface temperatures were recorded during the test runs ranging from about 35 to 52 degrees Fahrenheit. The narrow range of temperatures was the result of testing only during the evening and early morning hours necessitated by a delay in the installation and calibration of a new bending plate within the LTPP lane. The computed values of 95% confidence limits of each statistic for the total population are within Table 7-1.

Characteristic	Tolerance	Computed 95% Confidence	Pass/Fail
		Interval	
Axle Weights			
Steering	" 20 %	-0.32% " 11.02%	PASS
Tandem	" 15 %	5.21% " 14.24%	FAIL
Gross Vehicle Weights	" 10 %	4.49% " 7.41%	FAIL
Axle Spacing			
Between Groups	" 0.5 ft	0.74 ft " 1.18 ft	FAIL
Within Groups	" 0.5 ft	0.16 ft " 0.45 ft	FAIL

Table 7-1 Overall AZ site 0600 results

7.2 Florida

This analysis is based on test runs conducted during the afternoon of November 7 and the morning of November 8, 2001 at a WIM site on US 319 south of Tallahassee, FL. This location, which is not part of any LTPP experiment, is on an undivided two-lane facility. It is near a major intersection and is subject to congestion as well as stop-and-go traffic during morning and afternoon rush hours. The posted limit is 55 mph and normal traffic speeds

range from 30 to 55 miles per hour. The traffic monitoring equipment includes inductance loops/piezo sensors as well as inductance loops/bending plate installations on both the northbound and southbound lanes. These sensors are installed within an AC pavement. The two trucks used for this calibration and for the subsequent testing included:

- 1. A five-axle tractor semi-trailer with a box van trailer loaded to near 77,000 lbs.
- 2. A five-axle tractor semi-trailer with a box van trailer loaded to just over 77,000 lbs.

Both five-axle units were equipped with air suspensions on tractor and trailer. The lighter box van trailer on truck #1 was evenly loaded across both tandem axle groups while the heavier truck #2 was loaded with a bias toward the rear tandem group.

For this comparison test, each truck made a total of 40 plus passes over each of the four WIM scales (piezo & bending plate – both directions) at speeds ranging from approximately 30 to 50 miles per hour. Pavement surface temperatures were recorded during the test runs ranging from about 40 to 100 degrees Fahrenheit. This range of temperatures was achieved by testing during the afternoon of Nov. 7 and again in the early morning hours of the following day. Heavy traffic necessitated that testing be stopped at 7:00am on Nov. 8 and resumed again at 9:00am. The computed values of 95% confidence limits of each statistic for the total population on the four scales are recorded in Table 7-2 and Table 7-3.

Characteristic	Tolerance	Northbound		Southbound		
		Computed 95%	Computed 95% Pass/Fail		Pass/Fail	
		Confidence Interval		Confidence Interval		
Axle Weights						
Steering	" 20 %	-11.0% " 24.0%	FAIL	6.30% " 17.0%	FAIL	
Tandem	" 15 %	-2.96% "26.9%	FAIL	-4.11% " 17.0%	FAIL	
GVW	" 10 %	-3.61% " 25.0%	FAIL	-2.34% " 11.3%	FAIL	
Axle Spacing						
Between Groups	" 0.5 ft	0.16 ft " 0.36 ft	FAIL	0.16 ft " 0.34 ft	PASS	
Within Groups	" 0.5 ft	-0.12 ft "0.10 ft	PASS	-0.12 ft " 0.10 ft	PASS	

Table 7-2 Overall FL site - Piezo-electric WIM results

Table 7-3	Overall	FL site -	Bending	Plate	WIM	results
I ubic / C	O v Ci uni		Demaning	I IUUU	· · AIVA	I COULCO

Characteristic	Tolerance	Northbound		Southbound			
		Computed 95%	Pass/Fail	Computed 95%	Pass/Fail		
		Confidence Interval		Confidence Interval			
Axle Weights							
Steering	" 20 %	0.95% " 6.53%	PASS	-0.81% " 6.46%	PASS		
Tandem	" 15 %	-0.17% " 5.04%	PASS	0.52% " 5.02%	PASS		
GVW	" 10 %	0.25% " 3.52%	PASS	0.56% " 3.77%	PASS		
Axle Spacing							
Between Groups	" 0.5 ft	0.01 ft " 0.50 ft	FAIL	0.71 ft " 1.33 ft	FAIL		
Within Groups	" 0.5 ft	-0.15 ft " 0.29 ft	PASS	-0.03 ft " 0.31 ft	PASS		

7.3 Maryland

A WIM site evaluation study was conducted at the SPS-5 site on US 15 near Frederick, Maryland on June 12 and 13, 2001. This location is on a two-lane roadway between two unsignalized intersections on a slight, but long continuous upgrade. Neither intersection affected site operations. The posted speed for this site is 55 mph. Actual running speeds range from 45 to slightly over 60 mph. This site has a system with a pair of piezo sensors without direct temperature compensation and a pair of inductance loops installed in asphalt. The sensors were installed in late 2000 and had never been calibrated. The previous sensors were also piezo and still visible in the pavement. Only one piezo sensor was used to capture weight data. The original sensors at the site were bending plate that had never produced satisfactory results. The two trucks used for site calibration and validation included:

- 1. A five-axle tractor semi-trailer loaded to 78,400 lbs.
- 2. A five-axle tractor semi-trailer loaded to 78,100 lbs.

One truck had air suspension tandems on the tractor and trailer. The other had an air suspension tandem on the tractor and a steel spring suspension spread tandem on the trailer. The spread tandem was moved to its minimum spacing.

The WIM system was calibrated on June 12th as part of this site visit with three runs in each direction of a Class 9 (3S2) truck loaded to 78,100 pounds. The first run was used to adjust the calibration and the next two runs were used to confirm the correction was appropriate. Since the piezo sensors are temperature sensitive, this site's WIM equipment uses the front axle of Class 2 vehicles for automatic calibration. Class 2 vehicles are used since the site does not have enough Class 9 vehicles to recalibrate often enough to handle the temperature variations. Still the number of Class 2 vehicles was reduced from 100 to 30, then 20 and finally 10 during data collection to force the auto-calibration to take effect more often. This was an attempt to keep the WIM system calibrated to known truck weights during the hot part of the day. The results of the pilot are shown in Table 7-4.

Characteristic	Tolerance	Computed 95%	Pass/Fail
		Confidence Interval	
Axle weights			
Steering axles	<u>+</u> 20%	2.2% <u>+</u> 21.1%	FAIL
Tandem axles	<u>+</u> 15%	-4.2% <u>+</u> 22.8%	FAIL
Gross vehicle weights	<u>+</u> 10%	-3.25% <u>+</u> 18.90%	FAIL
Vehicle speed	<u>+</u> 1 mph [2 km/hr]	<u>+</u> 2 mph	FAIL
Axle spacing length	<u>+</u> 0.5 ft [150 mm]	+0.35 ft	PASS

Table 7-4 Overall MD 0500 site results

7.4 Michigan

The site evaluation for the SPS-1 north of Lansing, Michigan was conducted July 25 and 26, 2001. This site was visibly rough as vehicles were observed to bounce approaching the scale. The initial analysis was based on 80 runs split between three trucks:

- 1. 5-axle tractor-trailer combination weighing 68,600 lbs.
- 2. 2-axle bucket truck ("Cherry Picker") weighing 15,700 lbs.
- 3. 2-axle dump truck weighing 33,100 lbs.

There were several truck breakdowns in the course of the testing and as a result the preferred vehicle completed a minority of the runs. The impact of site roughness and vehicle dynamics resulted in two analyses, one with all three trucks and one where the bucket truck was omitted. Of all the runs, nineteen percent of the runs were made by the Class 9 (5-axle tractor-trailer), half were made by the bucket truck and the remainder by the dump truck. The total population statistics in comparison with the LTPP WIM tolerances are shown in Table 7-5. Table 7-6 shows the results after omitting the bucket truck.

Characteristic	95% Confidence	Computed Value	Pass/Fail
	Limit of Error		
Axle Weights			
All single axles	"20%	+3.2% " 18.8%	FAIL
Steering axles	"20%	-2.1% " 5.1%	PASS
Other single axles	"20%	+9.8% " 21.4%	FAIL
Tandem axles	"15%	-3.5% " 28.9%	FAIL
Gross Vehicle Weights	"10%	+3.0% " 14.0%	FAIL
Vehicle Speed	"1 mph	+1.5 " 4.2	FAIL
Axle spacing			
All lengths	"0.5 ft	+0.3 " 1.6	FAIL
Tandem axles	"0.5 ft	+0.1 " 0.6	FAIL

Table 7-5 Overall MI 0100 site results

Table 7-6 MI site results with selected trucks

Characteristic	95% Confidence	Computed Value	Pass/Fail
	Limit of Error		
Axle Weights			
All single axles	"20%	-2.8% " 4.7%	PASS
Steering axles	"20%	-2.5% " 5.1%	PASS
Other single axles	"20%	-3.3% " 1.8%	PASS
Tandem axles	"15%	-3.5% " 28.9%	FAIL
Gross Vehicle Weights	"10%	-3.4% " 7.3%	PASS
Vehicle Speed	"1 mph	+1.1 " 2.8	FAIL
Axle spacing			
All lengths	"0.5 ft	+0.5 " 1.8	FAIL
Tandem axles	"0.5 ft	+0.1 " 0.6	FAIL

7.5 Texas

This analysis is based on a full day of test runs conducted on October 17, 2001 at test site 480100 on US 281 north of Edinburg, TX. This SPS-1 site is on the southbound, right hand lane of a divided four-lane facility. The equipment is a bending plate installed in asphalt. The trucks used included:

- 1. A three-axle dump truck loaded with gravel to a GVW of approximately 45,500 lbs.
- 2. A six-axle 'lowboy' tractor semi-trailer combination loaded to about 86,800 lbs.
- 3. A five-axle tractor semi-trailer with a split-tandem trailer loaded to near 79,000 lbs.
- 4. A five-axle tractor semi-trailer with standard trailer axles loaded to 57,300 lbs.

Both five-axle units were equipped with air suspension on tractor and trailer while the dump truck and 6-axle semi had steel leaf springs only. This evaluation was unique in that a triple axle group was available for testing. Since no tolerance level for statistics based on such groups has been specified by LTPP, this report uses the premise of ASTM 1318 that all axle groups should meet the same qualifications. Each truck made a total of 14 passes over the WIM scale at speeds ranging from approximately 50 to 70 miles per hour. Pavement surface temperatures were recorded during the test runs ranging from about 65 to 105 degrees Fahrenheit. The computed values of 95% confidence limits of each statistic for the total population are in Table 7-7.

Characteristic	Tolerance	Computed 95% Confidence	Pass/Fail
		Interval	
Axle Weights			
Steering	" 20 %	-2.5% " 6.41%	PASS
Tandem	" 15 %	0.70% " 9.47%	PASS
Triple	" 15 %	-1.4% " 6.56%	PASS
Gross Vehicle Weights	" 10 %	0.31% " 7.08%	PASS
Axle Spacing			
Between Groups	" 0.5 ft	0.14 ft " 0.86 ft	FAIL
Within Groups	" 0.5 ft	0.02 ft " 0.32 ft	PASS

Table 7-7 Overall TX 0100 results

This section contains all of the basic information that went into the analyses discussed in this document and the pilot reports.

8.1 Arizona

Table 8-1 AZ Static Test Truck measurements

Test	S	Static Measu	rements (lbs	s)	Static Spacings (ft)				
Trucks	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E	
1	11030	29,360	28,940	69,330	9.17	4.25	38.54	4.50	
2	10860	35,120	30,700	76,680	19.42	4.50	34.33	10.21	

Table 8-2 AZ raw data for LTPP lane

			Pavement		WIM Weights (lbs)				WIM Spacings (ft)				
Date	Truck #	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E	
11/1/01	1	6:46	36	52	11,200	32,900	27,700	71,900	9.7	4.2	40.1	4.8	
11/1/01	2	6:46	36	49	11,400	36,800	30,400	78,700	20.0	4.6	35.1	10.5	
11/1/01	1	6:57	35	60	10,600	32,200	29,900	72,700	8.8	4.4	38.0	4.8	
11/1/01	2	6:57	35	60	11,500	38,500	29,100	79,200	20.0	4.4	35.2	10.4	
11/1/01	1	7:09	36	67	9,700	30,500	31,800	72,000	9.3	4.0	39.9	4.4	
11/1/01	2	7:09	36	65	10,100	35,700	28,600	74,400	19.8	4.7	34.9	10.3	
11/1/01	1	7:21	37	51	11,100	32,300	28,200	71,500	9.5	4.4	40.0	4.8	
11/1/01	2	7:21	37	49	11,500	37,400	31,600	80,500	19.6	4.6	35.1	10.1	
11/1/01	2	7:35	36	60	11,400	39,900	29,800	81,000	20.0	4.8	35.2	10.4	
11/1/01	1	7:38	36	52	11,600	32,600	27,800	72,000	9.4	4.2	39.4	4.5	
11/1/01	1	7:52	39	67	10,700	31,700	32,000	74,400	9.3	4.4	39.4	4.4	
11/1/01	2	7:52	39	65	10,700	36,800	31,000	78,400	19.8	4.7	34.9	10.3	
11/1/01	2	8:04	41	50	11,300	37,400	31,900	80,600	20.1	4.7	35.1	10.4	
11/1/01	1	8:05	41	51	11,000	32,400	27,500	70,800	9.2	4.4	38.9	4.8	
11/1/01	2	8:16	41	60	10,700	39,600	29,100	79,500	20.4	4.8	35.2	10.8	
11/1/01	1	8:17	41	59	11,100	31,300	31,400	73,700	8.9	3.9	38.2	4.7	
11/1/01	1	8:29	44	68	10,500	31,100	35,000	76,600	9.5	4.5	39.6	4.5	

			Pavement			WIM Wei	ghts (lbs)			WIM Spa	acings (ft)	
Date	Truck #	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
11/1/01	2	8:29	44	65	10,500	36,400	30,900	77,800	19.8	4.7	34.9	10.3
11/1/01	1	8:46	47	52	10,900	33,200	27,500	71,600	9.4	4.2	39.8	4.5
11/1/01	2	8:46	47	48	11,600	36,800	30,800	79,100	19.6	4.8	34.4	9.9
11/1/01	1	9:02	49	63	10,800	30,300	29,800	71,000	9.2	4.6	39.5	4.6
11/1/01	2	9:02	49	59	11,800	39,000	29,100	79,900	19.9	4.3	35.1	10.1
10/31/01	2	17:30	52	55	11,900	41,900	31,700	85,500	20.0	4.4	35.0	10.6
10/31/01	1	17:31	52	56	11,100	33,000	26,100	70,200	9.3	4.5	39.1	4.5
10/31/01	1	17:41	50	63	10,000	31,100	31,200	72,400	9.6	4.6	40.4	4.6
10/31/01	2	17:41	50	59	11,000	40,600	30,600	82,200	20.3	4.7	35.1	10.5
10/31/01	2	17:52	50	65	10,200	36,600	31,000	77,700	20.3	4.7	34.9	10.3
10/31/01	1	17:53	50	68	11,000	30,100	33,500	74,600	9.5	4.1	40.0	5.0
10/31/01	1	18:05	49	70	10,600	32,100	34,600	77,300	9.8	4.2	40.3	5.2
10/31/01	2	18:05	49	56	11,900	40,000	32,200	84,100	20.9	4.8	36.5	10.8
10/31/01	1	18:16	49	62	10,400	31,000	30,100	71,500	9.0	4.5	39.8	4.5
10/31/01	2	18:16	49	57	11,200	39,100	30,300	80,700	19.8	4.6	34.7	10.3
10/31/01	2	18:30	49	67	10,700	35,000	29,900	75,600	20.4	4.8	36.3	10.6
10/31/01	1	18:31	49	70	11,000	31,900	35,900	78,800	9.8	4.2	40.7	4.7
10/31/01	1	18:43	48	57	10,500	32,200	25,800	68,500	9.5	4.6	40.0	4.6
10/31/01	2	18:43	48	56	11,900	41,500	31,700	85,100	20.5	4.8	35.8	10.8
10/31/01	1	18:58	48	63	9,800	30,300	29,000	69,200	9.6	4.6	39.9	5.0
10/31/01	2	18:58	48	62	10,700	37,700	29,000	77,500	20.9	4.5	36.5	10.6
10/31/01	2	19:12	47	67	10,600	36,700	30,300	77,600	20.4	4.8	35.9	11.1
10/31/01	1	19:13	47	68	10,100	31,500	33,200	74,900	9.5	4.1	39.6	4.5

8.2 Florida

Table 8-3 FL Static Test Truck Measurements

Test	S	tatic Measu	rements (lbs	Static Spacings (ft)					
Trucks	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E	
1	11,750	30,270	0:00	76,400	16.8	4.4	34.3	4.3	-
2	11,460	33,080	0:00	77,160	19.9	4.4	33.4	4.3	

$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Piezo							WIM Spacings (ft)						
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:08	98	27	11,900	31,200	38,300	81,600	16.8	4.4	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:19	98	37	12,400	31,900	37,100	81,600	16.8	4.3	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:28	99	46	11,600	30,200	35,400	77,400	16.7	4.3	34.4	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:35	98	28	11,200	29,900	36,300	77,600	16.7	4.3	34.3	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:43	98	36	12,000	32,500	37,700	82,300	16.7	4.3	34.4	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:51	96	47	11,200	29,600	34,400	75,500	16.7	4.3	34.4	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	13:58	100	29	11,800	30,100	36,300	78,500	16.7	4.3	34.3	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:07	99	37	11,800	31,100	35,300	78,400	16.7	4.3	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:14	99	48	11,200	30,500	35,000	77,000	16.8	4.3	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:22	97	27	11,900	29,900	35,700	77,600	16.7	4.3	34.3	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:29	96	38	12,500	31,600	36,900	81,200	16.8	4.4	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:35	96	48	11,700	29,900	35,200	76,900	16.8	4.4	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	14:43	97	27	11,800	29,000	35,800	76,800	16.7	4.3	34.3	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:00	97	47	11,800	31,200	36,800	79,900	16.7	4.3	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:08	97	27	11,200	30,500	36,000	77,900	16.7	4.3	34.3	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:19	96	38	12,500	32,800	39,200	84,600	16.8	4.3	34.5	4.1
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:26	96	48	11,900	30,000	34,900	77,000	16.8	4.4	34.5	4.1
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:34	96	28	11,800	30,600	35,800	78,200	16.7	4.4	34.4	4.1
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:42	91	38	10,600	26,800	30,600	68,200	16.8	4.3	34.5	4.1
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:50	87	48	11,200	28,900	33,900	74,200	16.8	4.3	34.6	4.1
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1	1	11/7/01	15:59	83	28	10,600	28,500	34,200	73,500	16.8	4.3	34.4	4.1
1 1 11/8/01 6:20 41 27 8,000 21,000 25,000 54,100 16.7 4.3 34.3 4.1 1 1 11/8/01 6:29 40 37 8,100 22,300 25,500 56,200 16.7 4.3 34.3 4.1 1 1 11/8/01 6:37 41 47 7,800 21,600 24,600 54,300 16.7 4.3 34.3 4.1 1 1 11/8/01 6:37 41 47 7,800 21,600 24,600 54,300 16.7 4.3 34.3 4.1 1 1 11/8/01 6:47 42 27 7,800 21,200 24,600 53,800 16.7 4.3 34.3 4.1 1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3	1	1	11/7/01	16:07	83	38	10,600	28,100	33,000	71,800	16.7	4.3	34.4	4.1
1 1 11/8/01 6:29 40 37 8,100 22,300 25,500 56,200 16.7 4.3 34.3 4.1 1 1 11/8/01 6:37 41 47 7,800 21,600 24,600 54,300 16.7 4.3 34.3 4.1 1 1 11/8/01 6:47 42 27 7,800 21,200 24,600 53,800 16.7 4.3 34.3 4.1 1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3 34.4 4.1	1	1	11/7/01	16:16	80	49	10,800	28,100	34,800	73,900	16.8	4.4	34.4	4.1
1 1 11/8/01 6:37 41 47 7,800 21,600 24,600 54,300 16.7 4.3 34.3 4.1 1 1 11/8/01 6:47 42 27 7,800 21,200 24,600 53,800 16.7 4.3 34.3 4.1 1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3 34.4 4.1	1	1	11/8/01	6:20	41	27	8,000	21,000	25,000	54,100	16.7	4.3	34.3	4.1
1 1 11/8/01 6:47 42 27 7,800 21,200 24,600 53,800 16.7 4.3 34.3 4.1 1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3 34.4 4.1	1	1	11/8/01	6:29	40	37	8,100	22,300	25,500	56,200	16.7	4.3	34.3	4.1
1 1 11/8/01 9:08 61 27 7,700 19,600 24,100 66,000 16.8 4.3 34.4 4.1 1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3 34.4 4.1	1	1	11/8/01	6:37	41	47	7,800	21,600	24,600	54,300	16.7	4.3	34.3	4.1
1 1 11/8/01 9:16 62 37 9,100 24,000 28,100 61,500 16.7 4.3 34.4 4.1	1	1	11/8/01	6:47	42	27	7,800	21,200	24,600	53,800	16.7	4.3	34.3	4.1
	1	1	11/8/01	9:08	61	27	7,700	19,600	24,100	66,000	16.8	4.3	34.4	4.1
1 1 11/8/01 9:24 62 46 9,400 24,300 28,800 62,700 16.7 4.3 34.5 4.1	1	1	11/8/01	9:16	62	37	9,100	24,000	28,100	61,500	16.7	4.3	34.4	4.1
	1	1	11/8/01	9:24	62	46	9,400	24,300	28,800	62,700	16.7	4.3	34.5	4.1

 Table 8-4 FL Piezo Lane 1 – Raw data for Truck 1

Piezo						WIM Weights (lbs)					WIM Spacings (ft)		
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	1	11/8/01	9:32	66	31	9,400	25,500	30,400	65,400	16.7	4.3	34.4	4.1
1	1	11/8/01	9:40	67	37	9,600	24,900	29,200	63,900	16.7	4.3	34.3	4.1
1	1	11/8/01	9:47	67	47	9,800	24,400	27,700	62,100	16.7	4.3	34.5	4.1
1	1	11/8/01	10:02	72	36	10,200	26,800	31,500	68,800	16.8	4.3	34.5	4.1
1	1	11/8/01	10:10	73	46	10,000	25,600	28,700	64,500	16.6	4.3	34.3	4.1
1	1	11/8/01	10:18	74	26	9,900	26,200	30,300	66,600	16.7	4.3	34.3	4.1
1	1	11/8/01	10:25	76	37	10,100	26,100	31,400	67,900	16.7	4.3	34.4	4.1
1	1	11/8/01	10:32	76	47	10,000	25,700	28,900	64,900	16.8	4.3	34.7	4.1
1	1	11/8/01	10:41	80	27	10,300	27,200	33,300	70,900	16.8	4.3	34.4	4.1
1	1	11/8/01	10:50	81	37	10,500	28,500	33,500	72,700	16.8	4.4	34.5	4.1
1	1	11/8/01	10:57	83	46	11,000	27,500	30,200	68,900	16.6	4.3	34.4	4.1

Table 8-5 FL Piezo Lane 1 – Raw data Truck 2

Piezo							WIM Spacings (ft)						
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	1	11/7/01	13:07	95	29	9,700	32,500	31,200	73,400	20.1	4.3	33.8	4.1
2	1	11/7/01	13:18	98	39	12,500	41,000	39,300	93,000	20.1	4.3	33.6	4.1
2	1	11/7/01	13:25	99	46	11,500	36,800	36,400	84,900	20.3	4.4	33.9	4.1
2	1	11/7/01	13:34	98	29	11,700	38,200	38,900	88,900	20.3	4.4	33.8	4.1
2	1	11/7/01	13:41	98	48	10,900	35,700	36,200	82,900	20.3	4.3	33.8	4.1
2	1	11/7/01	13:50	97	39	11,200	39,000	38,600	89,000	20.2	4.4	33.8	4.1
2	1	11/7/01	13:57	96	49	11,400	37,100	37,200	85,900	20.2	4.3	33.7	4.1
2	1	11/7/01	14:04	99	39	10,700	37,500	37,200	85,600	20.2	4.4	33.8	4.1
2	1	11/7/01	14:12	99	49	11,100	36,200	34,200	81,600	20.3	4.4	33.9	4.1
2	1	11/7/01	14:19	97	30	11,200	37,900	36,300	85,600	20.3	4.4	33.9	4.1
2	1	11/7/01	14:27	96	40	11,100	38,500	36,700	86,600	20.3	4.4	33.9	4.1
2	1	11/7/01	14:34	96	50	11,200	37,200	34,500	83,000	20.3	4.4	33.9	4.1
2	1	11/7/01	14:42	97	29	11,300	37,900	36,500	85,800	19.9	4.3	33.4	4.1
2	1	11/7/01	14:50	97	39	11,200	37,800	35,900	85,100	20.2	4.4	33.8	4.1
2	1	11/7/01	14:59	97	46	11,200	37,100	36,800	85,300	20.3	4.3	33.9	4.1
2	1	11/7/01	15:07	97	29	10,900	36,500	37,900	85,500	20.2	4.3	33.7	4.1

Piezo							WIM Wei	ghts (lbs)		W	/IM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	1	11/7/01	15:23	96	49	11,200	36,100	35,300	82,800	20.4	4.4	34.0	4.1
2	1	11/7/01	15:32	96	30	12,000	38,000	40,000	90,200	20.2	4.4	33.7	4.1
2	1	11/7/01	15:40	91	36	11,100	38,800	36,600	86,700	20.2	4.4	33.7	4.1
2	1	11/7/01	15:49	87	45	10,400	34,800	33,100	78,500	20.2	4.4	33.7	4.1
2	1	11/7/01	15:58	83	29	11,800	37,200	37,900	87,000	20.2	4.3	33.7	4.1
2	1	11/7/01	16:06	83	37	10,200	36,000	33,600	79,900	20.1	4.3	33.6	4.1
2	1	11/7/01	16:14	80	41	9,400	32,700	33,300	75,600	20.2	4.3	33.6	4.1
2	1	11/8/01	6:18	41	30	7,800	27,600	25,700	61,200	20.2	4.4	33.8	4.1
2	1	11/8/01	6:27	40	38	8,100	26,500	26,000	60,800	20.2	4.3	33.8	4.1
2	1	11/8/01	6:35	41	50	7,700	26,700	24,400	58,900	20.2	4.4	33.7	4.1
2	1	11/8/01	6:43	41	27	6,200	19,900	20,400	50,600	20.2	4.3	33.5	4.0
2	1	11/8/01	6:51	42	31	7,900	25,800	26,000	60,000	20.1	4.3	33.6	4.1
2	1	11/8/01	7:00	42	37	7,900	27,100	26,400	61,600	20.0	4.3	33.5	4.1
2	1	11/8/01	9:07	60	29	8,400	28,300	28,200	65,100	20.1	4.3	33.7	4.1
2	1	11/8/01	9:15	62	29	8,600	28,500	28,200	65,500	20.3	4.4	33.9	4.1
2	1	11/8/01	9:22	62	38	8,600	30,100	27,500	66,500	20.1	4.4	33.7	4.1
2	1	11/8/01	9:30	66	49	9,300	29,300	28,900	67,600	20.2	4.4	33.7	4.1
2	1	11/8/01	9:38	67	30	9,200	31,400	29,400	70,300	20.1	4.4	33.7	4.1
2	1	11/8/01	9:46	67	38	9,100	31,400	30,800	71,500	20.1	4.3	33.7	4.1
2	1	11/8/01	10:01	70	30	9,700	30,600	30,500	70,900	20.1	4.4	33.7	4.1
2	1	11/8/01	10:08	73	38	9,500	32,900	30,900	73,500	20.1	4.3	33.6	4.1
2	1	11/8/01	10:16	74	38	9,700	31,800	31,500	73,200	20.2	4.4	33.8	4.1
2	1	11/8/01	10:23	75	49	9,900	32,400	31,700	74,100	20.1	4.4	33.6	4.1
2	1	11/8/01	10:31	76	30	9,700	32,600	31,000	73,500	20.1	4.4	33.7	4.1
2	1	11/8/01	10:39	78	38	10,800	33,600	31,000	75,600	20.0	4.3	33.5	4.1
2	1	11/8/01	10:47	80	48	9,600	32,300	30,300	72,300	20.2	4.3	33.6	4.1
2	1	11/8/01	10:55	83	29	9,900	32,900	32,600	75,600	20.1	4.3	33.6	4.1

	5 FL Pie	zo Lane 2 -	- Raw dat	a Truck	l			abto (lbo)		10		in an (ft)	
Piezo Truck #	Lane	Date	Time	Tomp	Speed	Steer	WIM Weig Tandem1	Tandem2	Gross	A-B	VIM Spac B-C	C-D	D-E
1 1	2	11/7/01	13:03	Temp	Speed 28					<u>А-Б</u> 16.8	<u>в-с</u> 4.4	34.5	4.1
				95		13,200	29,500	36,000	78,900				
1	2	11/7/01	13:11	98	37	13,700	30,700	31,700	76,300	16.8	4.3	34.5	4.1
1	2	11/7/01	13:23	99	47	13,700	29,400	35,700	79,000	16.8	4.3	34.5	4.1
1	2	11/7/01	13:31	98	27	12,300	31,100	32,600	76,300	16.7	4.3	34.4	4.1
1	2	11/7/01	13:39	98	37	12,800	31,800	35,300	80,100	16.7	4.3	34.4	4.1
1	2	11/7/01	13:45	97	46	12,900	31,500	35,400	80,000	16.7	4.3	34.3	4.0
1	2	11/7/01	13:54	96	28	13,500	29,700	36,100	79,500	16.8	4.4	34.6	4.1
1	2	11/7/01	14:02	100	38	13,800	29,300	36,400	79,500	16.8	4.4	34.4	4.1
1	2	11/7/01	14:10	99	48	13,800	30,600	31,500	76,000	16.9	4.3	34.6	4.1
1	2	11/7/01	14:17	99	27	12,200	30,500	32,700	75,600	16.7	4.3	34.4	4.1
1	2	11/7/01	14:25	96	38	12,700	31,300	33,900	78,000	16.8	4.3	34.4	4.1
1	2	11/7/01	14:31	96	47	13,900	31,000	31,100	76,100	16.8	4.3	34.5	4.1
1	2	11/7/01	14:39	97	28	12,800	31,200	31,900	76,000	16.7	4.3	34.4	4.1
1	2	11/7/01	14:47	97	37	13,100	32,400	33,000	78,700	16.8	4.3	34.4	4.1
1	2	11/7/01	14:55	97	47	12,500	31,500	34,600	78,700	16.7	4.3	34.4	4.0
1	2	11/7/01	15:03	97	28	12,600	31,100	32,500	76,400	16.8	4.4	34.5	4.1
1	2	11/7/01	15:14	96	38	13,100	31,900	36,300	81,400	16.8	4.4	34.6	4.1
1	2	11/7/01	15:22	96	47	12,400	31,900	34,300	78,800	16.7	4.3	34.3	4.0
1	2	11/7/01	15:29	96	28	13,000	30,600	35,900	79,700	16.8	4.3	34.5	4.1
1	2	11/7/01	15:38	91	37	13,000	28,200	32,600	74,100	16.7	4.3	34.3	4.0
1	2	11/7/01	15:45	91	46	12,400	31,900	33,700	78,300	16.8	4.3	34.4	4.0
1	2	11/7/01	15:54	84	28	13,500	31,000	31,100	75,800	16.8	4.3	34.5	4.1
1	2	11/7/01	16:02	83	38	12,600	32,300	35,400	80,400	16.8	4.4	34.4	4.1
1	2	11/7/01	16:11	81	48	13,600	30,300	31,600	75,700	16.8	4.3	34.4	4.1
1	2	11/7/01	16:19	80	29	12,900	31,300	34,500	78,900	16.7	4.3	34.3	4.1
1	2	11/8/01	6:14	41	28	12,900	29,600	35,200	78,100	16.7	4.3	34.3	4.1
1	2	11/8/01	6:24	41	37	13,100	32,700	36,200	82,200	16.8	4.3	34.5	4.1
1	2	11/8/01	6:32	41	47	13,000	31,400	36,300	80,900	16.8	4.3	34.4	4.1
1	2	11/8/01	6:40	41	27	13,400	30,900	30,200	74,600	16.8	4.3	34.4	4.1
1	2	11/8/01	6:50	42	37	12,900	31,800	32,700	77,600	16.8	4.3	34.4	4.1
1	2	11/8/01	9:04	60	28	12,300	29,800	29,200	71,500	16.8	4.3	34.4	4.1
-	—					,	,	,	.,				

Table 8-6 FL Piezo Lane 2 – Raw data Truck 1

Piezo							WIM Wei	ghts (lbs)		V	/IM Spac	ings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	2	11/8/01	9:12	61	37	12,800	29,200	35,700	77,900	16.7	4.3	34.4	4.1
1	2	11/8/01	9:20	62	46	12,500	30,000	29,300	72,000	16.8	4.3	34.5	4.1
1	2	11/8/01	9:28	66	27	12,800	28,500	35,500	77,100	16.8	4.3	34.4	4.1
1	2	11/8/01	9:36	67	37	13,300	29,400	35,900	79,000	16.7	4.3	34.3	4.1
1	2	11/8/01	9:43	67	46	12,100	27,700	29,300	69,400	16.7	4.3	34.3	4.1
1	2	11/8/01	9:51	68	28	10,500	27,300	29,700	67,800	16.7	4.3	34.4	4.1
1	2	11/8/01	9:58	70	36	12,200	30,700	34,000	77,100	16.8	4.3	34.3	4.1
1	2	11/8/01	10:06	72	47	12,900	29,000	34,200	76,200	16.8	4.3	34.6	4.1
1	2	11/8/01	10:13	74	28	11,900	29,700	30,800	72,500	16.8	4.3	34.4	4.1
1	2	11/8/01	10:21	75	37	13,000	29,700	36,000	78,900	16.8	4.3	34.4	4.1
1	2	11/8/01	10:28	76	47	12,600	30,500	30,300	73,500	16.8	4.3	34.6	4.1
1	2	11/8/01	10:37	78	28	12,100	31,100	31,800	75,200	16.8	4.3	34.5	4.1
1	2	11/8/01	10:45	80	37	13,800	28,400	34,300	76,700	16.8	4.4	34.5	4.1
1	2	11/8/01	10:52	82	46	12,300	30,300	34,400	77,200	16.8	4.4	34.6	4.1

Table 8-7 FL Piezo Lane 2 – Raw data Truck 2

Piezo							WIM Wei	ghts (lbs)		W	/IM Spac	ings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	2	11/7/01	13:01	95	29	12,600	27,700	32,700	73,200	20.2	4.4	33.8	4.1
2	2	11/7/01	13:10	98	39	10,300	27,600	29,000	67,200	20.2	4.3	33.7	4.1
2	2	11/7/01	13:21	98	50	9,800	27,700	31,400	69,100	20.2	4.3	33.7	4.1
2	2	11/7/01	13:37	98	39	12,400	28,400	29,400	70,400	20.3	4.4	33.6	4.1
2	2	11/7/01	13:45	97	30	12,400	29,200	34,500	76,200	20.2	4.3	33.7	4.1
2	2	11/7/01	13:53	96	49	13,200	26,300	30,700	70,400	20.2	4.3	33.7	4.1
2	2	11/7/01	14:00	100	29	11,900	31,300	32,300	75,600	20.2	4.4	33.8	4.1
2	2	11/7/01	14:08	99	50	11,700	25,700	27,500	65,000	20.2	4.3	33.7	4.1
2	2	11/7/01	14:15	99	29	12,500	30,300	34,900	77,900	20.2	4.3	33.7	4.1
2	2	11/7/01	14:23	97	39	10,800	28,000	29,800	68,800	20.1	4.3	33.6	4.1
2	2	11/7/01	14:30	96	49	13,100	29,600	33,700	76,500	20.2	4.4	33.8	4.1
2	2	11/7/01	14:38	97	29	12,900	27,500	34,700	75,300	20.2	4.4	33.8	4.1
2	2	11/7/01	14:46	97	39	11,900	25,500	29,000	66,500	20.2	4.4	33.8	4.1

Piezo							WIM Wei	ghts (lbs)		W	/IM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	2	11/7/01	14:53	97	49	13,400	30,400	30,900	74,800	20.2	4.3	33.8	4.1
2	2	11/7/01	15:02	97	30	12,300	32,400	31,900	76,800	20.3	4.4	33.9	4.1
2	2	11/7/01	15:11	97	39	10,900	28,300	28,700	68,100	20.3	4.4	33.9	4.1
2	2	11/7/01	15:19	96	50	10,200	27,500	29,600	67,500	20.3	4.4	33.9	4.1
2	2	11/7/01	15:27	96	29	11,700	30,900	35,200	77,900	20.1	4.3	33.5	4.1
2	2	11/7/01	15:36	96	37	10,600	28,600	31,500	70,900	20.2	4.3	33.6	4.1
2	2	11/7/01	15:44	91	49	11,900	31,900	32,800	76,800	20.1	4.4	33.7	4.1
2	2	11/7/01	15:53	84	29	12,700	30,800	36,000	79,700	20.3	4.4	33.8	4.1
2	2	11/7/01	16:01	83	39	10,700	27,300	30,200	68,400	20.1	4.3	33.6	4.1
2	2	11/7/01	16:09	81	40	12,800	28,500	30,300	71,800	20.3	4.4	34.0	4.1
2	2	11/7/01	16:18	80	30	13,400	28,100	31,100	72,800	20.2	4.3	33.7	4.1
2	2	11/8/01	6:12	41	29	10,000	27,000	31,800	69,000	20.3	4.4	33.7	4.1
2	2	11/8/01	6:21	41	37	12,800	27,200	32,900	73,100	20.2	4.4	33.7	4.1
2	2	11/8/01	6:30	40	50	13,400	26,900	32,600	73,000	20.2	4.3	33.8	4.1
2	2	11/8/01	6:38	41	30	12,400	32,000	37,200	81,700	20.3	4.4	33.9	4.1
2	2	11/8/01	6:46	41	29	11,900	30,100	39,200	81,400	20.1	4.3	33.6	4.1
2	2	11/8/01	6:54	42	39	12,300	27,800	32,800	73,100	20.2	4.3	33.8	4.1
2	2	11/8/01	9:02	60	30	12,600	29,300	31,200	73,300	20.3	4.4	34.0	4.1
2	2	11/8/01	9:10	61	39	12,700	27,900	36,400	77,200	20.2	4.3	33.8	4.1
2	2	11/8/01	9:18	62	38	12,600	28,300	33,000	74,200	20.2	4.3	33.7	4.1
2	2	11/8/01	9:26	62	50	11,200	30,000	34,800	76,300	20.1	4.3	33.7	4.1
2	2	11/8/01	9:33	66	26	11,600	26,800	31,800	70,400	19.9	4.3	33.4	4.1
2	2	11/8/01	9:41	67	38	11,200	30,300	34,900	76,600	20.1	4.3	33.6	4.1
2	2	11/8/01	9:49	68	49	10,600	30,900	34,400	76,100	20.1	4.3	33.7	4.1
2	2	11/8/01	9:53	68	39	8,400	34,700	30,600	73,900	19.5	4.5	33.5	4.1
2	2	11/8/01	9:56	70	29	11,900	28,800	34,700	75,500	20.1	4.3	33.7	4.1
2	2	11/8/01	10:04	72	37	9,900	27,000	32,100	69,200	20.2	4.3	33.7	4.1
2	2	11/8/01	10:11	74	39	12,400	28,000	33,400	74,000	20.2	4.3	33.7	4.1
2	2	11/8/01	10:19	74	49	12,600	26,000	29,600	68,300	20.0	4.3	33.4	4.0
2	2	11/8/01	10:35	78	38	12,300	25,600	30,300	68,400	20.1	4.3	33.6	4.1
2	2	11/8/01	10:43	80	47	11,000	26,400	28,900	66,400	19.9	4.3	33.4	4.0
2	2	11/8/01	10:50	81	29	12,300	28,300	34,100	74,900	20.1	4.3	33.6	4.1

Bending p	lata							ghts (lbs)		۱۸	/IM Spac	vinge (ft)	
Senaing p Truck #		Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
	Lane 1	11/7/01	13:09	98	27								4.1
1	-					12,200	30,500	35,900	78,800	16.8	4.5	34.8	
1	1	11/7/01	13:20	98	37	12,000	30,400	34,300	76,900	17.0	4.4	35.1	4.1
1	1	11/7/01	13:29	99	45	11,800	29,600	33,900	75,500	16.4	4.4	34.3	4.1
1	1	11/7/01	13:36	98	27	11,900	30,400	35,000	77,600	16.6	4.3	34.6	4.1
1	1	11/7/01	13:44	98	36	12,000	29,500	32,700	74,300	16.7	4.3	34.5	4.0
1	1	11/7/01	13:52	96	45	11,700	30,400	34,800	77,000	16.4	4.4	33.6	4.1
1	1	11/7/01	13:59	100	28	12,200	30,500	34,900	77,800	16.2	4.4	33.5	4.0
1	1	11/7/01	14:08	99	37	12,100	30,000	34,400	76,800	17.0	4.4	35.1	4.1
1	1	11/7/01	14:15	99	47	11,500	30,700	34,700	77,200	16.8	4.2	34.3	3.9
1	1	11/7/01	14:23	97	28	12,200	31,000	35,900	79,300	17.5	4.5	36.1	4.3
1	1	11/7/01	14:30	96	37	12,500	30,300	33,900	76,900	17.0	4.5	34.9	3.9
1	1	11/7/01	14:36	96	45	11,500	31,300	34,800	77,800	16.1	4.1	32.9	3.7
1	1	11/7/01	14:45	97	27	12,200	30,500	36,400	79,200	16.4	4.4	33.7	4.0
1	1	11/7/01	14:53	97	31	12,200	29,500	33,600	75,500	16.2	4.4	33.8	3.9
1	1	11/7/01	15:01	97	45	12,100	30,900	34,100	77,300	16.4	4.1	33.9	3.7
1	1	11/7/01	15:09	97	27	12,300	30,500	34,400	77,300	16.6	4.2	34.1	4.0
1	1	11/7/01	15:20	96	37	12,200	29,700	34,200	76,200	16.4	4.4	33.7	3.9
1	1	11/7/01	15:27	96	48	12,000	29,900	34,400	76,500	17.1	4.3	35.1	4.0
1	1	11/7/01	15:35	96	27	12,000	30,500	34,900	77,500	16.6	4.3	34.6	4.1
1	1	11/7/01	15:43	91	37	12,400	29,600	33,900	76,000	16.7	4.1	34.3	4.1
1	1	11/7/01	15:51	87	48	12,200	30,200	34,700	77,300	17.5	4.3	35.4	4.3
1	1	11/7/01	16:00	83	27	12,000	30,500	34,900	77,600	16.4	4.3	34.1	4.1
1	1	11/7/01	16:08	83	37	12,400	30,000	33,800	76,300	16.1	4.4	33.4	3.9
1	1	11/7/01	16:17	80	47	11,600	30,700	34,900	77,400	16.8	4.2	33.6	3.9
1	1	11/8/01	6:30	40	36	12,100	29,400	33,300	75,000	16.4	4.3	33.7	4.1
1	1	11/8/01	6:38	41	46	11,700	29,700	34,300	75,700	16.8	4.2	33.9	4.2
1	1	11/8/01	6:48	41	27	11,700	30,000	34,500	76,400	16.8	4.5	34.8	4.1
1	1	11/8/01	7:00	42	37	12,100	29,400	33,900	75,500	17.0	4.2	35.2	3.9
1	1	11/8/01	9:09	61	27	11,900	30,100	34,200	76,300	17.0	4.3	35.0	4.1
1	1	11/8/01	9:17	62	36	12,200	29,700	34,700	76,800	16.4	4.3	34.3	3.8

 Table 8-8 FL Bending Plate Lane 1 – Raw data Truck 1

Bending p	olate						WIM Wei	ghts (lbs)		V	VIM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	1	11/8/01	9:27	62	44	12,000	30,000	35,100	77,400	16.1	4.3	33.6	4.0
1	1	11/8/01	9:33	66	31	12,000	30,000	34,100	76,300	16.9	4.4	35.0	4.2
1	1	11/8/01	9:41	67	37	11,900	29,600	34,600	76,300	16.7	4.4	34.6	3.9
1	1	11/8/01	9:48	67	47	11,700	29,700	35,000	76,600	17.1	4.2	35.0	4.2
1	1	11/8/01	9:56	68	26	11,900	30,100	34,500	76,700	16.4	4.4	33.7	4.0
1	1	11/8/01	10:03	72	36	12,100	29,700	33,900	75,900	17.0	4.3	34.5	4.3
1	1	11/8/01	10:11	73	45	11,900	29,300	34,900	76,300	16.4	4.4	34.3	4.1
1	1	11/8/01	10:14	74	27	11,900	30,100	34,900	77,200	16.7	4.6	34.5	4.1
1	1	11/8/01	10:19	74	25	11,900	29,400	33,400	74,900	16.2	4.2	33.5	4.0
1	1	11/8/01	10:26	76	37	12,000	29,200	33,200	74,500	16.7	4.1	34.6	3.9
1	1	11/8/01	10:33	78	47	11,900	29,700	35,400	77,200	17.1	4.6	35.0	4.2
1	1	11/8/01	10:43	80	26	12,200	30,400	35,900	78,600	16.4	4.3	33.9	3.9
1	1	11/8/01	10:51	81	36	12,100	29,700	34,300	76,300	16.4	4.1	34.0	4.1
1	1	11/8/01	10:58	83	46	11,900	30,000	35,200	77,300	16.8	4.2	35.0	3.8

Table 8-9 FL Bending Plate Lane 1 – Raw data Truck 1

Bending p	olate						WIM Wei	ghts (lbs)		V	VIM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	1	11/7/01	13:08	95	29	12,300	34,100	33,500	80,100	20.2	4.4	34.0	4.0
2	1	11/7/01	13:19	98	38	11,700	33,500	32,900	78,200	19.9	4.6	33.8	4.0
2	1	11/7/01	13:26	99	45	11,300	32,600	33,100	77,200	19.8	4.4	33.6	4.1
2	1	11/7/01	13:35	98	28	12,000	33,600	33,200	78,900	20.0	4.2	33.3	4.0
2	1	11/7/01	13:42	98	46	11,000	33,500	32,300	76,900	19.9	4.5	33.2	4.2
2	1	11/7/01	13:51	97	37	11,900	32,600	31,800	76,500	19.8	4.2	33.5	3.9
2	1	11/7/01	13:58	96	47	11,000	33,500	32,200	76,800	20.0	4.2	33.2	3.9
2	1	11/7/01	14:06	99	40	11,300	32,900	32,200	76,600	20.6	4.4	34.4	4.1
2	1	11/7/01	14:13	99	47	10,900	33,100	32,900	77,000	19.6	4.2	33.2	3.9
2	1	11/7/01	14:20	97	29	11,800	33,200	33,000	78,100	20.2	4.4	33.8	4.2
2	1	11/7/01	14:28	96	41	11,300	34,300	32,500	78,200	21.1	4.6	35.1	4.3
2	1	11/7/01	14:35	96	47	10,500	33,200	33,300	77,200	19.6	4.2	33.2	3.9
2	1	11/7/01	14:43	97	28	11,900	32,100	32,200	76,500	19.4	4.2	32.9	4.0

Bending	plate						WIM Wei	ghts (lbs)		W	/IM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	1	11/7/01	14:51	97	38	11,600	33,100	33,100	78,100	19.9	4.3	33.8	4.3
2	1	11/7/01	15:00	97	44	11,400	34,500	33,500	79,700	19.8	4.0	33.2	4.0
2	1	11/7/01	15:08	97	28	12,000	33,800	34,600	80,400	20.0	4.2	33.3	4.2
2	1	11/7/01	15:24	96	48	11,000	34,300	32,900	78,400	20.4	4.7	34.3	4.3
2	1	11/7/01	15:33	96	29	11,700	33,300	32,800	78,000	20.2	4.2	33.8	4.2
2	1	11/7/01	15:41	91	34	11,300	32,900	32,900	77,300	19.6	4.4	32.9	3.9
2	1	11/7/01	15:59	83	28	12,200	32,800	33,000	78,200	19.6	4.2	33.3	4.0
2	1	11/7/01	16:07	83	36	11,700	33,300	32,200	77,500	20.0	4.3	33.7	4.1
2	1	11/7/01	16:15	80	40	11,300	31,900	32,900	76,300	19.7	4.4	32.9	3.8
2	1	11/8/01	6:19	41	29	11,700	33,400	32,800	78,100	19.8	4.4	33.1	4.0
2	1	11/8/01	6:28	41	37	11,200	32,400	31,300	75,100	19.8	4.5	33.5	3.9
2	1	11/8/01	6:36	41	50	10,700	32,800	31,200	74,900	20.2	4.4	33.6	4.1
2	1	11/8/01	6:52	42	30	11,500	32,300	32,200	76,100	19.6	4.3	33.3	4.1
2	1	11/8/01	7:01	42	37	11,500	32,400	31,500	75,600	20.0	4.4	33.4	4.1
2	1	11/8/01	9:08	60	29	11,400	32,200	32,200	76,000	20.2	4.3	34.0	4.1
2	1	11/8/01	9:16	62	29	11,700	33,500	34,300	79,800	20.6	4.4	34.4	4.2
2	1	11/8/01	9:23	62	37	11,400	32,800	32,500	76,900	19.8	4.4	33.7	3.9
2	1	11/8/01	9:31	66	48	10,300	30,400	32,300	73,200	20.4	4.3	34.0	4.3
2	1	11/8/01	9:39	67	28	11,800	33,000	33,300	78,300	19.7	4.3	32.7	4.1
2	1	11/8/01	9:47	67	36	11,400	32,000	31,800	75,300	19.4	4.3	32.6	4.1
2	1	11/8/01	10:02	70	29	11,800	33,100	32,700	77,700	19.5	4.4	32.9	4.0
2	1	11/8/01	10:09	73	37	11,300	32,500	32,200	76,200	20.1	4.5	33.7	4.2
2	1	11/8/01	10:17	74	36	11,300	31,700	31,200	74,400	19.7	4.3	32.9	4.1
2	1	11/8/01	10:24	75	47	11,100	34,100	31,900	77,400	19.6	4.2	32.9	3.9
2	1	11/8/01	10:32	76	29	11,800	33,500	33,700	79,200	19.8	4.2	33.1	4.0
2	1	11/8/01	10:40	80	37	11,000	32,200	31,000	74,500	20.1	4.5	33.5	4.2
2	1	11/8/01	10:48	81	46	10,600	33,300	32,500	76,700	19.9	4.2	33.2	3.8
2	1	11/8/01	10:56	83	28	11,500	32,800	32,300	76,700	20.3	4.3	34.0	4.1

Bending p	olate						WIM Wei	ghts (lbs)		V	VIM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	4	11/7/01	13:04	95	29	12,200	30,600	34,800	77,800	17.3	4.5	35.4	4.1
1	4	11/7/01	13:12	98	37	12,300	32,000	36,000	80,500	17.0	4.5	35.1	4.2
1	4	11/7/01	13:24	99	47	11,500	30,400	33,200	75,300	17.0	4.2	35.2	4.2
1	4	11/7/01	13:32	98	27	12,000	30,700	35,200	78,100	17.1	4.3	35.3	4.1
1	4	11/7/01	13:40	98	37	11,900	30,300	36,200	78,600	17.0	4.4	35.1	4.1
1	4	11/7/01	13:47	97	47	11,500	30,800	34,200	76,600	17.4	4.2	35.6	3.9
1	4	11/7/01	13:55	96	28	12,000	30,800	34,900	77,900	16.9	4.2	34.7	4.2
1	4	11/7/01	14:04	100	38	11,800	30,400	34,400	76,900	17.0	4.6	34.9	4.3
1	4	11/7/01	14:11	99	47	11,700	30,200	34,200	76,300	16.7	4.2	34.5	3.9
1	4	11/7/01	14:18	99	27	12,400	31,500	35,500	79,600	16.9	4.5	34.6	4.1
1	4	11/7/01	14:26	96	39	12,000	29,800	34,100	76,000	17.9	4.4	36.7	4.4
1	4	11/7/01	14:32	96	49	11,500	31,200	35,200	78,100	17.8	4.4	36.4	4.4
1	4	11/7/01	14:40	97	28	12,100	31,100	35,300	78,800	17.1	4.5	35.3	4.2
1	4	11/7/01	14:48	97	39	12,200	30,500	35,100	78,000	17.9	4.4	37.0	4.1
1	4	11/7/01	14:56	97	48	11,600	30,600	33,700	76,000	17.8	4.3	36.0	4.3
1	4	11/7/01	15:05	97	28	12,000	30,500	35,000	77,700	16.9	4.5	35.1	4.0
1	4	11/7/01	15:15	96	38	12,000	30,200	34,700	77,100	17.3	4.3	35.4	4.3
1	4	11/7/01	15:23	96	48	11,400	30,800	34,100	76,500	17.4	4.3	35.2	4.3
1	4	11/7/01	15:30	96	28	11,900	30,600	35,200	77,900	17.1	4.4	35.1	4.2
1	4	11/7/01	15:39	91	37	11,800	30,300	34,700	77,000	16.7	4.5	34.5	4.2
1	4	11/7/01	15:46	87	47	10,900	30,700	34,500	76,300	17.4	4.2	35.6	3.9
1	4	11/7/01	15:55	84	28	12,300	30,600	35,200	78,200	17.3	4.5	35.3	4.2
1	4	11/7/01	16:03	83	39	11,700	30,300	34,700	77,000	17.3	4.4	35.5	4.4
1	4	11/7/01	16:12	81	47	11,300	31,100	34,300	77,000	17.0	4.2	34.5	3.9
1	4	11/7/01	16:20	80	30	12,000	30,900	35,400	78,400	17.4	4.5	35.4	4.2
1	4	11/8/01	6:15	41	27	11,700	29,800	34,800	76,600	16.9	4.3	34.7	4.1
1	4	11/8/01	6:25	41	37	11,500	30,000	34,500	76,200	16.7	4.4	34.2	4.1
1	4	11/8/01	6:33	40	48	11,400	30,400	33,000	75,000	17.4	4.3	36.0	4.3
1	4	11/8/01	6:41	41	27	11,700	29,800	34,500	76,300	16.9	4.3	34.6	4.1
1	4	11/8/01	6:51	42	38	11,700	30,400	34,800	77,000	17.3	4.6	35.4	4.3

Table 8-10 FL Bending Plate Lane 4 – Raw data Truck 1

Bending p	olate						WIM Wei	ghts (lbs)		V	VIM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	4	11/8/01	9:05	60	29	11,700	29,800	33,600	75,200	17.3	4.5	35.8	4.3
1	4	11/8/01	9:13	61	38	11,400	29,900	34,900	76,300	17.3	4.6	35.7	4.3
1	4	11/8/01	9:21	62	44	11,200	30,600	34,100	76,200	16.3	4.0	33.7	4.0
1	4	11/8/01	9:29	66	27	11,800	29,400	33,100	74,500	16.9	4.3	34.9	4.1
1	4	11/8/01	9:37	67	38	11,700	30,100	34,400	76,300	17.3	4.6	35.4	4.3
1	4	11/8/01	9:44	67	47	11,100	29,900	33,400	74,700	17.4	4.2	35.6	4.2
1	4	11/8/01	9:52	68	28	11,800	29,900	33,900	75,700	16.9	4.4	34.9	4.0
1	4	11/8/01	9:59	70	36	11,500	30,100	35,000	76,800	17.0	4.6	34.8	4.1
1	4	11/8/01	10:07	72	45	11,400	30,300	33,400	75,200	16.7	4.0	34.1	4.0
1	4	11/8/01	10:22	74	36	11,800	30,300	35,200	77,500	16.4	4.3	33.7	4.1
1	4	11/8/01	10:29	76	46	11,200	29,700	33,300	74,500	16.7	4.8	34.1	4.1
1	4	11/8/01	10:38	78	29	11,700	30,500	36,000	78,300	17.3	4.5	35.4	4.1
1	4	11/8/01	10:46	80	37	12,000	30,500	35,500	78,200	16.7	4.4	34.5	3.9
1	4	11/8/01	10:53	83	48	11,400	30,100	33,900	75,600	17.4	4.7	36.3	4.3

Table 8-11 FL Bending Plate Lane 4 – Raw data Truck 2

Bending p	olate						WIM Wei	ghts (lbs)		V	VIM Spac	ings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	4	11/7/01	13:02	95	29	12,000	34,100	33,100	79,500	20.9	4.4	35.0	4.4
2	4	11/7/01	13:11	98	41	11,100	33,700	32,700	77,600	21.3	4.6	35.6	4.3
2	4	11/7/01	13:22	98	49	11,100	32,500	33,100	76,800	20.4	4.4	33.8	4.0
2	4	11/7/01	13:29	99	27	11,900	33,400	34,400	79,900	20.7	4.6	34.7	4.1
2	4	11/7/01	13:38	98	39	10,800	33,300	33,100	77,400	19.9	4.4	33.1	4.1
2	4	11/7/01	13:46	97	29	11,900	33,900	33,200	79,200	20.2	4.4	34.1	4.2
2	4	11/7/01	13:54	96	46	11,000	33,800	33,600	78,600	19.5	4.1	32.4	3.8
2	4	11/7/01	14:01	100	29	11,900	33,100	33,500	78,600	20.5	4.4	34.3	4.2
2	4	11/7/01	14:09	99	50	11,000	33,800	32,600	77,500	20.9	4.1	34.9	4.1
2	4	11/7/01	14:16	99	29	11,900	33,500	34,100	79,700	20.2	4.5	33.8	4.1
2	4	11/7/01	14:24	97	40	11,100	33,300	33,400	78,100	20.9	4.5	34.9	4.2
2	4	11/7/01	14:31	96	48	11,200	32,700	33,200	77,300	19.9	4.3	33.8	4.3
2	4	11/7/01	14:39	97	30	11,800	33,700	33,700	79,400	21.3	4.8	35.5	4.3

Bending	plate				V	VIM Spac	cings (ft)						
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	4	11/7/01	14:47	97	38	11,300	33,400	33,100	78,000	20.1	4.6	33.7	4.3
2	4	11/7/01	14:54	97	51	11,000	33,400	34,800	79,300	21.3	4.6	35.7	4.6
2	4	11/7/01	15:03	97	29	12,000	34,000	34,400	80,500	20.2	4.4	33.9	4.2
2	4	11/7/01	15:13	96	40	11,200	33,300	33,100	77,800	20.9	4.5	35.5	4.2
2	4	11/7/01	15:20	96	51	11,000	31,600	32,100	74,900	21.0	4.6	35.4	4.2
2	4	11/7/01	15:28	96	29	11,900	33,900	33,100	79,000	20.5	4.4	34.3	4.2
2	4	11/7/01	15:37	91	37	11,300	33,100	32,800	77,500	20.4	4.5	34.0	4.2
2	4	11/7/01	15:45	91	50	10,600	34,200	33,500	78,500	20.9	4.5	35.3	4.1
2	4	11/7/01	15:54	84	29	11,800	33,500	33,400	78,800	20.4	4.3	34.1	4.3
2	4	11/7/01	16:02	83	38	11,200	32,500	33,200	77,000	19.9	4.3	33.4	4.0
2	4	11/7/01	16:11	81	39	11,400	33,800	32,400	77,700	20.2	4.4	33.7	4.1
2	4	11/7/01	16:19	80	30	11,700	34,100	33,400	79,300	20.3	4.5	34.3	4.1
2	4	11/8/01	6:14	41	29	11,300	33,200	33,100	77,900	20.4	4.4	34.1	4.1
2	4	11/8/01	6:22	41	37	11,100	32,600	31,900	75,800	20.0	4.4	34.0	4.1
2	4	11/8/01	6:31	40	50	10,700	32,600	32,000	75,500	20.5	4.5	34.6	4.1
2	4	11/8/01	6:39	41	30	11,500	33,000	33,400	78,200	20.3	4.5	34.3	4.3
2	4	11/8/01	6:44	41	26	11,300	33,400	32,900	77,800	19.8	4.4	32.9	4.0
2	4	11/8/01	6:47	41	29	11,500	33,200	33,000	77,900	20.9	4.4	35.0	4.4
2	4	11/8/01	6:55	42	38	10,900	32,100	31,800	74,900	19.9	4.3	33.7	4.0
2	4	11/8/01	9:03	60	30	11,500	33,000	32,500	77,100	20.8	4.5	34.6	4.3
2	4	11/8/01	9:11	61	40	11,100	32,600	32,400	76,400	21.5	4.5	35.5	4.5
2	4	11/8/01	9:19	62	38	11,200	32,000	32,900	76,300	20.1	4.6	33.7	4.3
2	4	11/8/01	9:27	62	50	10,800	32,500	31,500	75,000	20.5	4.5	34.6	4.1
2	4	11/8/01	9:34	66	27	11,400	32,700	32,900	77,200	20.8	4.5	34.9	4.3
2	4	11/8/01	9:42	67	39	11,300	33,600	34,300	79,300	21.2	4.7	35.5	4.4
2	4	11/8/01	9:50	68	48	10,700	32,200	32,500	75,600	19.9	4.3	33.8	4.0
2	4	11/8/01	9:57	70	30	11,700	33,200	33,500	78,600	20.8	4.5	35.2	4.3
2	4	11/8/01	10:05	72	38	10,900	32,900	33,500	77,500	20.7	4.6	34.9	4.3
2	4	11/8/01	10:12	74	39	11,100	32,700	32,800	76,800	20.3	4.4	34.3	4.1
2	4	11/8/01	10:20	75	50	10,700	31,400	31,900	74,200	20.9	4.5	35.3	4.1
2	4	11/8/01	10:27	76	29	11,800	32,000	32,800	76,800	21.1	4.6	35.6	4.4
2	4	11/8/01	10:36	78	37	11,200	32,200	32,300	75,900	19.8	4.2	33.4	3.9

Bending p	olate						WIM Wei	ghts (lbs)		V	/IM Spac	cings (ft)	
Truck #	Lane	Date	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
2	4	11/8/01	10:44	80	48	10,800	31,400	31,300	73,600	20.7	4.7	34.9	4.3
2	4	11/8/01	10:51	82	30	11,800	33,100	32,000	77,000	20.8	4.5	35.0	4.3

8.3 Maryland

Table 8-12 MD Static Test Truck Measurements

Test	S	static Measu	rements (lbs	6)		Static Sp	acings (ft)		
Trucks	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E	
1	11,600	30,500	36,000	78,100	19.3	4.3	25.7	5.1	-
2	11,200	34,800	32,400	78,400	20.0	4.4	31.0	4.1	

Table 8-13 MD Raw data for LTPP lane

		Pavement			WIM Wei	ghts (lbs)			WIM Spa	acings (ft)	
Truck #	Time	Temp	Speed	Steer	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	8:16	75	60	11,300	36,700	42,500	90,500	19.1	4.3	25.5	5.0
1	8:29	78	56	9,600	32,200	39,200	81,000	19.2	4.2	25.7	5.1
1	8:40	79	55	11,300	34,400	44,700	90,400	19.0	4.3	25.4	5.0
1	8:50	79	55	12,200	36,400	42,400	91,000	19.1	4.3	25.6	5.0
1	9:02	80	57	11,200	30,100	38,300	79,600	19.2	4.3	25.5	5.1
1	9:15	82	52	11,500	30,200	38,000	79,700	19.3	4.3	25.7	5.1
1	9:28	83	57	9,200	30,600	36,000	75,800	19.2	4.3	25.5	5.1
1	9:40	86	55	11,500	29,900	36,800	78,200	19.2	4.3	25.6	5.1
1	9:50	91	46	11,700	28,100	36,200	76,000	19.1	4.3	25.6	5.1
1	11:27	110	53	11,000	29,100	35,200	75,300	19.1	4.3	25.7	5.1
1	11:38	110	55	11,300	28,100	34,000	73,400	19.1	4.3	25.7	5.0
1	11:50	113	55	10,800	29,400	35,900	76,100	19.2	4.3	25.5	5.1
1	12:01	113	55	9,600	25,600	31,700	66,900	19.1	4.3	25.5	5.1
1	12:12	113	56	11,300	25,800	31,200	68,300	19.1	4.4	25.7	5.1
1	12:24	114	56	13,400	29,500	36,500	79,400	19.1	4.3	25.7	5.1
1	12:37	116	53	11,900	25,100	29,700	66,700	19.0	4.3	25.5	5.0

		Pavement			WIM Wei	ghts (lbs)			WIM Spa	acings (ft)	
Truck #	Time	Temp	Speed	Steer		Tandem2	Gross	A-B	B-C	C-D	D-E
1	12:52	119	46	14,100	27,700	34,400	76,200	19.2	4.3	26.0	5.2
1	13:10	120	49	10,500	25,400	30,000	65,900	19.0	4.3	25.5	5.0
1	13:21	119	53	11,800	23,200	27,800	62,800	19.0	4.3	25.5	5.0
1	14:29	124	62	11,400	28,000	32,400	71,800	19.0	4.3	25.4	5.0
1	14:43	115	61	10,900	27,000	32,100	70,000	19.0	4.3	25.6	5.1
1	14:54	117	61	13,900	31,000	36,400	81,300	19.0	4.3	25.4	5.0
1	15:05	117	62	12,300	27,300	31,300	70,900	19.1	4.4	25.6	5.1
1	15:17	117	62	12,300	25,700	30,100	68,100	19.1	4.2	25.5	5.1
2	8:19	76	59	12,800	38,100	35,500	86,400	19.6	4.3	30.7	4.1
2	8:31	77	58	10,500	38,100	36,700	85,300	19.7	4.3	30.9	4.0
2	8:43	79	61	13,700	36,600	42,500	92,800	19.6	4.4	30.6	4.1
2	8:55	79	60	10,700	38,000	39,200	87,900	19.6	4.3	30.7	4.0
2	9:07	80	61	11,500	28,300	30,500	70,300	19.6	4.3	30.5	4.0
2	9:21	82	55	10,300	37,300	36,400	84,000	19.7	4.3	30.5	4.0
2	9:34	84	55	12,400	33,000	34,600	80,000	19.8	4.4	30.9	4.0
2	9:48	91	55	11,800	33,700	33,300	78,800	19.7	4.4	30.6	4.0
2	10:01	95	55	9,400	31,700	32,200	73,300	19.7	4.3	30.5	4.0
2	11:30	110	51	11,800	29,300	30,800	71,900	19.7	4.4	30.6	4.1
2	11:42	111	49	10,900	31,000	31,200	73,100	19.7	4.3	30.5	4.0
2	11:55	113	50	9,500	27,200	27,000	63,700	19.7	4.3	30.5	4.1
2	12:08	113	50	12,200	30,900	29,500	72,600	19.7	4.4	30.7	4.1
2	12:21	113	50	12,600	31,700	30,200	74,500	19.7	4.4	30.5	4.1
2	12:34	117	55	11,800	29,000	28,700	69,500	19.5	4.3	30.4	4.0
2	12:47	117	56	12,300	28,700	27,900	68,900	19.7	4.3	30.6	4.1
2	12:59	118	55	11,900	27,700	27,900	67,500	19.7	4.3	30.7	4.1
2	13:12	118	56	12,100	32,300	30,400	74,800	19.7	4.4	30.7	4.0
2	13:25	116	55	13,300	32,600	31,700	77,600	19.7	4.3	30.5	4.0
2	14:32	123	59	12,500	28,600	29,100	70,200	19.4	4.3	31.3	4.0
2	14:44	114	61	11,400	29,800	29,300	70,500	19.6	4.3	30.5	4.0
2	14:57	119	61	11,700	31,200	31,100	74,000	19.7	4.3	31.6	4.0
2	15:11	116	60	12,700	31,700	32,500	76,900	19.6	4.3	30.4	4.0
2	15:23	118	62	13,200	30,300	30,600	74,100	19.7	4.3	30.6	4.1

8.4 Michigan

Table 8-14 MI Static Test Truck Measurements

Test Trucks		Static N	Measuremei			Static Spa	acings (ft)			
	Steer	Single 2	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E	
1	11,800		26,400	30,400	68,600	11.1	4.5	30.3	4.5	-
3	6,700	9,000			15,700	13.5				
4	11,300	21,800			33,100	15.7				

Table 8-15 MI Raw data for LTPP lane

		Pavement			WI	M Weights ((lbs)			WIM Spa	acings (ft)	
Truck #	Time	Temp	Speed	Steer	Single 2	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
1	8:29	19	48	11,900		28,300	26,700	66,900	12.0	4.6	31.7	4.0
1	8:52	20	53	11,400		29,500	26,800	67,700	12.0	4.8	31.9	4.3
1	9:00	24	58	11,900		31,000	31,600	74,500	12.4	4.9	32.4	4.5
1	9:31	25	48	11,800		28,500	26,800	67,100	12.1	5.0	32.0	4.2
1	10:42	26	57	11,400		29,500	26,700	67,600	12.0	4.6	31.9	4.3
1	11:01	28	48	11,500		28,900	20,500	60,900	12.5	4.8	32.4	4.1
1	11:22	30	53	11,600		27,200	23,800	62,600	12.3	5.1	32.7	4.3
1	11:42	31	58	11,300		28,200	26,300	65,800	12.0	4.4	31.6	4.3
1	12:02	32	49	11,500		30,200	29,300	71,000	12.4	4.9	32.4	4.5
1	12:23	30	53	10,900		27,200	25,100	63,200	12.5	5.0	32.2	4.4
1	12:42	36	58	11,100		25,900	21,100	58,100	12.0	4.8	31.6	4.6
1	13:03	39	48	11,800		30,200	22,200	64,200	12.7	5.0	33.4	4.6
1	13:28	39	61	11,200		28,000	26,400	65,600	12.2	4.9	32.7	4.3
1	13:49	34	48	11,300		29,300	26,700	67,300	12.5	4.8	32.4	4.5
1	9:53	28	54	11,000		27,500	27,000	65,500	12.8	4.9	33.0	4.3
3	10:27	28	49	6,700	10,800			17,500	13.5			
3	10:32	26	52	6,700	11,100			17,800	13.2			
3	10:39	27	57	7,000	10,300			17,300	13.0			
3	10:44	27	47	6,400	10,900			17,300	13.5			
3	10:49	27	50	6,500	11,000			17,500	13.8			

		Pavement			WI	M Weights (lbs)			WIM Spa	acings (ft)	
Truck #	Time	Temp	Speed	Steer	Single 2	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
3	10:54	25	57	6,700	10,200			16,900	13.8			
3	10:59	29	46	6,800	11,000			17,800	13.1			
3	11:04	29	51	6,700	10,900			17,600	13.8			
3	11:09	30	56	6,700	10,200			16,900	13.6			
3	11:14	29	47	6,500	10,800			17,300	13.5			
3	11:19	30	51	6,600	10,800			17,400	14.0			
3	11:24	30	56	6,600	10,100			16,700	13.3			
3	11:30	30	47	6,300	10,800			17,100	14.0			
3	11:35	30	51	6,700	10,600			17,300	13.1			
3	11:40	30	59	6,800	10,200			17,000	13.3			
3	11:55	31	57	6,500	10,100			16,600	13.3			
3	12:01	32	47	6,600	11,000			17,600	13.0			
3	12:05	31	52	6,400	10,700			17,100	13.5			
3	12:10	33	57	6,700	10,200			16,900	13.8			
3	12:15	31	47	6,500	10,800			17,300	13.3			
3	12:21	31	52	6,400	10,700			17,100	14.0			
3	12:27	32	56	6,700	10,300			17,000	13.8			
3	12:33	34	46	6,600	11,100			17,700	13.3			
3	12:39	33	53	6,600	10,700			17,300	13.5			
3	12:45	34	56	6,600	10,100			16,700	13.6			
3	12:49	34	47	6,300	10,800			17,100	13.3			
3	12:54	37	52	6,300	10,800			17,100	13.8			
3	12:59	38	55	6,700	10,000			16,700	13.6			
3	13:05	40	51	6,200	10,800			17,000	13.8			
3	13:10	40	59	6,500	10,100			16,600	13.6			
3	13:15	36	46	6,600	11,000			17,600	13.5			
3	13:20	39	52	6,500	10,400			16,900	13.0			
3	13:25	41	56	6,800	10,100			16,900	13.0			
3	13:31	38	47	6,400	10,900			17,300	13.4			
3	13:36	37	51	6,500	10,500			17,000	13.0			
3	13:40	35	57	6,500	10,100			16,600	13.3			
3	13:45	37	47	6,400	10,900			17,300	13.3			

		Pavement			WI	M Weights ((lbs)			WIM Spa	acings (ft)	
Truck #	Time	Temp	Speed	Steer	Single 2	Tandem1	Tandem2	Gross	A-B	B-C	C-D	D-E
3	13:50	38	52	6,400	10,900			17,300	13.0			
3	13:55	36	58	6,400	10,000			16,400	13.3			
3	14:00	42	47	6,400	10,900			17,300	13.4			
4	10:52	27	50	11,000	21,400			32400	15.3			
4	10:58	27	56	11,300	21,600			32900	15.2			
4	11:05	30	59	11,500	20,900			32400	15.1			
4	11:10	30	52	10,800	21,200			32000	15.3			
4	11:18	29	56	11,000	21,100			32100	15.5			
4	11:23	30	59	11,100	21,000			32100	15.7			
4	11:29	29	52	10,600	20,500			31100	15.3			
4	11:36	30	55	11,400	21,300			32700	15.2			
4	11:44	30	46	11,200	20,800			32000	15.4			
4	11:49	29	51	10,300	21,200			31500	15.3			
4	11:54	32	54	10,900	21,400			32300	15.5			
4	11:59	30	56	11,300	21,500			32800	15.5			
4	12:03	32	59	11,400	21,400			32800	15.7			
4	12:09	34	51	10,400	21,200			31600	15.0			
4	12:14	31	56	10,900	21,600			32500	15.5			
4	13:07	40	52	10,800	20,800			31600	15.5			
4	13:12	38	55	11,200	21,700			32900	15.5			
4	13:18	40	59	10,700	20,800			31500	15.4			
4	13:23	40	52	10,500	21,200			31700	15.5			
4	13:32	36	55	11,200	21,200			32400	15.5			
4	13:37	36	59	10,900	20,200			31100	14.8			
4	13:42	36	52	11,000	20,400			31400	15.3			
4	13:47	35	56	11,200	20,700			31900	15.5			
4	13:52	38	59	11,100	20,800			31900	15.7			
4	13:57	36	50	10,600	21,200			31800	15.0			

8.5 Texas

Table 8-16 TX Static Test Truck Measurements

Test		Static N	Measuremen	its (lbs)			Sta	tic Spacings	s (ft)	
Trucks	Steer	Tandem1	Tandem2	Triple	Gross	A-B	B-C	C-D	D-E	E-F
1	10,100	29,000	39,500		78,500	10.25	4.33	29.58	10.00	
2	12,300	29,700		45,100	87,100	13.71	4.42	28.25	4.13	4.00
3	8,300	23,000	24,200		55,600	11.2	4.2	31.6	4.0	
4	12,500	38,300			50,700	14.7	4.4			

Table 8-17 TX Raw data for LTPP lane

		Temp		Mean		WIM Weights (lbs)						WIIM	Spacing	s (ft)	
Truck #	Time	#1	#2	Temp	Speed	Steer	Tandem1	Tandem2	Triple	Gross	A-B	B-C	C-D	D-E	E-F
1	9:57	68	61	64.5	52	10,200	29,200	40,700		80,100	11.2	4.4	30.0	10.1	
1	10:07	72	66	69.0	59	10,100	29,500	40,000		79,500	11.1	4.3	29.7	10.2	
1	10:20	71	69	70.0	65	10,900	29,100	39,700		79,700	10.9	4.4	29.4	10.2	
1	10:30	73	68	70.5	51	10,600	29,400	39,600		79,700	10.8	4.6	29.3	10.2	
1	10:43	75	73	74.0	57	9,900	29,400	40,100		79,400	11.0	4.2	29.5	9.8	
1	10:58	73	70	71.5	65	10,700	29,900	38,800		79,400	10.9	4.4	29.4	9.9	
1	11:08	80	76	78.0	52	10,300	29,700	40,600		80,600	11.2	4.4	30.0	10.1	
1	11:17	80	76	78.0	58	9,900	29,300	40,200		79,400	10.9	4.6	29.4	10.3	
1	13:53	97	89	93.0	52	10,000	29,100	39,300		78,400	10.9	4.4	29.8	9.8	
1	14:03	99	95	97.0	60	9,600	28,200	39,000		76,700	11.0	4.4	30.0	10.4	
1	14:16	99	100	99.5	65	10,000	29,200	39,900		79,100	10.9	4.1	29.7	9.9	
1	14:26	104	101	102.5	51	10,500	29,400	39,400		79,200	11.0	4.3	29.6	10.2	
1	14:35	105	103	104.0	59	9,600	28,800	39,600		78,000	10.8	4.3	29.7	9.9	
1	14:48	105	104	104.5	67	10,100	29,000	39,500		78,500	11.2	4.5	30.0	10.5	
2	9:57	68	61	64.5	53	12,300	29,700		45,100	87,100	13.6	4.4	28.3	3.9	4.2
2	10:06	72	66	69.0	60	12,100	27,800		45,400	85,400	13.5	4.4	28.4	4.1	3.8
2	10:19	71	69	70.0	70	12,100	26,500		48,400	87,000	13.8	4.7	29.1	4.0	4.4
2	10:29	73	68	70.5	55	12,500	29,500		44,300	86,200	14.1	4.3	29.0	4.0	4.0
2	10:43	75	73	74.0	60	11,900	27,500		44,000	83,600	13.5	4.4	28.1	4.1	4.1

		Temp		Mean			WI	/ Weights (I	bs)			WIIM	Spacing	s (ft)	
Truck #	Time	#1	#2	Temp	Speed	Steer	Tandem1	Tandem2	Triple	Gross	A-B	B-C	C-D	D-E	E-F
2	10:58	73	70	71.5	68	12,200	26,300		48,000	86,600	13.9	4.3	28.5	3.9	4.3
2	11:07	80	76	78.0	52	12,000	30,100		47,000	89,200	13.9	4.4	28.4	3.8	4.1
2	11:17	80	76	78.0	60	12,000	27,700		45,700	85,300	13.5	4.4	28.1	3.8	4.1
2	13:52	97	89	93.0	51	12,200	29,700		46,100	87,900	13.4	4.3	27.7	3.8	4.0
2	14:02	99	95	97.0	61	11,700	26,400		45,300	82,500	14.1	4.5	28.6	4.2	4.2
2	14:15	99	100	99.5	67	12,100	26,600		47,300	85,900	13.3	4.2	27.5	4.2	3.8
2	14:25	104	101	102.5	55	12,300	29,000		43,900	85,100	14.1	4.3	28.8	4.3	4.0
2	14:35	105	103	104.0	60	12,200	26,600		45,100	83,800	13.5	4.4	27.7	4.1	4.1
2	14:48	105	104	104.5	70	11,700	26,100		45,600	83,400	13.8	4.4	28.4	4.0	4.0
3	9:57	68	61	64.5	51	9,600	24,800	23,200		57,600	11.1	4.2	31.5	4.0	
3	10:06	72	66	69.0	59	8,900	24,900	23,500		57,200	11.3	4.3	32.3	4.0	
3	10:19	71	69	70.0	68	9,600	23,400	23,800		56,800	11.4	4.3	32.4	3.9	
3	10:29	73	68	70.5	55	9,500	25,400	25,100		60,000	11.5	4.3	32.6	4.2	
3	10:43	75	73	74.0	59	9,200	24,400	23,800		57,500	11.2	4.5	32.3	4.0	
3	10:58	73	70	71.5	68	9,100	23,600	24,500		57,200	11.2	4.3	32.4	3.9	
3	11:07	80	76	78.0	49	9,300	24,800	22,800		56,800	10.8	4.2	30.7	4.0	
3	11:17	80	76	78.0	60	9,300	24,500	24,600		58,500	11.2	4.4	32.3	4.1	
3	13:52	97	89	93.0	53	9,100	24,200	22,400		55,900	11.5	4.4	33.1	4.2	
3	14:02	99	95	97.0	58	8,600	23,300	23,100		55,000	11.1	4.2	31.9	4.0	
3	14:15	99	100	99.5	71	9,000	23,500	22,600		55,100	11.6	4.5	33.5	4.1	
3	14:25	104	101	102.5	53	9,000	24,400	23,900		57,400	11.4	4.4	32.5	4.2	
3	14:35	105	103	104.0	59	8,800	22,800	23,000		54,600	11.4	4.3	32.2	4.0	
3	14:48	105	104	104.5	67	8,300	23,000	24,200		55,600	11.2	4.2	31.6	4.0	
4	9:57	68	61	64.5	53	12,500	38,300			50,700	14.7	4.4			
4	10:06	72	66	69.0	60	12,100	33,100			45,200	14.8	4.4			
4	10:19	71	69	70.0	64	11,700	33,200			44,900	15.1	4.3			
4	10:29	73	68	70.5	53	12,300	38,100			50,400	14.4	4.2			
4	10:43	75	73	74.0	58	12,200	35,500			47,700	15.5	4.6			
4	10:58	73	70	71.5	64	12,300	33,200			45,500	15.4	4.7			
4	11:08	80	76	78.0	52	12,300	38,500			50,900	14.8	4.4			
4	11:17	80	76	78.0	58	12,300	35,100			47,300	14.6	4.3			
4	13:53	97	89	93.0	55	11,900	36,400			48,300	14.9	4.3			

		Temp				WIM Weights (lbs)					WIIM Spacings (ft)				
Truck #	Time	#1	#2	Temp	Speed	Steer	Tandem1	Tandem2	Triple	Gross	A-B	B-C	C-D	D-E	E-F
4	14:02	99	95	97.0	63	12,000	32,200			44,200	15.1	4.6			
4	14:16	99	100	99.5	63	11,800	35,300			47,100	15.1	4.3			
4	14:26	104	101	102.5	54	11,800	36,400			48,200	15.0	4.5			
4	14:35	105	103	104.0	61	12,200	32,000			44,200	14.8	4.5			
4	14:48	105	104	104.5	65	11,900	32,900			44,800	15.0	4.4			